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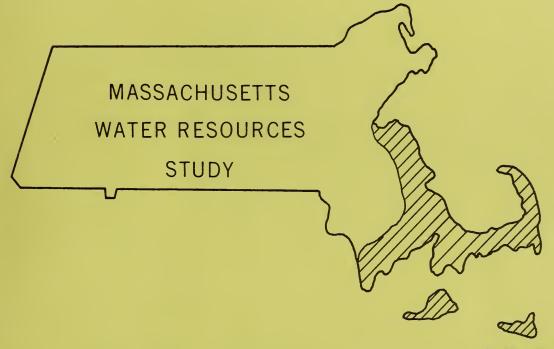


INVENTORY

of

POTENTIAL and EXISTING UPSTREAM RESERVOIR SITES

SOUTH SHORE, CAPE COD, BUZZARDS BAY & ISLANDS STUDY AREAS



U.S. DEPARTMENT OF AGRICULTURE NATIONAL SORIC

✓ Soil Conservation Service Economic Research Service Forest Service

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In cooperation with the

MASSACHUSETTS WATER RESOURCES COMMISSION

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FOREWORD

The United States Department of Agriculture, in cooperation with the Massachusetts Water Resources Commission, is participating in the Massachusetts Water Resources Study of the water and related land resources of the Commonwealth. One phase of the study is the inventorying of potential and existing upstream reservoir sites.

The Commonwealth of Massachusetts, through the Water Resources Commission, provides guidance and a significant financial contribution toward this phase of the Massachusetts Water Resources Study. The Massachusetts Water Resources Commission, to fulfill its responsibilities under Chapter 620, Acts of 1956 and Chapter 767, Acts of 1970, requires technical and engineering data and information on potential upstream reservoir sites. The Department of Agriculture is participating in this study under the provisions of Section 6 of the Watershed Protection and Flood Prevention Act (Public Law 566, 83rd Congress, as amended) which authorizes the Secretary of Agriculture to cooperate with other federal, state and local agencies, in surveys and investigations of the watersheds of rivers and other waterways as a basis for the development of coordinated programs.

This report, prepared by the Soil Conservation Service and submitted by the USDA Field Advisory Committee to the Water Resources Commission, identifies and inventories potential and existing upstream reservoir sites within the Buzzards Bay, Cape Cod, Islands, and South Shore Study Areas.

The Massachusetts Water Resources Commission will use this report, together with other reports and studies prepared by the United States Department of Agriculture and others, in the preparation of a comprehensive plan for the Commonwealth's water and land resources.

The information and data contained herein will also assist local, state and federal agencies in their specific planning activities for the coordinated and orderly conservation, development, utilization and management of the water and land resources to meet rapidly expanding needs.

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Soil Conservation Service

and Chairman,

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ACKNOWLEDGEMENTS

Acknowledgement is made to those who assisted in and contributed to the investigations, studies and development of this report. These include:

Board of Supervisors
Barnstable Conservation District
Bristol Conservation District
Dukes Conservation District
Nantucket Conservation District
Norfolk Conservation District

Plymouth Conservation District

Department of Civil Engineering University of Massachusetts - Amherst

Division of Water Pollution Control Massachusetts Water Resources Commission

Dam Inspection Engineers
Massachusetts Department of Public Works

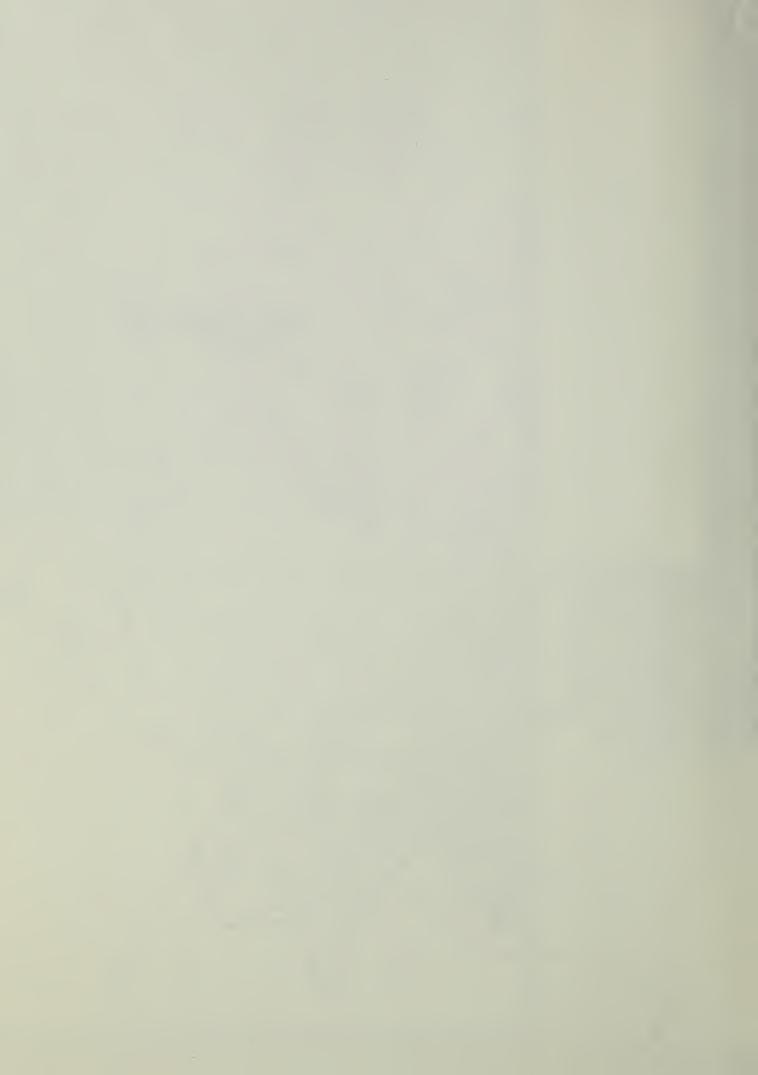
Soil Conservation Service personnel prepared this report. Lloyd French, Clifford Jones, John Raleigh III, and Richard Vieira developed the basic data under the direction of Walter Bolles and Larry Boutiette. Donald Mills investigated geological conditions. Gloria Rosen, Doris Butts and Patricia Cobb typed the manuscript. James Wesoloski was responsible for editing and publication.

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INVENTORY OF

POTENTIAL AND EXISTING UPSTREAM RESERVOIR SITES

in the

BUZZARDS BAY, CAPE COD, ISLANDS, AND SOUTH SHORE STUDY AREAS

prepared by the

UNITED STATES DEPARTMENT OF AGRICULTURE SOIL CONSERVATION SERVICE

in cooperation with the

MASSACHUSETTS WATER RESOURCES COMMISSION

INTRODUCTION

This report presents data on 65 potential and 112 existing reservoirs in the Buzzards Bay, Cape Cod, Islands, and South Shore Study Area in Barnstable, Bristol, Dukes, Nantucket, Norfolk, and Plymouth Counties in Massachusetts.

DESCRIPTION OF STUDY AREAS

The Buzzards Bay Study Area is located in Barnstable, Bristol and Plymouth Counties in southeastern Massachusetts. The main rivers include the Agawam, Acushnet, Mattapoisett, Westport, and Weweantic Rivers. The Study Area, which covers about 243,400 acres or 380 square miles, is divided into six subwatersheds. All or portions of sixteen cities and towns are located within the study area.

The Cape Cod Study Area consists of Barnstable County in southeastern Massachusetts. There are many small streams which originate in the center portion of the Cape and flow directly to the Atlantic Ocean, Cape Cod Bay, or Nantucket Sound. The Study Area, which covers about 269,800 acres or 422 square miles, is divided into nine subwatersheds. Fifteen towns are located within the Study Area.

The Islands Study Area consists of the islands in Dukes and Nantucket Counties located off the southern coast of Massachusetts. There are many small streams which flow directly to the Atlantic Ocean, Nantucket Sound, or Vineyard Sound. The study area, which covers about 103,100 acres or 161 square miles, is divided into two subwatersheds. Eight towns are located within the study area.

The South Shore Study Area is located in Bristol and Norfolk Counties in southeastern Massachusetts. The main rivers include the Eel, Jones, North, South, and Weir Rivers. The Study Area, which covers about 210,000 acres or 328 square miles, is divided into nine subwatersheds. All or portions of 29 cities and towns are located within the study area.

CRITERIA

Potential Reservoir Sites

The primary considerations used to identify potential reservoir sites were: suitable topography for a dam and reservoir, sufficient drainage area to maintain the proposed reservoir, and a relatively undeveloped pool area.

The following criteria were used as a guide in site selection:

- 1. Drainage area -- larger than one-half square mile, but not greater than 50 square miles.
- 2. Ratio of drainage area to beneficial pool area -- not less than 10 to 1.
- 3. Minimum beneficial pool depth -- 7 feet at the dam.
- 4. Minimum beneficial pool area -- 10 acres.
- 5. Minimum beneficial pool capacity -- 100 acre-feet.
- 6. Maximum beneficial pool capacity -- storage volume equal to 25 inches of runoff from the drainage area.
- 7. Maximum height of dam -- 100 feet.
- 8. Pool area relatively undeveloped -- no housing developments, industrial area, or major highways inundated.

Existing Reservoirs

Existing reservoirs were located using the U.S. Geological Survey (USGS) quadrangle sheets. Two criteria were used to determine sites to be included in this report:

- 1. Surface area -- at least 10 surface acres or a pond identified by name on the USGS topographic map.
- 2. Man-made dam -- natural ponds and beaver dams are excluded.

INVESTIGATIONS AND ANALYSES

Potential Reservoir Sites

Sites were located using the latest available USGS $7\frac{1}{2}$ minute quadrangle sheets. Natural basins, or topography favorable for storage of water, and an undeveloped pool area were the primary considerations in the initial site selection. Watershed boundaries were delineated on the quadrangle sheets and the drainage area was determined for each site. Water storage areas and volumes available upstream of the site centerline were calculated. Data were also obtained to calculate the volume of earthfill required for the dam and any supplementary dikes that might be needed to maintain a reservoir.

At each site a field reconnaissance was made that included an inventory of land and facilities (man-made structures) that would be affected if a dam and reservoir were developed at the site. If it was determined that the reservoir would flood extensive man-made facilities, or a study of the elevation-area storage data showed that the site did not meet criteria for the study, the site was dropped from further consideration.

A surficial geologic investigation was made of each potential site to determine any obvious geologic conditions that might affect the water-holding capability or require extensive foundation preparation. A preliminary geological report was prepared which outlined the types of materials that might be expected at the site and their effect on construction costs and waterholding capabilities for the site. The report of geologic conditions was based on the geologist's interpretation following the surficial investigation of the site and surrounding area. No borings were made and subsurface conditions may vary from those indicated in this report.

Hydrologic and hydraulic data were calculated using methods developed by the Soil Conservation Service. Rainfall data were obtained from Technical Papers 40 and 49, U.S. Department of Commerce, Weather Bureau. Preliminary structure site analyses for several levels of development for each site were processed by computer, using a program which determines the most economical type of principal spillway; determines the runoff and peak flow for the 100-year frequency, 10-day duration, principal spillway design storm; routes the design storm to set the emergency spillway crest; performs other routings to determine the design high water and top of dam elevations; calculates embankment yardage and other construction quantities; determines the total estimated cost of the reservoir; and calculates "safe yield" for water supply purposes.

Existing Reservoirs

An inventory was made of 70 existing reservoirs that cover at least ten acres or are identified by name on the USGS quadrangle sheet, and are formed by a man-made dam. The reservoirs were located using the USGS quadrangle sheets. An engineer made a field reconnaissance to determine the physical condition of each structure and to assess the potential for expansion of the reservoir. While at the site, photographs were taken. Selected photographs are included in this report. Ownership and use information for the reservoirs was obtained from records of the Massachusetts Department of Public Works, the Massachusetts Water Resources Commission and from local interviews.

COSTS

Preliminary cost estimates for potential reservoir sites were based on construction costs and land values as of 1973. The cost estimates include: (1) construction costs; (2) contingencies; (3) engineering and administrative services necessary for surveys, geology, final design, and construction inspection; (4) cost for land required for the reservoir and construction of the dam and spillway; and (5) costs associated with purchase or relocation of man-made facilities affected by the constructed reservoir.

Construction costs were based on recent dam-construction contract costs in Massachusetts. A factor for contingencies, equal to 15% to 35% of the construction cost, was included to account for items that were not considered at this intensity of study. Engineering and administrative services ranged from 20% to 40% of the construction cost.

Costs for land acquisition were based on an evaluation of current real estate transactions and market conditions. Land with potential for development was valued at from \$1,000 to \$10,000 per acre; land with little development potential was valued at from \$200 to \$500 per acre. Land values also varied from site to site based on the proximity to developed areas and highways, development taking place in the area, and suitability for development. Land needed for the dam, spillway, and design high water pool was included in the land acquisition cost.

Cost estimates are presented on the basis of a cost per acre-foot of storage and cost per surface-acre to provide a comparison between different sites and different levels of development at the same site. Costs are based on preliminary estimates; firm cost estimates for any site can be determined only after completion of detailed geologic and engineering investigations, final structural designs, and land appraisals.

No cost estimates are included for existing reservoirs.

REPORT FORMAT

The report is divided into sections based on the eighteen subwatersheds in the study areas. The location map, placed after the Table of Contents, outlines the area covered by each subwatershed. To aid local residents in determining which sites are located in their city or town, the Municipal Index of Sites lists the site identification numbers for potential and existing reservoir sites within each municipality and the page number of this report on which data are recorded.

Each subwatershed section provides site data for the potential and existing reservoir sites located within the subwatershed which are included in this report.

Potential Reservoir Sites

Data for potential reservoirs are presented in the following format:

Location:

includes a narrative description of the location of the site by reference to nearby roads, railroads, or other physical landmarks. In addition, the latitude, longitude, and USGS quadrangle sheet name are provided for more accurate location.

Facilities Affected:

describes any man-made facilities that would be flooded by a reservoir at the potential site. The elevation of existing facilities was estimated during the engineer's field reconnaissance with the aid of the USGS quadrangle sheets.

Geologic Conditions:

provides a summary of the preliminary geologic report. The material in the abutments (the valley sides) and the foundation (the valley floor) is described. An estimate is made of the depth to bedrock and the probable type of rock. The availability of fill material which could be used in the dam construction is noted.

Possible leakage problems are indicated and the waterholding capability of the site is subjectively described as "good," "fair," or "poor." The waterholding capability statement is based on the geologist's interpretation of the surficial conditions observed during the field reconnaissance.

Engineering Notes:

provides information which should be helpful in preliminary design of a dam. One of the abutments is recommended as the location for an excavated emergency spillway. If an excavated emergency spillway is unable to carry the required flows at safe velocity, the need for a concrete emergency spillway is noted.

Public Ownership:

indicates that some portion of a reservoir site is located on land owned by a governmental or quasi-public unit.

Sites which meet study criteria have been analyzed using a computer program which develops preliminary structure site analyses for several levels of beneficial pool. Results of the computer program are presented in the tables entitled "Summary Data for Potential Upstream Reservoir Sites" at the end of each subwatershed section. Two information lines contain data on site drainage area, USGS quadrangle name on which the site is located, latitude and longitude of the site, site rating, stream water quality, and principal spillway design storm runoff and peak flow. The site rating is based on geologic conditions and the expected waterholding capability. Sites are given one of the following ratings:

- 1. Suited for deep permanent storage (over 10 feet in depth).
- 2. Best suited for shallow water storage (3 to 5 feet maximum depth).
- 3. Best suited for temporary storage (e.g., floodwater and sediment storage).

In order to furnish the most data for potential reservoir sites, each site was considered to be suitable for deep permanent storage (rating "l") for purposes of design and analyses. The rating for any site could change based on detailed geologic investigations.

Stream water quality ratings are based on classifications assigned by the Division of Water Pollution Control, Massachusetts Water Resources Commission, and published in "Water Quality Standard," June 1967, and are as follows:

- "Class A -- Waters designated for use as public water supply in accordance with Chapter 111 of the General Laws. Character uniformly excellent.
- "Class B -- Suitable for bathing and recreational purpose including water contact sports. Acceptable for public water supply with appropriate treatment.

 Suitable for agricultural, and certain industrial cooling and process uses; excellent fish and wildlife habitat; excellent aesthetic value.
- "Class C -- Suitable for recreational boating; habitat for wildlife and common food and game fishes indigenous to the region; certain industrial cooling and process uses; under some conditions acceptable for public water supply with appropriate treatment. Suitable for irrigation of crops used for consumption after cooking. Good aesthetic value.
- "Class D -- Suitable for aesthetic enjoyment, power, navigation, and certain industrial cooling and process uses. Class "D" waters will be assigned only where a higher water use class cannot be attained after all appropriate waste treatment methods are utilized."

The Summary Data for Potential Upstream Reservoir Sites tables also contain data for as many as six possible levels of development at each site. Elevations of the beneficial pool, emergency spillway crest, design high water, and top of dam are shown along with pertinent storage volumes, surface areas and depths. Total cost expressed in dollars per acre-foot of storage and dollars per surface-acre are provided to aid in comparison of levels of development. The emergency spillway type which was used in the preliminary design is indicated by an emergency spillway type code explained in the table notes.

These tables are photo-reductions of the computer output sheets. Elevations are shown to the tenth of a foot and costs to the nearest \$10, but are not to be considered that accurate because of the limited investigations made with preliminary data. All the Summary Data Tables are based on preliminary reconnaissance-type investigations and computerproduced structure designs. Additional detailed engineering, geologic and design investigations must be made before final site selection, land acquisition and final design would be practical.

Estimated safe yield for each potential reservoir are also shown on the tables and were based on information extrapolated from data developed by Professor G. R. Higgins, Civil Engineering Department, University of Massachusetts. These estimated safe yields are based on a 95% chance. or the minimum yield that could be expected 19 years out of 20 -- taking into consideration reservoir storage-volume and expected runoff. These data do not consider evaporation, seepage, or prior upstream usage losses.

The Committee on Rainfall and Yield of Drainage Areas of the New England Water Works Association has recommended a figure of 600,000 gallons per day per square mile as a maximum economically feasible safe yield. Data for some of the potential sites in this report show a safe yield above 600,000 gallons per square mile per day. These higher -alues are useful to define the upper portion of a discharge-storage curve for preliminary analysis. For detailed evaluation of a potential site or water supply purposes, the recommendation of the New England Water Works Association should be considered.

Existing Reservoirs

Site data for existing reservoir sites are presented in the following format:

Location:

of the dam is indicated by reference to nearby roads, railroads, or other physical landmarks. The appropriate USGS quadrangle sheet, latitude, and longitude are provided for more accurate location.

Physical data (surface area, height of dam, and drainage area) were estimated from the quadrangle sheet and by field reconnaissance.

Potential for

Expansion: potential is estimated and any major man-made facilities which would be affected by an enlarged reservoir are noted. the site narratives contain the phrase "Significant expansion

does not appear practical." The phrase is used to indicate that although the pool level might be raised by a few feet or the pool area increased by a few acres, any greater expansion does not appear feasible due to topography or facilities which would be flooded.

In some instances, the drainage area of the reservoir does not meet the criteria requiring a 10 to 1 drainage area to pool area ratio, below which there may be relatively high evaporation losses. An increase in reservoir surface area might increase evaporation losses to a point where the reservoir could not be maintained during the summer months. These situations are indicated by the statement "The small drainage area limits expansion potential."

Many of the existing reservoirs have low dams and pond levels which are less than 5 feet above the surrounding ground water elevation. Because of the predominance of sand and gravel in the foundation and abutments of the reservoirs in the study areas, excessive seepage losses may severely limit any significant increase in the depth of the reservoir. If it was apparent that excessive seepage might be a problem, the condition is noted with the statement "Sandy soils may limit expansion potential."

Remarks:

includes a description of the dam and spillway system. Construction materials, spillway type and size, and condition of the structure are noted.

Ownership and Use:

is indicated, if available. In some cases, the pool is not maintained for a specific purpose, but may have incidental use for recreation. This is probably the situation for existing reservoirs which are indicated in the Massachusetts Department of Public Works records as being used to "store water." Typical of these sites are old mill dams which are no longer utilized for mill power.

Selected photographs of existing dams, spillways, and reservoirs are included in the report.

MAPS

Individual subwatershed maps appearing at the end of each section indicate the location of the potential and existing reservoir sites in that subwatershed. The maps are reductions of mosaics prepared from 7½ minute USGS quadrangle sheets (1" = 2000' scale). The quadrangle sheets used and publication dates are listed on the maps. Potential sites are indicated with a red rectangle surrounding the site number. Existing reservoirs are identified by a red circle surrounding the site number.

SOUTH SHORE STUDY AREA SITE DATA FOR

Subwatershed SS-23, Town River

This subwatershed covers about 6200 acres in Braintree, Quincy, and Weymouth in Norfolk County.

The main stream in the subwatershed is the Town River which originates in Braintree and flows northeasterly through Quincy to Town River Bay and the Atlantic Ocean. Elevations range from a high of 360 feet in the Blue Hills Reservation to sea level in Town River Bay.

One existing reservoir was studied.

Site SS-2303(Old Quincy Reservoir)

Location:

On Town Brook about 600 feet upstream from Walnut Street in Braintree, Mass.

Blue Hills, Mass. USGS quadrangle

Latitude: 42°13'03" Longitude: 71°00'55"

Surface Area
(Acres)
45

Height of Dam(Ft.) Drainage Area (Acres)
1.050

Potential for Expansion: Limited by development which surrounds the reservoir. The South Shore Plaza Shopping Center and several industrial buildings would be affected by expansion.

Remarks:

The dam is an earthfill structure. The upstream face of the dam is riprapped with granite blocks. The principal spillway is a granite block riser and a cast iron pipe conduit. The emergency spillway is a granite block weir about 25 feet long and 4 feet deep outletting to a granite-lined channel. Trees are growing on the dam.

Ownership and Use:

The reservoir is owned by the City of Quincy and is used for recreation.



Old Quincy Reservoir (SS-2303)





SOUTH SHORE STUDY AREA SITE DATA FOR

Subwatershed SS-24, Monatiquot River

This subwatershed covers about 18,500 acres in Avon, Braintree, Canton, Holbrook, Milton, Quincy, Randolph, and Stoughton in Norfolk County; and Abington and Brockton in Plymouth County.

Major streams in the subwatershed include the Blue Hill River, Farm River, Cochato River, and the Monatiquot River. The Blue Hill River originates in Milton and flows easterly to form the Farm River downstream of the Braintree Great Pond Reservoir. The Farm River flows southeasterly to South Braintree where it joins the Cochato River to form the Monatiquot. The Monatiquot flows northeasterly to the Weymouth Fore River and the ocean at the Braintree-Weymouth town line. The Cochato River is formed at the confluence of Tumbling and Glovers Brook on the Braintree-Randolph town line. The river flows north to the confluence with the Farm River in South Braintree.

Four existing reservoirs were studied.

Site SS-2409(Upper Reservoir)

Location:

On a tributary to Blue Hill River at the Braintree-Randolph town line. About 80% of the reservoir area is in Randolph.

Blue Hills, Mass. USGS quadrangle

Latitude: 42⁰11'44" Longitude: 71⁰02'31"

Surface Area Height of Drainage Area (Acres) Dam(Ft.) (Acres) Sq. Mi. 1.150 1.80

Potential for Expansion:

Limited. The west edge of the reservoir is a residential area which would be affected by expansion.

Remarks:

The dam is an earthfill structure with granite block riprap on the upstream slope. The principal spillway is a concrete ogee weir with flashboards. There are four spillway bays each about 7 feet wide and 4 feet deep. The outlet channel is about 30 feet wide and has granite block sidewalls. Dam and spillway are in good condition.

Site SS-2409(Upper Reservoir)(Cont.)

Ownership and Use:

The reservoir is owned by the Town of Braintree and is used as a water supply reservoir.

Site SS-2410(Great Pond)

Location:

On a tributary to Blue Hill River about 100 feet upstream from Pond St. in Braintree, Mass. About one-quarter of the pond area is in Randolph.

Blue Hills, Mass. USGS quadrangle

Latitude: 42°12'21" Longitude: 70°02'42"

Surface Area Height of Drainage Area (Acres) Dam(Ft.) (Acres) Sq. Mi. 3.91

Potential for Expansion:

Limited. Upper Reservoir (Site SS-2409) is located immediately upstream. Both ponds are surrounded by residential areas which would be affected by expansion.

Remarks:

The dam is an earthfill structure with riprap on the upstream slope. The spillway is a concrete and steel structure with flashboards to control the pond level. The spillway weir is about 30 feet long and 4 feet deep. Dam and spillway are in good condition.

Ownership and Use:

The pond is owned by the Town of Braintree and is used as a water supply reservoir.

Site SS-2411 (Sunset Lake)

Location:

On a tributary to the Monatiquot River about 500 feet upstream from Pond Street in Braintree, Mass.

Blue Hills, Mass. USGS quadrangle

Latitude: 42°12'05" Longitude: 71°00'58"

Surface Area Height of Drainage Area (Acres) Dam(Ft.) (Acres) 325

Potential for Expansion: The small drainage area limits expansion potential. A great deal of residential development would be affected by expansion.

Remarks:

The dam is a concrete and granite block weir spillway. The spillway consists of two five-foot wide bays with provision for flashboards and a central bay constructed of granite blocks. The downstream stilling basin is filled with silt and debris.

Ownership and Use:

The lake is owned by the Town of Braintree and is used for recreation.

Site SS-2412(Lake Holbrook)

Location:

On Trout Brook at North Shore Road in Holbrook, Mass.

Blue Hills, Mass. USGS quadrangle

Latitude: 42°08'49" Longitude: 71°01'13"

Surface Area Height of Drainage Area (Acres) Dam(Ft.) (Acres) 1,650

Potential for Expansion: Limited by residential development which surrounds the lake.

Remarks:

North Shore Road forms the dam. The principal spillway consists of four concrete drop inlets which outlet under North Shore Road through four concrete culverts. There is also a corrugated metal half-round riser and metal pipe conduit spillway.

Site SS-2412(Lake Holbrook)(Cont.)

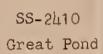
Ownership and Use:

The lake is owned by the Town of Holbrook and is used for recreation.





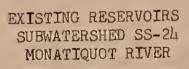
SS-2409



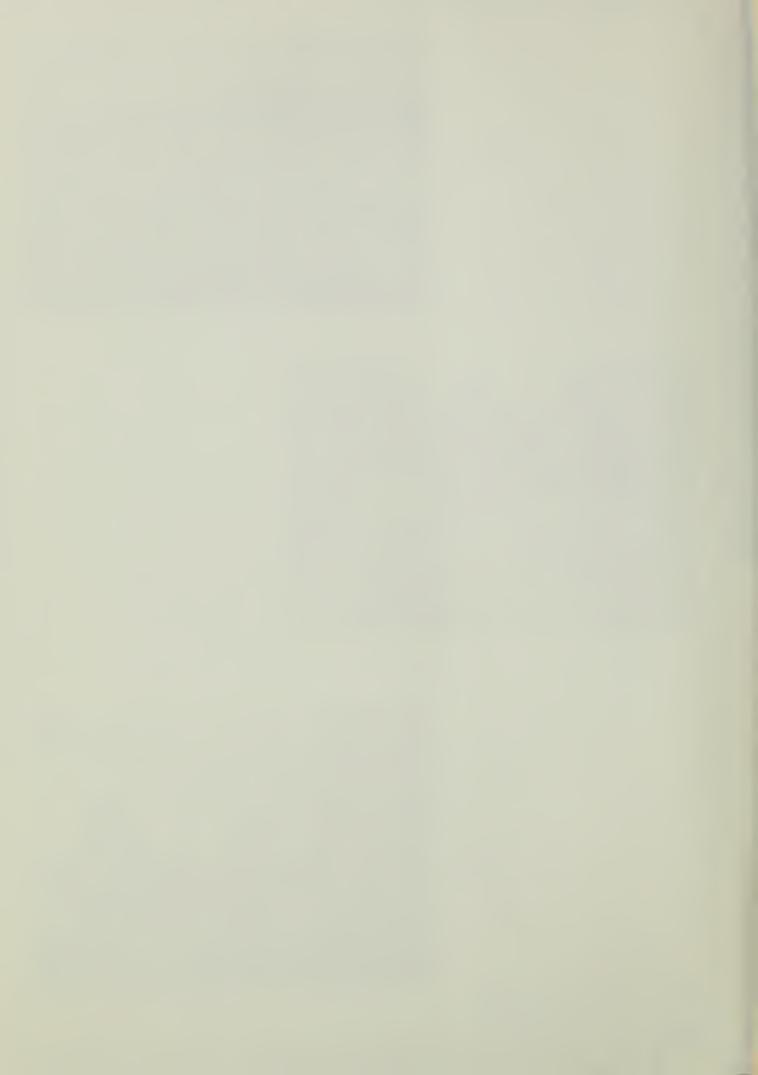




SS-2411 Sunset Lake







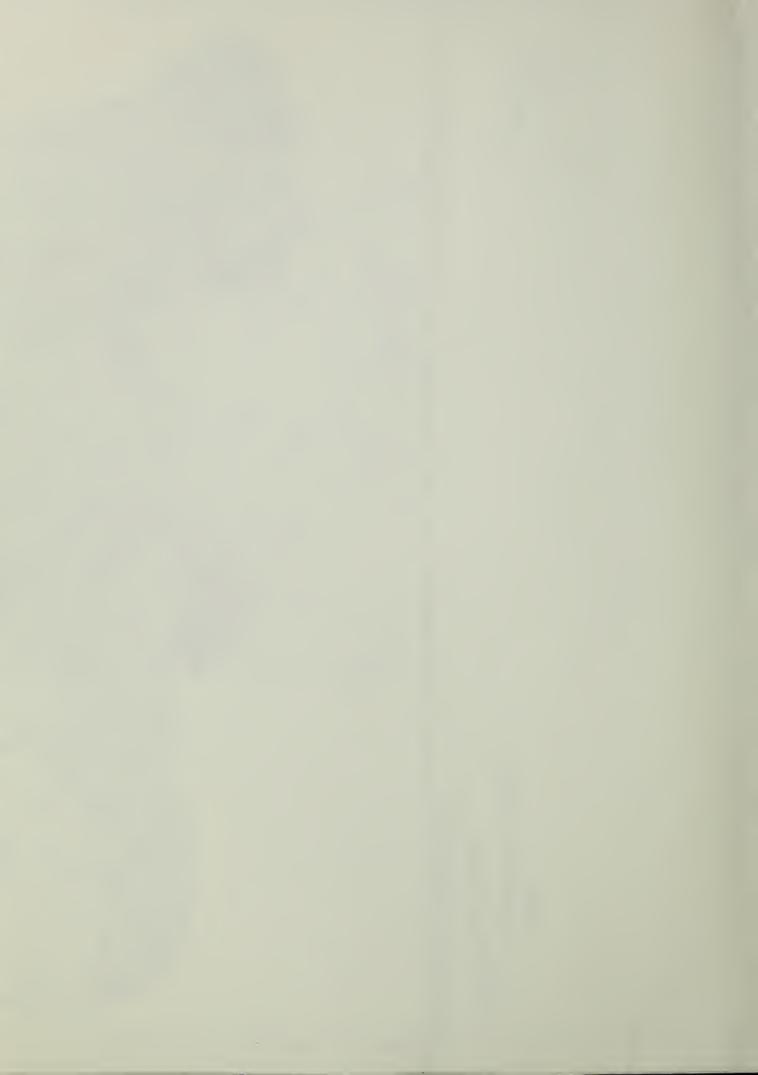


Source-USGS Quad. Sheets Whitman-1963 Weymouth-1958 Brockton-1963 Blue Hills-1958

MONATIQUOT RIVER (SS-24)
SOUTH SHORE STUDY AREA
MASSACHUSETTS

EXISTING AND POTENTIAL RESERVOIR SITES

UNITED STATES DEPARTMENT OF AGRICULTURE 50)L CONSERVATION SERVICE



SOUTH SHORE STUDY AREA SITE DATA FOR

Subwatershed SS-25, Smelt Brook

This subwatershed covers about 1200 acres in Braintree and Weymouth, in Norfolk County.

The main stream in this subwatershed is Smelt Brook which originates in Braintree and flows northerly into Weymouth Fore River, a tidal inlet of Massachusetts Bay. Elevations range from a high of about 220 feet in Braintree to tide water at the confluence with the Weymouth Fore River.

Geology in the subwatershed is characterized by granite bedrock overlain by 5 to 10 feet of englacial drift and glacial till.

This watershed is currently under study as a flood control project by the U.S. Army, Corps of Engineers.

One potential reservoir site was studied.

POTENTIAL SITE SS-2501

Location: On Smelt Brook about 2200 feet upstream from Stetson Street in

Braintree, Mass.

Weymouth, Mass. USGS quadrangle

Latitude: 42⁰12'36" Longitude: 71⁰58'28"

Facilities Facility
Affected: 6 houses 75
Swimming pool 75

Geologic Both abutments are granite bedrock overlain by thin discontinuous Conditions: englacial drift and glacial till. Depth to granite bedrock in the foundation is estimated to be from 5 to 10 feet. Waterholding capabilities appear to be good. Borrow material for dam construction was located near the site.

Engineering The right abutment is recommended for the emergency spillway location. Notes:

The spillway will probably be excavated in bedrock.

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RSHED		COST * PER * AC FT *	* (5)	USGS QUAD-WEYMOUTH, MASS 100-YR PRIN SPWY DESIGN STORM	* 070	2950 *	1880 *	1240 *	* :	
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	EMERGENCY SPILLWAY	AGE REST	NI	YMOUT WY DE			7.8		1	COST
	ENC Y	STORAGE AT CREST	AC FT IN (\$)	AD-WE IN SP	31	24	594	15	1	A AND
1 1	EMERG			USGS QUAD-WEYMOUTH, MASS O-YR PRIN SPWY DESIGN S	66.5 E	65.6 E	67.8 E	0.6 E	1 1 1	CRITERIA AND COST DATA
***************************************		* CREST * ELEV *+ TYPE	* (MSL)	US 100-	• •	•	* *	* 7		GN CR
STUDY AREA-SOUTH SHORE		EPTH AT DAM	(FT)	5 AC (B)	5.9	13.1	15.2	18.2	1	DESI
		COST/ DEPTH SURF AT AC CAM) " (91 ALITY				9410	1 1 1	. C. S.
		COST AREA SURF AC	\$)	MI =		17700	10410	76	1 1 1 1	973 S
	BENEFICIAL POOL	AREA	(AC)	43 SQ M WAT	4	41	00 84	66	1 1	0N 1
TUDY	BENEFICIAL POOL	COST PER AC FT	(\$) (AC) (\$)	DA= 1.43 SQ MI = 915 AC STREAM WATER QUALITY (B)		7300	3700	2040	1 1	BASED
S	EFICI	O G A	2 1	DA			۲. ۶ ۲. ۴	0	1	ARE
1	BEN	STORAGE	I 1.	3	0.0	100			1	COSTS
1		ST	AC F	TE-SS-2501 SITE RATING (1)		10	338	458	1	<u>=</u>
1		ELEV	(MSL) AC FT IN (\$) (AC) (\$) (FT)	SITE-SS-2501 SITE RATING	55.9	63.0	65.3	68.1	1 1	namentamentamentamentamentamentamentamen
	i	•		S					,	Z

EMERGENCY SPILLWAY STORAGE AND COSTS ARE BASED ON TOTAL STORAGE, INCLUDING BENEFICIAL POOL.

EMERGENCY SPILLWAY TYPE CODE- C=CONCRETE CHUTE, D=CONCRETE DROP, E=EXCAVATED, T= TWO SPILLWAYS, N= NONE TABULAR DATA ARE BASED ON PRELIMINARY INFORMATION. FIGURES SHOWN ARE PRIMARILY FOR COMPARISON PURPOSES. ELEVATIONS ARE SHOWN TO THE NEAREST 0.1 FOOT TO SHOW VARIATION BETWEEN DEVELOPMENTS ONLY, AND ARE NOT TO BE 2643

CONSIDERED ACCURATE TO THAT DEGREE.

** DO NOT USE FOR FINAL SITE SELECTION OR LAND ACQUISITION. **

LEGEND

■ ■ WATERSHED BOUNDARY



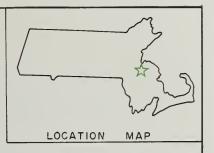
DRAINAGE AREA ABOVE STRUCTURE

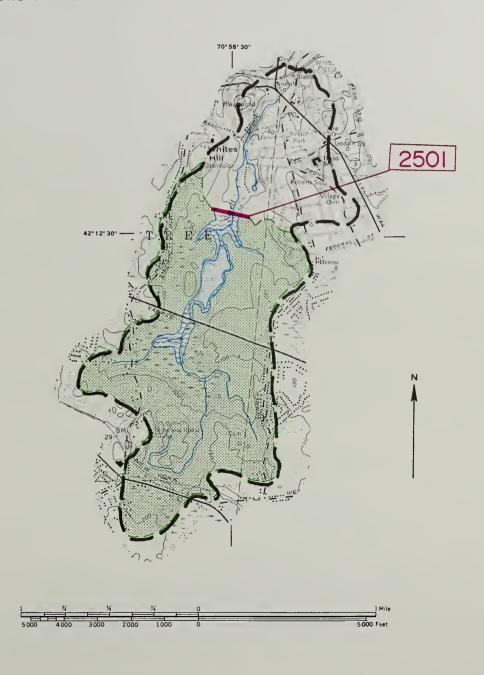


POTENTIAL SITE SHOWING BENEFICIAL POOL FOR LARGEST STRUCTURE



EXISTING POND OR RESERVOIR





SMELT BROOK (SS-25)
SOUTH SHORE STUDY AREA
MASSACHUSETTS
EXISTING AND POTENTIAL RESERVOIR SITES
UNITED STATES DEPARTMENT OF AGRICULTURE
SOIL CONSERVATION SERVICE

Source - USGS Quad. Sheets Weymouth - 1958



SOUTH SHORE STUDY AREA SITE DATA FOR

Subwatershed SS-26, Weir River

This subwatershed covers about 44,400 acres in Braintree, Cohasset, Holbrook and Weymouth in Norfolk County; and Abington, Hingham, Hull, Norwell, Rockland and Situate, in Plymouth County. There are U.S Geological Survey Gaging stations on Bound Brook in Cohasset and Old Swamp River in South Weymouth.

Major streams in this subwatershed include the Weir River, Mill River, Old Swamp Brook and Bound Brook. The Weir River originates in Weymouth as Plymouth River which flows generally northeasterly into Cushing Pond in Hingham, where it emerges as Crook Meadow River, which flows northeasterly to the confluence with Fulling Mill Brook. These streams form the Weir River which continues to flow northeasterly into Massachusetts Bay. The Mill River originates in Weymouth, in Great Pond, and flows generally northeasterly to Weymouth Back River, a tidal inlet to Massachusetts Bay. The Old Swamp River originates in Rockland and flows northwesterly through Hingham to Whitmans Pond in Weymouth. Bound Brook originates in Norwell and flows generally northeasterly through Hingham and Cohasset to the Gulf, a tidal inlet of Massachusetts Bay. Elevations range from a high of about 300 feet in Holbrook to tide water in Massachusetts Bay.

Geology of the subwatershed is characterized by granite bedrock overlain by 10 to 25 feet of outwash sand and gravel or glacial till.

Eight potential reservoir sites and 12 existing reservoirs were studied.

POTENTIAL SITE SS-2601

Location:

On the Old Swamp River about 4700 feet downstream from Forrest Street in Hingham, Mass.

Weymouth, Mass. USGS quadrangle

Latitude: 42°09'44" Longitude: 70⁰55'22"

Facilities

Elevation Facility 3 houses 130 Affected: 130

Geologic Conditions: Both abutments are poorly graded sand and gravel outwash. Depth to granite bedrock in the foundation is estimated to be from 20 to 25 feet. Waterholding capabilities appear to be poor. Leakage is expected through both abutments. Pervious borrow material for dam construction was located near the site; impervious material was not located.

Engineering Notes:

The right abutment is recommended for the excavated emergency spillway location. Part of the left abutment has been removed for sand and gravel.

POTENTIAL SITE SS-2602

Location:

On Accord Brook about 4000 feet downstream from Prospect Street in Hingham, Mass.

Cohasset, Mass. USGS quadrangle

Latitude: 42°12'04" Longitude: 70°51'47"

Facilities Affected:

Facility	Elevation
5 houses	120
2 barns	120
2 sheds	120
Charles Street	120
Road	120
5 houses	115
Swimming pool	115

Geologic Conditions:

The left abutment is poorly graded sand and gravel outwash. The right abutment is poorly graded sand and gravel outwash underlain by granite bedrock. Depth to granite bedrock in the foundation is estimated to be from 15 to 25 feet. Waterholding capabilities appear to be poor. Leakage is expected through the left abutment and the foundation. Pervious borrow material for dam construction was located near the site; impervious material was not located.

Engineering Notes:

The right abutment is recommended for the excavated emergency spillway location. Waterholding capabilities might be improved if a cut-off to bedrock can be made.

POTENTIAL SITE SS-2603

Location:

On Accord Brook about 4400 feet upstream from Triphammer Pond Dam in Hingham, Mass.

Cohasset, Mass. USGS quadrangle

Latitude: 42⁰12'55" Longitude: 70⁰51'27"

Facilities
Affected:

Facility Union Street

Elevation 65

Geologic Conditions:

Both abutments are granitic bedrock with poorly graded sand and gravel at the lower portion of each abutment. Depth to bedrock in the foundation is estimated to be from 10 to 15 feet. Waterholding capabilities appear to be fair. Leakage is expected through the outwash sand and gravel. Pervious borrow material for dam construction was located near the site; impervious material was not located.

Engineering Notes:

The right abutment is recommended for the emergency spillway location. The spillway will probably be excavated in bedrock. Waterholding capabilities might be improved if a cut-off to bedrock can be made.

Location:

On Accord Brook at Triphammer Pond about 2200 feet upstream from Weir River in Hingham, Mass.

Cohasset, Mass. USGS quadrangle

Latitude: 42°13'25" Longitude: 70°52'04"

Facilities Affected:

Facility Leavitt Street Union Street

Elevation 70

Geologic Conditions: The left abutment is poorly graded sand and gravel outwash. right abutment is poorly graded sand and gravel outwash underlain by granite bedrock. Depth to granite bedrock in the foundation is estimated to be from 10 to 20 feet. Waterholding capabilities appear to be fair. Leakage is expected through the left abutment. Pervious borrow material for dam construction was located near the site; impervious material was not located.

Engineering Notes:

The right abutment is recommended for the emergency spillway location. The spillway will probably be excavated in bedrock. Waterholding capabilities might be improved if a cut-off to bedrock can be made.

POTENTIAL SITE SS-2605

Location:

On Bound Brook about 5000 feet upstream from the Cohasset-Hingham town line in Hingham, Mass.

Cohasset, Mass. USGS quadrangle

Latitude: 42⁰11'42" Longitude: 70⁰50'34"

Facilities Affected:

Facility Road

Elevation 90

Geologic Conditions: Both abutments are poorly graded sand and gravel outwash. Depth to granite bedrock in the foundation is estimated to be from 10 to 20 feet. Waterholding capabilities appear to be poor. Leakage is expected through both abutments and the foundation. Pervious borrow material for dam construction was located near the site; impervious material was not located.

Engineering Notes:

The left abutment is recommended for the excavated emergency spillway location. Waterholding capabilities might be improved if a cut-off can be made. The right abutment has been excavated for sand and gravel.

Location: On Bound Brook at Beechwood Street in Cohasset, Mass.

Cohasset, Mass. USGS quadrangle

Latitude: 42°12'35" Longitude: 70°40'25"

Facilities Facility Elevation
Affected: Beechwood Street
Utility poles 55

Geologic The left abutment is granitic bedrock. The right abutment is glacial till underlain by granitic bedrock. Depth to granitic bedrock in the foundation is estimated to be from 5 to 10 feet. Waterholding capabilities appear to be good. Borrow material for dam construction was located near the site.

Engineering The left abutment is recommended for the emergency spillway Notes: location. The spillway will probably be excavated in bedrock.

POTENTIAL SITE SS-2607

Location: On Brass Kettle Brook about 1600 feet upstream from Lily Pond in Cohasset, Mass.

Cohasset, Mass. USGS quadrangle

Latitude: 42°13'25" Longitude: 70°49'21"

Facilities Facility Elevation
Affected: Utility poles 50

Geologic Both abutments are granite bedrock. Depth to granite bedrock in the foundation is estimated to be from 5 to 10 feet.

Waterholding capabilities appear to be good. Borrow material for dam construction was not located near the site.

Engineering The left abutment is recommended for the emergency spillway Notes:

1 location. The spillway will probably be excavated in bedrock.

Location:

On the Plymouth River about 3200 feet upstream from Ward Street in Hingham, Mass.

Weymouth, Mass. USGS quadrangle

Latitude: 42°11'40" Longitude: 70°54'18"

Facilities	<u>Facility</u>	Elevation
Affected:	3 houses	80
	6 houses	75
	2 Commercial buildings	75
	2 sheds	75
	Swimming pool	75
	Cushing Street	70

Geologic Conditions:

Both abutments are poorly graded sand and gravel outwash. Depth to bedrock in the foundation is estimated to be from 5 to 15 feet. Waterholding capabilities appear to be poor. Leakage is expected through both abutments and the foundation. Pervious borrow material for dam construction was located near the site; impervious material was not located.

Engineering Notes:

The left abutment is recommended for the excavated emergency spillway location.

SUMMARY DATA FOR POTENTIAL UPSTREAM RESERVOIR SITES

1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	* SAFE * YIELD	*PERCENT *CHANCE * (MGD)	******** 70-55-22 367 CFS	* 0 . 17 * 0 . 39 * 0 . 55 * 0 . 69	********* 70-51-47 826 CFS	0.18 0.36 0.62 0.97	70-51-27 908 CFS	0.20 0.59 1.17	****
; ; ;		FILL VOL (1000 CY)	LONGITUDE (FLOW =	25 22 33 41 50		27 26 26 29 38	LONGITUDE FLOW =	62 49 74 71	* * * * * * *
1	DAM	HGT FT	i Ž	13 12 15 17	AK	18 17 17 18 20	**************************************	29 27 28 30 30	* * * * *
1	۵	TOP ELEV (MSL)	42-09- 0 IN	126.6 125.8 128.6 130.6	2-12- IN	117.6 117.3 117.1 118.0 120.0	2-12- IN:	78.6 77.1 77.6 79.6 79.9	* * * * * *
RIVER	Z W	AREA (AC)	LATITUDE .	97 * 93 * 106 * 116 * 125 *	**************************************	331 * 313 * 308 * 351 * 455 *	LATITUDE 4.	159 * 149 * 176 * 184 * 184 * *	***
3	i I	ELEV AR	RUNOFF	123.6 122.8 125.6 127.6 127.6	LATITU RUNOFF =	114.6 114.3 114.1 115.0 117.0	RUNOFF	73.8 70.4 72.6 76.0 76.9	***
SUBWATERSHED	* *	COST + PER + AC FT + (\$)	**************************************	1660 * 2180 * 1340 * 1070 * 920 *	HASS GN STORM	1140 * 1300 * 1410 * 1520 * 960 * 960 * 1	MASS GN STORM	710 * 2630 * 1220 * 760 * 680 *	******* DATA.
SUBM	SPILLWAY	ST	* •	4.1 3.4 6.4 8.8 11.3	**************************************	4.1 3.8 7.6 7.8	**************************************	412.50	*******
1		STORAGE AT CREST	D-WEYMC N SPWY	339 279 519 727 934	O-COHAS	757 701 688 850 1441	O-COHA:	974 238 588 1220 1518	******
1	EMERGENCY	- III -	*		*			68.8 E 60.0 E 65.1 E 72.8 E	**************************************
	m	CREST ELEV + TYPE (MSL)	USGS 100-YR	121.1 120.5 123.1 125.3 125.3	.***** USGS 100-YR	112.1 111.8 1111.6 1112.5 114.6	USGS 100-YR	689	*
CRE		DEPTH AT DAM (FT)	979 AC TY (B)	0.8 4.0 6.6 8.8	2189 AC LITY (B)	2.3 5.3 7.1 10.0	816 AC TY (B)	3.8 7.5 12.6 18.2 20.2	等等水油水水油水水水水水水水水水水水水水水水水水水水水水水水水水水水水水水水
STUDY AREA-SOUTH SHORE		COST/ SURF AC (\$)	**************************************	1153C 836C 8410 843C	**************************************	19620 15390 12540 6860	**************************************	17530 10490 8090 7900	****** 73 S.C.
REA-S	7	AREA (AC)	3 SQ 1	12 53 83 93 102	3.42 SQ MI EAM WATER (20 46 63 89 201	O SO	18 36 68 114 131	ON 197
STUDY AREA-SOUTH SHORE	BENEFICIAL POOL	COST COST/ DEPT ELEV STORAGE PER AREA SURF AT AC FT AC DAP (MSL) AC FT IN (\$) (AC) (\$) (FT)	**************************************	6090 2440 1670 1310	**************************************	9100 4740 2700 1890	SITE-SS-2603 DA= 4.40 SQ MI = 2816 A SITE RATING (2) STREAM WATER QUALITY (B	6250 2010 1060 920	本本本本本本本本本本本本本本本本本本本本本本本本本本本本本本本本本本本本
1 1 3 2	BENEFI	AGE In	(3)	8 2 2 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	(3)	10.0	(2)	0.00	***** STS AR
3 3 3 1		STORAGE AC FT I	**************************************	100 284 469 653	**************************************	100 205 415 730	-2603 RATING	0 100 357 871 1128	(1) CO
3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3		ELEV (MSL)	**************************************	114.8 118.0 120.6 122.8 124.6	**************************************	102.3 105.3 107.1 110.0	SITE-SS-260 SITE RATI	53.8 57.5 62.5 68.1 70.3	NOTES -

EMERGENCY SPILLWAY STORAGE AND COSTS ARE BASED ON TOTAL STORAGE, INCLUDING BENEFICIAL POOL.

EMERGENCY SPILLWAY STORAGE AND COSTS ARE BASED ON TOTAL STORAGE, INCLUDING BENEFICIAL POOL.

EMERGENCY SPILLWAY TYPE CODE— C=CONCRETE CHUTE, D=CONCRETE DROP, E=EXCAVATED, T** TWO SPILLWAYS, N= NONE

TABULAR DATA ARE BASED ON PRELIMINARY INFORMATION. FIGURES SHOWN ARE PRIMARILY FOR COMPARISON PURPOSES.

ELEVATIONS ARE SHOWN TO THE NEAREST 0.1 FGOT TO SHOW VARIATION BETWEEN DEVELOPMENTS ONLY, AND ARE NOT TO BE
CONSIDERED ACCURATE TO THAT DEGREE.

** DO NOT USE FOR FINAL SITE SELECTION OR LAND ACQUISITION. **

SUMMARY CATA FOR POTENTIAL UPSTREAM RESERVOIR SITES

*	E E E	SE .	2-04 CFS * 20 20 20	### 0-34 CFS	114 221 33 37	0-25 CFS **	54 81 03	# B #
	SAFE	* PERCENT *CHANGE * (MGD)	70-52-04 1021 CFS 0.20 0.87 1.75	**** 70-5 232	* 0000	70-40	00-	NONE SES.
		FILL VOL (1000 CY)	FLOW = 1	**************************************	_	FLOW = **	31.	**** PURPO ARE N
1	Σ	1	EAK LOI 242 452 455			# _	22 22 22	POOL. PILLWAPARISOILY, ANI
	DA	TOP ELEV (MSL)	42-13-2 0 IN, P 67.1 52.0 66.1	03.0 ******* 42-11-42	108 108 108 111 117 117	* C + C + C + C + C + C + C + C + C + C	68.5	**************************************
~	* *	REA *	* Ш * * * * *	* * * U .	221 * * * * * * * * * * * * * * * * * *	TUDE 6.	130 * 141 * 149 *	BENER D. Ta ILY F LOPME
RIVER	DESIGN GH WATER	AR				A .		ING VATE VATE DEVE DEVE
WEIR	DES HIGH	ELEV (MSL)	RUN OF F 60.0 45.9 59.5	**************************************	105. 105. 108. 114.	**************************************	64.	GE, INCLUDING OP, E=EXCAVATE OWN ARE PRIMAE N BETWEEN DEVE
SUBMATERSHED WEIR RIVER		COST PER AC F	+ 10 10 0 0 + 1	* 5	w 4 w 0 H	4ASS 5N STORM 620 *	1040 * 800 * * 690 * *	TA. DRAGE, DROP, SHOWN TION BE
SUBW/		_ Z	SET 9 1 0 0 8 0 0 8 7 3 3 7 3 3	**** SET*	1000%	* B B B B B B B B B B B B B B B B B B B	7000	* * * * CRECREGUR VAR
		STORAGE AT CREST	QUAD-CCHAS QUAD-CCHAS PRIN SPWY 1 E 1096 1 E 200 1 E 775 1 E 1936	C C 4 0 1 0 1 0 1 0 1 0 1 0 1 0 1 0 1 0 1 0	153 147 220 369 1 492 1	* 4 A 6 6	549 791 036	* * N N N N N N N N N N N N N N N N N N
	MERGENCY	N A	* 3 ¢ m m m m ′	# E E E E E E E E E E E E E E E E E E E		PRIN SE		**** RIA A SED 0 UTE; MATIO T TO
	: U :	CRES CRES ELEV + TYP (MSL	4 K K K K K K K K K K K K K K K K K K K	* * * * * * * * * * * * * * * * * * *		GS **	57.4	**************************************
ORE		I .	168 AC TY (B) 4 · 8 * 22 · 7 *			3C91 AC 3C91 AC ALITY (B) * 1.7 *		S. DESIGN ND COSTS C = C = C = C ONCR ELIMINARY NEAREST O DEGREE.
JTH SH		COST/ SURF AC (\$)	1 = 3 0UALI 2655C 1859C 1013C	113c	3994c 3508c 3192c 2507c	****** QUALI 759C	7700 7200 7140	S.C. S.C. AGE A E CODE ON PR THE THAT
EA-SUL		AREA (AC)	++++ SQ MJ NATER 16 23 61 144	SQ MI	15 19 26 36	SQ M SQ M WATER 39	74 88 100	**************************************
STUDY AREA-SOUTH SHORE	BENEFICIAL POOL	COST COST COST/ DEPY COST/ DEPY COST/ DEPY COST/ DEPY COST/ DEPY COST/ COST/ DEPY COST/ COST/ DEPY COST/ COST/ DEPY COST/ COST	*	**************************************	5950 4140 2860 2350	SITE-SS-2606 DA= 4.83 SQ MI = 3C91 A SITE RAIING (1) STREAM WATER QUALITY (8 49.7 0 0.0 39 10 56 759C 3.51.8	1850 1230 990	**************************************
1	BENEFI	AGE IN	(2) 00.0 0.4 2.0 5.6	(3)	0.0 2.7 4.4 7.8 10.3	(1)	1.2 2.0 2.8	STS AR ERGENC ERGENC BULAR EVATIONS IDER
1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1		STORAGE AC FT I	TE-SS-2604 SITE RATING 28.7 0 33.5 100 46.7 567 56.4 1502	######################################	100 163 289 384	TE-SS-2606 SITE RATING 49.7 0 51.8 100	308 517 725	(1) COS (2) EMB (3) EMB (4) TAB (5) ELE (5) COV
1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1		ELEV (MSL)	SITE - S - 2604 SITE RATIN 28.7 33.5 100 46.7 56.4 150	**************************************	88.1 100.1 103.8 109.3	SITE-SS-2606 SITE RATIN 49.7 51.8	54.9 57.5 59.6	* I * U * U * U * U

SUMMARY DATA FOR POTENTIAL UPSTREAM RESERVOIR SITES

	SAFE YIELD	PERCENT CHANCE	(MCD)	LONGITUDE 70-49-21 FLOW = 335 CFS	***	0.17	0.25	0.37	0.42	1	70-54-18	761 CFS	**	0.18	0.45	0.66	0.85		*****
1	* * *		* 1	E 70	*	*	*	*	*	# 1	-		* *	*	*	*	*	*	***
		FILL VOL (1000		NGITUD LOW =	12	12	13	14	14	1 1 1	LONGITUDE	LOW =	128	121	139	145	145		*****
1	рам	НСТ	- 4		15	15	15	16	16	1	je k	¥	21	20	22	22	22		*****
1	۵	TOP ELEV	(MSL)	LATITUDE 42-13-25 LONGI FF = 6.00 IN, PEAK FLOW	58.9	58.8	59.5	0.09	0.09	1	LATITUDE 42-11-40	6.20 IN, PEAK FLOW	78.6	78.3	79.6	80.0	80.0		******
RIVER	N TER +	AREA	. AC		139 *	141 *	153 *	166 *	168 *	*	TITUDE		153 *	147 *	164 *	171 *	180 *	*	******
WEIR RI	DESIGN HIGH WATER	ELEV		LATITU RUNOFF =	55.4	55.5	56.3	57.0	57.1		4	RUNDFF =	75.6	75.3	76.6	77.1	17.9		
SUBWATERSHED		COST * PER *	. (4)	USGS QUAD-COHASSET, MASS	780 *	1160 *	* 068	720 *	720 *	*	IASS	DESIGN STORM	1400 *	1570 *	1440 *	1470 *	1440 *	up t 1	*****
SUBMA	ILLWAY	ST	2 * * * * * * * * * * * * * * * * * * *	SSET, M	4.1	3.3	4.6	7.1	7.8		OUTH. P	DESIG	5.4	5.1	6.1	6.5	7.1	1	****
*	EMERGENCY SPILLWAY	STORAGE AT CREST		-COHA	323	255	366	557	615		QUAD-WEYMOUTH, MASS	A SPWY	948	968	1062	1146	1250	1	***
*	MERGEN		*	S QUAC	.4 E	51.5 E	8 E	•5 E	•0 E		t		•3 E	72.8 E	.1 E	74.8 E	•5 E	1	
	ш	CREST ELEV + TYPE	1021	USG 100-Y	52	51	1 52	1 54	55		USGS	100-YR	73	1 72	14	14	15		
ORE		DEPTH AT DAM	*******	934 AC TY (B)	1.4	5.1 #	6.3 #	8.1 #	8.5 #		106 AC	1Y (B)	1.6	4.3 *	7.6 *	10.3 #	12.5	1	
STUDY AREA-SOUTH SHORE		COST/ SURF AC	*****	MI = R QUALI		9169	584C	477C	488C		MI = 2	R QUALI		31650	25130	2294C	20560	1 1 1	
AREA-S	ار	AREA	*****	46 SQ M WATE	12	43	26	84	91	1	29 50	M WATE	56	44	61	73	88	1 1	
STUDY	BENEFICIAL POOL	COST PER AC FT	***********	DA= 1.46 SQ MI = 934 STREAM WATER QUALITY (2970	2020	1410	1380	1	DA= 3.	STREAM WATER QUALITY (14080	5550	3740	2880	3 3 3	
*	BENEFI	AGE	****	3	0.0	1.2	2.0	3.5	4.1	***			0.0	9.0	1.6	2.5	3.5	1	
***************************************		STORAGE	****	TE-SS-2607 SITE RATING (1)	0	100	191	284	323	***************************************	2609	SITE RATING (3)	0	100	275	450	629		
STUDY AREA-SOUTH SHORE STUDY AREA-SOUTH SHORE	BENEFICIAL POOL	ELEV STO	NT -L コズ (JOE)	SITE-SS-2607 SITE RATIN	45.4	49.0	50.3	52.0	55.5	计算 化 	SITE-55-2609	SITE R	59.5	62.3	9.59	68.3	10.5		医医医医氏性医医医氏性医检检检检检检检检检检检检检检检检检检检检检检检检检检检

NOTES - (1) COSTS ARE BASED ON 1973 S.C.S. DESIGN CRITERIA AND COST DATA.

(2) EMERGENCY SPILLWAY STORAGE AND COSTS ARE BASED ON TOTAL STORAGE, INCLUDING BENEFICIAL POOL.

(3) EMERGENCY SPILLWAY TYPE CODE- C=CONCRETE CHUTE, D=CONCRETE DROP, E=EXCAVATED, T = TWO SPILLWAYS, N = NONE

(4) TABULAR DATA ARE BASED ON PRELIMINARY INFORMATION. FIGURES SHOWN ARE PRIMARILY FOR COMPARISON PURPOSES.

(5) ELEVATIONS ARE SHOWN TO THE NEAREST 0.1 FOOT TO SHOW VARIATION BETWEEN DEVELOPMENTS ONLY, AND ARE NOT TO BE

CONSIDERED ACCURATE TO THAT DEGREE.

** DO NOT USE FOR FINAL SITE SELECTION OR LAND ACQUISITION. **

Existing Site SS-2604(Triphammer Pond)

Location:

On Accord Brook about 2500 feet upstream from the Weir River in Hingham, Mass.

Cohasset, Mass. USGS quadrangle

Latitude: 42°13'25" Longitude: 70°52'02"

Surface Area Height of Drainage Area

(Acres) Dam(Ft.) (Acres) Sq. Mi.

8 3.150 L.92

Potential for Expansion: Please refer to Site Data and Design Summary Table for Potential Site SS-2604.

Remarks:

The dam is an earthfill structure with some riprap on the upstream slope. The principal spillway is a concrete fish ladder. There is also a stone masonry flume which may have been used for mill power. Concrete in the fish ladder is cracked, spalled, and broken. The dam has been overtopped near the right side. Large willow trees are growing on the dam.

Ownership and Use:

The pond is owned by the town of Hingham and is used for recreation.

Existing Site SS-2610(Foundry Pond)

Location:

On the Weir River about 1500 feet downstream from Route 3-A in Hingham, Mass.

Cohasset, Mass. USGS quadrangle

Latitude: 42°14'49" Longitude: 70°51'41"

Surface Area Height of Drainage Area (Acres) Dam(Ft.) (Acres) Sq. Mi. 9,425

Potential for Expansion: Limited by the Penn Central Railroad located immediately upstream, Route 3A, and several local streets.

Remarks:

The dam is an earthfill structure with stone masonry walls on the upstream and downstream faces. The principal spillway is a stone masonry and concrete drop structure about 75 feet long and 1 foot deep. There is also a concrete fish ladder and a stone box outlet about 5 feet by 1.5 feet.

Ownership and Use:

The pond is owned by the town of Hingham and is used for recreation.

Existing Site SS-2612(Fulling Mill Pond)

Location:

On Fulling Mill Brook just upstream from South Pleasant Street in Hingham, Mass.

Weymouth, Mass. USGS quadrangle

Latitude: 42°12'15" Longitude: 70°52'36"

Surface Area Height of Drainage Area (Acres)

13

Height of Drainage Area (Acres)

9

150

Sq. Mi.

Potential for Expansion: The small drainage area limits expansion potential. A pumping station would be affected by expansion.

Remarks:

The dam is an earthfill structure with South Pleasant Street at the downstream toe. The upstream face is a low stone wall. The spillway is a capped, stone masonry, drop inlet and a stone box culvert. There has been some wave ercsion at the upstream face of the dam.

Ownership and Use:

The pond is owned by the Hingham Water Company and is used for water supply.

Existing Site SS-2613(Cushing Pond)

Location:

On the Crooked Meadow River about 2500 feet upstream from Main Street in Hingham, Mass.

Weymouth, Mass. USGS quadrangle

Latitude: 42⁰12'31" Longitude: 70⁰53'16"

Surface Area Height of Drainage Area

(Acres) Jam(Ft.) (Acres) Sq. Mi.

3,250 5.08

Potential for Expansion: Limited by a housing development along the north shore of the pond.

Remarks:

The dam is an earthfill structure. The principal spillway is a three-bay concrete drop structure with provision for stop-logs. There is also a concrete structure which outlets into a channel to s mill building. The channel was recently rebuilt.

Ownership and Use:

The pond is owned by Charles and Alma Clemens and is used for wildlife and recreation.

Existing Site SS-2614 (Brewer Pond)

Location:

On an unnamed tributary to the Fresh River about 900 feet north of Hobart Street in Hingham, Mass.

Weymouth, Mass. USGS quadrangle

Latitude: 42°13'28" Longitude: 70°54'11"

Dam (Ft.)

Surface Area Height of Drainage Area (Acres) Dam (Ft.) (Acres)

Potential

The small drainage area limits expansion potential.

for

Expansion:

Remarks:

The dam is an earthfill structure. The spillway is concrete capped, stone masonry drop structure with flashboards.

Ownership and

The pond is owned by the Brewer Estate and is not used for a specific purpose.

Use:

Existing Site SS-2615 (Cranberry Pond)

Location:

On an unnamed tributary to the Weymouth Fore River about 1,700 feet downstream from Broad Street in Weymouth, Mass.

Weymouth, Mass. USGS quadrangle

Latitude: 42°13'18" Longitude: 70°57'28"

Surface Area Height of Drainage Area
(Acres) Dam (Ft.) (Acres)

5 250

Potential for

Expansion:

Limited. A residential area along the west edge of the pond would be affected. The small drainage area limits expansion potential.

Remarks:

The dam is an earthfill structure with old riprap on the upstream slope. the spillway is a concrete retaining wall with an 8-foot wide opening with provision for flashboards. Water has flowed around both ends of the wall. Brush and trees are growing on the dam.

Ownership and Use:

The pond is owned by the town of Weymouth and used for conservation purposes.

Existing Site SS-2616 (Whitman's Pond)

Location:

On Mill River between Lake Street and Pleasant Street in Weymouth, Mass.

Weymouth, Mass. USGS quadrangle

Latitude: 42⁰12'39"

Longitude: 70°55'48"

Surface Area Height of (Acres)

175

Dam (Ft.)

Drainage Area (Acres) 8,400

Potential for

Limited. Most of the shoreline is lined with residential development.

Expansion:

Remarks:

The dam is an earthfill structure with a stone wall along the downstream face. The spillway is a concrete weir with four 5-foot wide bays. There is another spillway located near the left abutment, as well as a concrete fish ladder. It appears that the dam was recently reconstructed. The dam and spillways are in good condition.

Ownership and

The pond is owned by the town of Weymouth and is used as a supplemental water supply.

Use:

Existing Site SS-2617 (Elias Pond)

Location:

On an unnamed tributary to Weymouth Back River about 1,600 feet downstream from Diersch Street in Weymouth, Mass.

Weymouth, Mass. USGS quadrangle

Latitude 42^o12'37''

Longitude: 70^o55'26"

Surface Area (Acres)

Height of Dam (Ft.)

Drainage Area (Acres) 450

Potential for

Limited. Fairmount Cemetery and several streets would be affected.

Expansion:

Remarks:

The dam is an earthfill structure with riprap on the upstream face. The spillway is a granite block weir about 4 feet wide with provision for flashboards. The weir outlets to a series of granite block steps.

Ownership and

The pond is owned by East Weymouth Wool Company and is used for recreation.

Use:

Existing Site SS-2618 (Great Pond)

Location: On Mill River about 50 feet upstream from Hollis Street in Weymouth, Mass.

Weymouth, Mass. USGS quadrangle

Latitude: 42°10'10" Longitude: 70°58'01"

Surface Area Height Drainage Area
(Acres) Dam (Ft.) (Acres) (Sq. Miles)

300 8 1,800 2.81

Potential Limited. An area of shallow water would be created by expansion. for The pond is already large in relation to its drainage area. Expansion:

Remarks: The dam is an earthfill structure. The principal spillway is a concrete weir with flashboards. Adjacent to the principal spillway is another weir which serves as the emergency spillway. The dam and spillways are in good condition.

Ownership The pond is owned by the town of Weymouth and is used for water supply.

Existing Site SS-2619 (Accord Pond)

Location: On Accord Brook about 500 feet upstream from Route 53 in Hingham,
Mass. About one-quarter of the pond area is in Norwell and
Rockland.

Weymouth, Mass. USGS quadrangle

Latitude: 42°10'27" Longitude: 70°53'24"

Surface Area Height of Drainage Area
(Acres) Dam (Ft.) (Acres) (Sq. Miles)

8 370 0.58

Potential Limited. The pond is already large in relation to its drainage for area.

Expansion:

Remarks: The dam is an earthfill structure. Most of the upstream slope is riprapped. The spillway is a capped, stone masonry drop structure. The downstream channel is lined with stone. Small trees are growing on the dam. Seepage was noted along the downstream toe of the dam and the adjoining esker.

Ownership The pond is owned by Hingham Water Company and is used for and water supply.

Use:

Existing Site SS-2620 (Bound Brook Pond)

On Bound Brook at Mount Hope Street in Norwell, Mass. Location:

Cohasset, Mass. USGS quadrangle

Latitude: 42°11'35" Longitude: 70°50'03"

Surface Area Height of Drainage Area (Acres) Dam (Ft.) (Acres) (Sq. Miles) 13 1,300 2.03

for Expansion:

Potential Raising the pond level by 10 feet would create a 300 acre pool. Most of the water would be quite shallow.

Remarks:

Mount Hope Street forms the dam. The spillway is a four-bay concrete structure with flashboard control. Each bay is about 5 feet wide and 5 feed deep. Water passes under Mount Hope Street through three 60-inch corrugated metal pipes. The spillway is in good condition and appears to be recently reconstructed.

Ownership and

The pond is privately owned and is not used for a specific purpose.

Use:

Existing Site SS-2621 (Sanctuary Pond)

Location: On James Brook about 3,600 feet upstream from Sohier Street in Cohasset, Mass.

Cohasset, Mass. USGS quadrangle

Latitude: 42^o14'28"

Longitude: 70°49'28"

Surface Area Height of Drainage Area
(Acres) Dam (Ft.) (Acres) (Sq. Miles)

5 0.47

Potential - for

Limited. Expansion would affect a railroad, Main Street, and Route 3A.

Expansion:

Remarks:

The dam is an earthfill structure. The spillway inlet is a one-foot high by three-foot wide orifice in a concrete wall. The pond outlet is covered with stone rubble and is not visible. Trees and brush are growing on the dam. Seepage was noted along the downstream toe of the dam.

Ownership and

The pond is owned by Reservation Trustees and is used for wildlife habitat.

Use:



SS-2610 Foundry Pond



SS-2616 Whitmans Pond



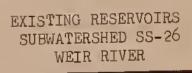
SS-2612 Fulling Pond



SS-2617 Elias Pond



SS-2615 Cranberry Pond

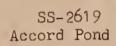








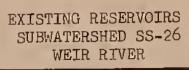
SS-2618 Great Pond





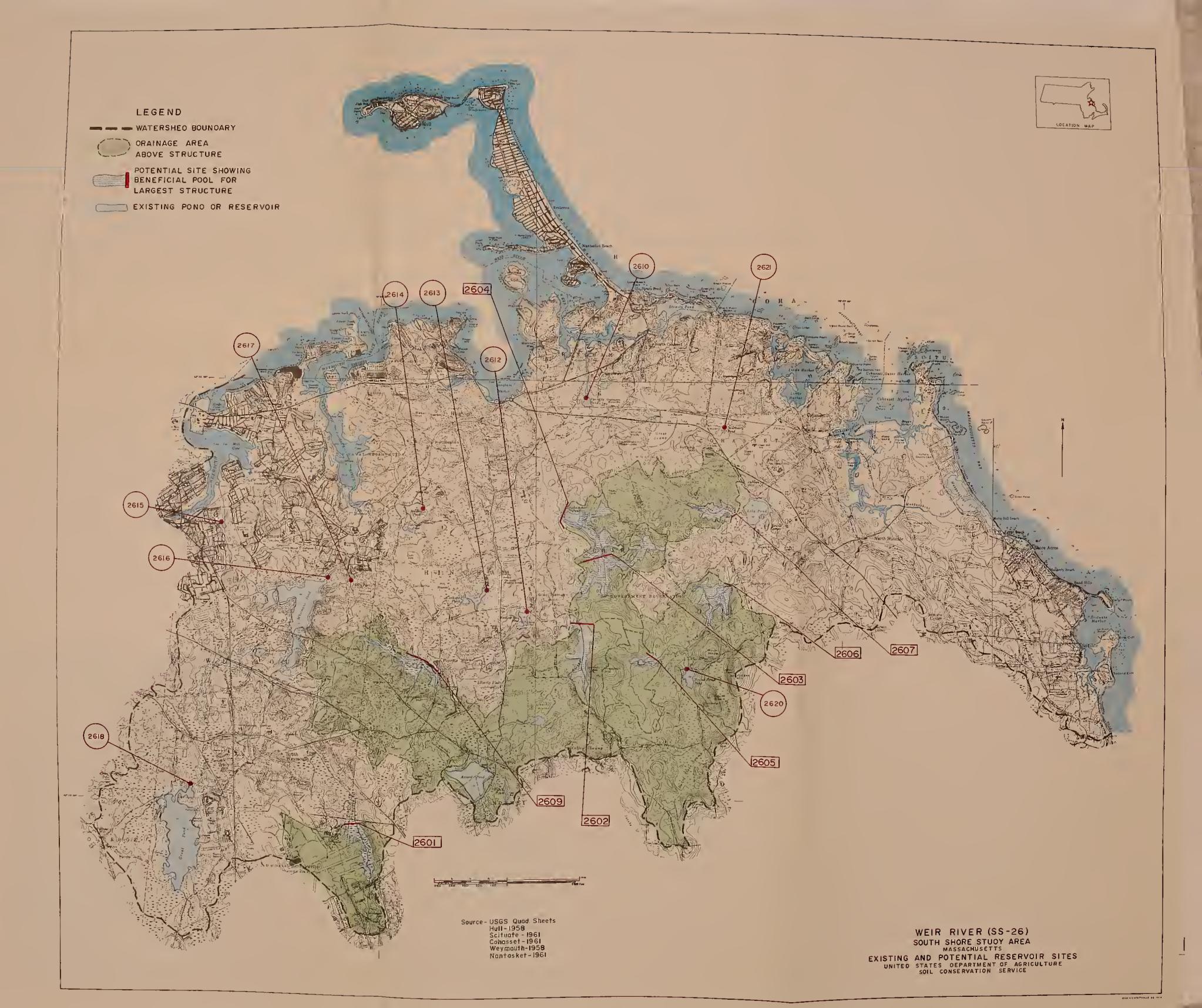


SS-2620 Bound Brook Pond











SOUTH SHORE STUDY AREA SITE DATA FOR

Subwatershed SS-27, North River

This subwatershed covers about 52,000 acres in Abington, Duxbury, Hanover, Hanson, Marshfield, Norwell, Pembrooke, Rockland, Situate and Whitman, in Plymouth County and Weymouth, in Norfolk County. There is a U.S. Geological Survey Stream gaging station on Indian Head Brook in Hanover.

The main stream in this subwatershed is the North River which begins at the confluence of Third Herring Brook and Indian Head River in Hanover, and flows generally northeasterly along the Norwell, Marshfield and Situate town boundaries to Massachusetts Bay. Third Herring Brook originates in Norwell and flows southeasterly along the Norwell-Hanover town boundaries to the confluence with the North River. Indian Head River originates in Factory Pond in Hanson and flows generally easterly along the Hanover, Hanson and Pembroke town boundaries to the confluence with the North River.

Elevations range from a high of about 210 feet in Marshfield to tide water in Massachusetts Bay.

Geology in this subwatershed is characterized by granite bedrock overlain by 10 to 50 feet of outwash sand and gravel or glacial till.

Ten potential reservoir sites and 21 existing reservoirs were studied.

POTENTIAL SITE SS-2701

Location:

On Second Herring Brook at Turner Pond about 1800 feet downstream from Cross Street in Norwell, Mass.

Cohasset, Mass. USGS quadrangle

Latitude: 42⁰10'21" Longitude: 70⁰47'14"

Facilities	Facility	Elevation
Affected:	2 houses	80
	3 houses	75
	Garage	75
	Cross Street	75

Geologic Conditions:

The right abutment is poorly graded sand and gravel outwash. The left abutment is poorly graded sand and gravel with cobbles and boulders underlain by bedrock. Depth to bedrock in the foundation is estimated to be from 15 to 20 feet. Waterholding capabilities appear to be poor. Leakage is expected through both abutments. Borrow material for dam construction was located near the site.

Engineering Notes:

The left abutment is recommended for the excavated emergency spillway location.

Location:

On First Herring Brook at the Norwell-Scituate town line in Norwell, Mass.

Cohasset, Mass. USGS quadrangle

Latitude: 42⁰11'06" Longitude: 70⁰47'02"

Facilities	Facility	Elevation
Affected:	3 houses	90
	Shed	90
	Old Oaken Bucket Road	90
	House	85
	Shed	85
	3 houses	80
	4 sheds	80
	Utility poles	75

Geologic Conditions: Both abutments are poorly graded sand and gravel with cobbles and boulders. Depth to bedrock in the foundation is estimated to be from 10 to 20 feet. Waterholding capabilities appear to be fair. Leakage is expected through both abutments. Borrow material for dam construction was located near the site.

Engineering Notes:

The left abutment is recommended for the excavated emergency spillway location. Waterholding capabilities might be improved if a cut-off can be made.

POTENTIAL SITE SS-2705

Location:

On an unnamed tributary to Second Herring Brook about 400 feet upstream from Norwell Avenue in Norwell, Mass.

Cohasset, Mass. USGS quadrangle

Latitude: 42°10'29" Longitude: 70°48'43"

Facility	Elevation
6 houses	130
3 sheds	130
Lincoln Street	130
Mt. Blue Street	130
Shed	125
	6 houses 3 sheds Lincoln Street Mt. Blue Street

Geologic Conditions:

Both abutments are poorly graded coarse sand and gravel underlain by bedrock. About 50% of the right abutment is covered with boulders. Depth to granite bedrock in the foundation is estimated to be from 15 to 20 feet. Waterholding capabilities appear to be fair. Leakage is expected through both abutments. Pervious borrow material for dam construction was located near the site; impervious material was not located.

POTENTIAL SITE SS-2705 (continued)

Engineering Notes:

The left abutment is recommended for the excavated emergency spillway location. Waterholding capabilities might be improved if a cut-off can be made.

POTENTIAL SITE SS-2706

Location:

On Wildcat Creek about 500 feet upstream from Pleasant Street in Norwell, Mass.

Cohasset, Mass. USGS quadrangle

Latitude: $12^{\circ}08^{\circ}52^{\circ}$ Lon

Longitude: 70°49'30"

Facilities Affected:

Utility poles

Elevation

Geologic Conditions:

Both abutments are poorly graded coarse sand and gravel. Depth to bedrock in the foundation is estimated to be from 15 to 20 feet. Waterholding capabilities appear to be poor. Leakage is expected through both abutments. Pervious borrow material for dam construction was located near the site; impervious material was not located.

Engineering Notes:

The right abutment is recommended for the excavated emergency spillway location. Waterholding capabilities might be improved if a cut-off can be made.

Location:

On Cove Brook about 400 feet upstream from Highland Street in Marshfield, Mass.

Cohasset, Mass. USGS quadrangle

Latitude: 42°08'37" Longitude: 70°45'41"

Facilities Affected:

Facility	Elevation
4 houses	60
Garage	60
Union Street	60 .

Geologic Conditions:

The left abutment is poorly graded sand and gravel outwash at low elevations and glacial till at higher elevations. The right abutment is glacial till underlain by bedrock. Depth to granite bedrock in the foundation is estimated to be from 15 to 20 feet. Waterholding capabilities appear to be fair. Leakage is expected low on the left abutment. Borrow material for dam construction was located near the site.

Engineering Notes:

The right abutment is recommended for the excavated emergency spillway location. Waterholding capabilities might be improved if a cut-off through the sand and gravel on the left abutment can be made.

POTENTIAL SITE SS-2713

Location:

On an unnamed tributary to Drinkwater River about 200 feet upstream from King Street in Hanover, Mass.

Whitman, Mass. USGS quadrangle

Latitude: 42°05'13" Longitude: 70°52'44"

Facilities Affected:

Facility	Elevation
9 houses	75
Swimming pool	75
Utility poles	75

Geologic Conditions: Both abutments are poorly graded sand and gravel. Depth to bedrock in the foundation is estimated to be from 30 to 40 feet. Waterholding capabilities appear to be poor. Leakage is expected through both abutments and the foundation. Pervious borrow material for dam construction was located near the site; impervious material was not located.

Engineering Notes:

Preliminary design information indicates that a concrete monolithic conduit emergency spillway may be needed at this site. The left abutment has been excavated for sand and gravel.

Location:

On Herring Brook about 800 feet downstream from Mountain Avenue in Pembroke, Mass.

Hanover, Mass. USGS quadrangle

Latitude: 42°03'51" Longitude: 70°48'09"

Facilities	Fac:	ility	Elevation
Affected:	15 houses	S	55
	2 sheds		55
	Hobomock	Street	55
	2 houses		50
	Shed		50
	House		45
	House		40
	Mountain	Avenue	J ₁ O

Geologic Conditions: Both abutments are poorly graded sand and gravel outwash. Depth to bedrock in the foundation is estimated to be from 40 to 50 feet. Waterholding capabilities appear to be poor. Leakage is expected through both abutments and the foundation. Pervious borrow material for dam construction was located near the site; impervious material was not located.

Engineering Notes:

The right abutment is recommended for the excavated emergency spillway location.

POTENTIAL SITE SS-2715

Location:

On Huldah Brook about 3000 feet downstream from Route 3 in Pembroke, Mass.

Hanover, Mass. USGS quadrangle

Latitude: 42°05'32" Longitude: 70°45'35"

Elevation 55 Facilities Facility Affected: House Shed

Geologic Conditions: Both abutments are poorly graded sand and gravel outwash. Depth to bedrock in the foundation is estimated to be from 40 to 50 feet. Waterholding capabilities appear to be poor to fair. Leakage is expected through both abutments and the foundation. Borrow material for dam construction was located near the site.

Engineering Notes:

The right abutment is recommended for the excavated emergency spillway location.

Location:

On Pudding Brook at the Reservoir about 4300 feet downstream from Pleasant Street in Pembroke, Mass.

Hanover, Mass. USGS quadrangle

Latitude: 42^o05'09" Longitude: 70^o45'33"

Facilities	<u>Facility</u>	Elevation
Affected:	7 houses	40
	North Street	40
	4 sheds	۷40
	3 Commercial buildings	40
	4 houses	35
	4 sheds	35
	Elm Street	35
	House	30
	Shed	30

Geologic Conditions: Both abutments are poorly graded sand and gravel outwash. Depth to bedrock in the foundation is estimated to be from 50 to 60 feet. Waterholding capabilities appear to be fair. Leakage is expected through both abutments. Pervious borrow material for dam construction was located near the site; impervious material was not located.

Engineering Notes:

The left abutment is recommended for the excavated emergency spillway location. A portion of the left abutment has been excavated for sand and gravel.

POTENTIAL SITE SS-2717

Location:

On an unnamed tributary to the North River at Magoun Pond about 2200 feet upstream from Union Street in Marshfield, Mass.

Hanover, Mass. USGS quadrangle

Latitude: 42°07'05" Longitude: 70°45'47"

Facilities	Facilit y	Elevation
Affected:	Cemetary (0.02 acres)	75
	Cemetary Road	70
	3 houses	65
	Shed	6 5

Geologic Conditions:

Both abutments are poorly graded sand and gravel outwash. Depth to bedrock in the foundation is estimated to be from 40 to 50 feet. Waterholding capabilities appear to be poor to fair. Leakage is expected through both abutments. Pervious borrow material for dam construction was located near the site; impervious material was not located.

Engineering Notes:

The right abutment is recommended for the excavated emergency spillway location.

SUMMARY DATA FOR POTENTIAL UPSTREAM RESERVOIR SITES

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SUMMARY DATA FOR POTENTIAL UPSTREAM RESERVOIR SITES

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SUBWATERSHED NORTH RIVER		***	COST *	* **	MASS GN STORM	2070 *	3200 * 2790 *	2690 *	中午年中午中午中午中午中午100000000000000000000000000	MASS GN STORM	*	3310 *	\$ 0685 *	3670 *	1920 *	*	sasasasasasasasasasasasasas SGS QUAD-WHITMAN, MASS	SIGN STORM		* 050 *	* 06/9	* 0664	3920 *	*	.****** DATA。	RAGE,	SHOWN	2	LAND ACQUISITION.
	SPILLWAY	***	AGE REST	Z	**************************************	5 4.1			*****	PRIN SPWY DESIGN S					8 10.8 0 18.2	:	QUAD-WHITMAN, MASS	SPWY DESI					2 7.1	:	***** COST	OTAL	ION. FIGURES SHOW		ON OR L
1	EMERGENCY	***************************************	STORAGE AT CREST	AC FT	UAD-CO	E 325 E 165	E 22.		*****	PRIN SP			E 147		E 508	1	UAD-WH	PRIN SP		140 1			E 252	:	*********	ED ON	ATION.		SELECTION OR
1	EMER	***	CREST ELEV		USGS Q 100-YR P	85.5	83.6	85.0	******	100-YR P		38.4	35.9	41.5	55.0 55.0	1	OSES C		i,	4.00	1.40	0.00	68.1	1			11. 11		SITE
JRE		* *	# * 1		941 AC TY (B)	5.3 * 17.5 *	19.2 * 20.2 *	20.5 *	*******	763 AC TY (B)	*	2.0 *	17.4 *	23.0 *	36.5 *	*	422 AC	ry (B)	# :	* 6.7	10.2 +	12.0 =	14.1 *	1	电电阻电阻电阻电阻电阻电阻电阻电阻电阻电阻电阻电阻电阻 BASED ON 1973 S.C.S. DESIGN	EMERGENCY SPILLWAY STORAGE AND COSTS ARE EMERGENCY SPILLWAY TYPE CODE— C=CONCRETE	ELIMINARY VEAREST O	DEGREE.	FOR FINAL
STUDY AREA-SOUTH SHORE			COST/ SURF	(\$)	= UALI	53770	3173C 2418C	22910	*	-			59840	4585C	21750	1	SITE-SS-2713 DA= 0.66 SQ MI = 422	STREAM WATER QUALITY (i i	22170	21760	21612	1	ndistrict and the state of the	STORAGE AND	TABULAR DATA ARE BASED ON PRELIM ELEVATIONS ARE SHOWN IN THE NEAR	THAT	
AREA-SI	JOL	***	AREA	(AC)	47 SQ I	12	23 33	35	****	M WATER		m j	15	27	56	1	DA= 0.66 SQ MI	M WATER	•	† (2 2	7 7	46	1	ON 19	MAY STO	E BASEL	RATE TO	** DO NOT USE
STUDY	BENEFICIAL POOL	***	COST PER	(\$)	DA= 1.47 SQ MI STREAM WATER C	6270	5540	4720	市市市市市市市市市市市市市市市市市市市市市市市市市市市市市市市市市市市市市市	STREA			8690	4960	1730	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	DA= 0	STREA			5140	7100	4000	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	E BASED	Y SPILLMAY	DATA AR	CONSIDERED ACCURATE TO THAT	•
1 1	BENEF	*	STORAGE	Z	(3)		1.7	2.2	***	(2)					15.0	1		(3)	•		7.0		7.0		COSTS ARE	EMERGENCY EMERGENCY	ABULAR LEVATIC	DNSIDER	
1		*	STO	AC FT	SITE-SS-2706 SITE RATING	001	131	169	*****	SITE RATING		0	100	107	704	1	-2713	SITE RATING	•	,	140	198	246	1	(1)	(2) EI (3) EI		_	
1		***	ELEV	(MSL)	SITE - SS-2706 SITE RATIN	67.3	81.1	82.5	**********	SITE		21.0	33.4	34.0	52.5	1	SITE-SS-2713	SITE	u 7 u	0.00	7.59	67.0	68.1	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	NOTES -				

SUMMARY CATA FOR POTENTIAL UPSTREAM RESERVOIR SITES

STUDY AREA-SOUTH SHCRE ************************************	* *	**************************************	STUCY AREA-SOUTH ************************************	REA-S	DUTF SH====================================	SHCRE ******	* * * * * * * * * * * * * * * * * * *	***** EMERGENCY	* * * *	SUBWAT ******* SPILLWAY	SUBWATERSHED	**************************************	RIVER ** GN **	* * * * * * * * * * * * * * * * * * * *	DAM	*	***** * SAFE
1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	1 1 1 1	1	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	1	2 3 3 1	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1		1		***************************************		HIGH WATER	TER *	1		***************************************	* YIELD
CLEV (MSL)	STGRAGE AC FT I	AGE In	CCST COST/OEP CLEV STGRAGE PER AREA SURF AT AC FT AC CA (MSL) AC FT IN (\$) (AC) (\$) (FT	AREA (AC)	COST/ SURF AC (E)	DEPTH AT CAM (FT)	CREST CREST ELEV + TYPE (MSL)	NA O	CRAGE CREST FT I	z	COST + PER + AC FT + (\$)	ELEV (MSL)	E A	TOP ELEV (MSL)	HGT FT		*PERCENT *CHANCE * (MGD)
**************************************	* 	(8)	**************************************	3 SC WATER	**************************************	6C3 AC	######################################		AD-HANCVE IN SPWY	**************************************	**************************************	LATITUD RUNDFF = 5	**************************************	103 IN	1 EAK	LONGITUDE FLOW =	70-48-09 841 CFS
35.5 40.2 43.7 51.9	100 233 500 901	0 1 2 1 0	10600 5050 2610 1670	15 29 46 70 194	3698C 2542C 2018C 776C	5.6 10.2 13.7 18.4	033°C 475°1		234 234 410 794 640	70 - 10 - 10 - 10 - 10 - 10 - 10 - 10 -	1010 * 4520 * 1770 * 920 * *	0 4 4 6 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	427 54 71 283 500	588 518.5 596.3 8.8	29. 18 22 26 30	51 19 27 41 57	0.20 0.43 0.81 1.26
SITE-SS-2715 SITE RATING	* 	(3)	CA= 1.01 SQ MI = 646 STREAM WATER QUALITY (NATER	**************************************	646 AC IY (B)	USGS 100-YR	CUAD	GUAD-HANCVE PRIN SPWY D	ER, MASS DESIGN	SSTORM	RUNDFF	LATITUDE RUNOFF = 5.8	42-0	5-32 LONGI PEAK FLOW	ONGITUDE FLOW =	70-45-35 335 CFS
7	100 177 253 406 424	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	764C 47C0 328C 2C2C 2C70	7 4 4 4 1 1 5 6 C C C C C C C C C C C C C C C C C C	764C 25 3054C 7 47C0 31 2626C 1C 328C 37 225CC 12 2C2C 48 17C1C 16 2C7O 5C 1773C 16		F 9 8 H 7 5 i	កាកកាកាក	1		2380 * * 4290 * * 2290 * 1500 * 1550	59	6 6 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5	63.0 61.5 64.1 66.5 70.0	119 118 118 24 24	26 30 30 52 52	0.024
SITE-SS-2716 SITE RATIN			CA= 3.58 SQ MI = 2291 STREAM WATER QUALITY	WATER	= 3.58 SG MI = 2251 STREAM WATER QUALITY (251 AC 1Y (B)	USGS 100-YR	QUAD-	HANCVE	CUAD-HANCVER, MAS	S STORM	RUNDFF	ATITUDE F = 5.8	42-05-09 L 80 IN, PEAK		ONGITUDE FLOW =	70-45-33 980 CFS
	545 1000 1455 1909	0.0 2.5 5.1 7.6 10.0	1850 1250 1000 870	24 122. 156. 196 239	828C 8C3C 744C 697C	1.6 8.7 12.0 14.6	29.0 29.7 35.8 35.8	T T T T T T T T T T T T T T T T T T T	090 221 892 023 1 605 1	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	740 * 830 * 720 * 640 *	382 385 386 386 386 386	200 217 * 287 * 300 *	4 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9	19 19 23 25	22 45 51 51	0.77 1.20 1.56
******* NCTES -	******* (1) COS	*****	本意本语本本本主要主要主要主要主要主要主要主要主要主要主要主要主要主要主要。 C(I) COSIS ARE BASED CN 1973 S.C.S.C.	CN 197	**************************************	**************************************	**************************************		********AND COST	*******	* * * * * * * * *	* * * * * *	* * * * * *	* * * * * * * * * * * * * * * * * * * *	* * * * * * * * * * * * * * * * * * * *	* * * * * * * * * * * * * * * * * * * *	* * * * * * * * * * * * * * * * * * * *

(2) EMERGENCY SPILLWAY STORAGE AND COSTS ARE BASED ON TOTAL STORAGE, INCLUDING BENEFICIAL POOL.

(3) EMERGENCY SPILLWAY TYPE COCE— C=CONCRETE CHUTE, D=CCNCRETE DROP, E=EXCAVATED, T= TWO SPILLWAYS, N= NONE

(4) TABULAR CATA ARE BASED ON PRELIMINARY INFORMATION. FIGURES SHOWN ARE PRIMARILY FOR COMPARISON PURPOSES.

(5) ELEVATIONS ARE SHOWN TO THE NEAREST 0.1 FCOT TO SHOW VARIATION BETWEEN DEVELOPMENTS ONLY, AND ARE NOT TO BE CONSIDERED ACCURATE TO THAT DEGREE.

** DO NOT USE FOR FINAL SITE SELECTION OR LAND ACQUISITION. **

SUMMARY CATA FOR POTENTIAL UPSTREAM RESERVOIR SITES

## BENEFICIAL POOL ## EMERGENCY SPILLWAY ## HIGH WATER ## YIELD ##	3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	1 1 1	STUCY	AREA-S	STUCY AREA-SOUTH SHERE	CRE	1 1 1 1 1 1	1	SUBM	SUBWATERSHED NORTH RIVER	NORTH	NIVER	1	1			
# HIGH ###################################	** ** ** ** ** **		BENEFI	CIAL PO	JOL.			######################################	GENCY S	PILLWA	*	DESI	* * Z	*	**** DAM	**	****** * SAFE	***
RAGE PER AREA SURF AT # ELEV AT CREST PER # ELEV AC FT # AC FT # AC FT # ELEV AT CREST PER # ELEV IN (\$) (AC) (\$) (FT) # (MSL) AC FT IN (\$) # (MSL) EA = 1.05 SQ MI = 672 AC USGS QUAD—HANCVER, MASS (3) STREAM WATER QUALITY (B) 100—YR PRIN SPWY DESIGN STORM RUNC 1.7 8360 20 4239C 17.0 # 61.5 E 167 3.0 5010 # 63. 3.3 4430 31 2626C 20.4 # 64.9 E 285 5.1 2890 # 67. 6.4 2310 48 1716C 24.7 # 69.3 E 499 8.8 1660 # 71. 6.4 2310 48 1716C 24.7 # 69.3 E 499 8.8 1660 # 71. 9.5 1660 6C 1454C 28.0 # 72.5 E 705 12.6 1250 # 76. 11.C 1660 6C 1547C 28.0 # 72.5 E 705 12.6 1250 # 76. STREAM WATER AND COSTS ARE BASED ON 1973 S.C.S. DESIGN CRITERIA AND COST DATA. MERGENCY SPILLMAY TYPE CODE— C=CONCRETE CHUTE, D=CONCRETE DROP, E=EXCLABALLAR DATA ARE BASED ON PRELIMINARY INFORMATION. FIGURES SHOWN ARE PROPERTY.	3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	1 1		1 1 1	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	1	1 1 1 1 1 1	, 1	1 1	**	HIGH W	ATER .	1	1		* YIELD	07
AC FT AC AC DAM + TYPE AC FT AC FT + (MSL) 1N (\$) (\$C) (\$C) (\$C) (\$C) (\$C) (\$C) (\$C)		CTO	n O A	COST	* U	COST/	DEPTH	CREST	STORA	6E ***	COST	* 3		T0P	uis .	FILL	* AT 95 *PERCENT	SENT SENT
IN (\$) (AC) (\$) (FI) * (MSL) AC FT IN (\$) * (MSL) ************************************		ה ה	1040	AC FT	AREA	AC	5	* clev		100	AC FT *	ברנג		EL EV		(1000	*CHANCE	ה ה
CA= 1.05 SQ MI = 672 AC USGS QUAD-HANCVER, MASS (3) STREAM WATER QUALITY (B) 100-YR PRIN SPWY DESIGN STORM RUNG * 6.6 C	(HSF)	AC FT	Z	(\$)	(AC)	(\$)	_	* (MSL)	AC FT	z		(WSF)	(AC) *	(WSF)	Ħ	CY)	CY) * (MGD)	(0:
SITE RATING (3) STREAM WATER QUALITY (B) 100-YR PRIN SPWY DESIGN STORM RUNDFF = 5.80 IN, PEAK FLOW = 47.7 47.7 0 0.0 3 5.6 * 63.5 E 232 4.1 2640 * 66.0 45 * 69.1 27 22 59.0 100 1.7 8360 20 42390 17.0 * 61.5 E 167 3.0 5010 * 63.9 37 * 66.9 25 18 62.4 186 3.3 4430 31 26260 20.4 * 64.9 E 285 5.1 2890 * 67.3 50 * 70.3 28 25 66.8 358 6.4 2310 48 17160 24.7 * 69.3 E 499 8.8 1660 * 71.6 68 * 74.6 33 36 47 71.0 530 9.5 1660 60 14540 28.0 * 72.5 E 705 12.6 1250 * 74.8 81 * 77.8 36 47 71.3 616 11.0 1660 66 15470 29.2 * 73.8 E 804 14.3 1270 * 76.1 87 * 79.1 37 53 **********************************	SITE-SS	-2717	*	EA= 1.	***** 05 SQ	* * * * * * * * * * * * * * * * * * *	****** 672 AC	***********	**************************************	***** CVER,M	****** ASS	*****	******* AT ITUDE	42-07-	****** 05 LON	******	7-02	15-47
47.7 0 0.0 c 3 5.6 * 63.5 E 232 4.1 2640 * 66.0 45 * 69.1 27 22 59.0 100 1.7 8360 20 4239C 17.0 * 61.5 E 167 3.0 5010 * 63.9 37 * 66.9 25 18 59.0 100 1.7 8360 20 4239C 17.0 * 61.5 E 167 3.0 5010 * 63.9 37 * 66.9 25 18 62.4 186 3.3 4430 31 2626C 20.4 * 64.9 E 285 5.1 2890 * 67.3 50 * 70.3 28 25 56.0 6.8 358 6.4 2310 48 1716C 24.7 * 69.3 E 499 8.8 1660 * 71.6 68 * 74.6 33 36 47 70.0 530 9.5 1660 6C 1454C 28.0 * 72.5 E 705 12.6 1250 * 74.8 81 * 77.8 36 47 71.3 616 11.0 1660 6C 1454C 28.0 * 72.5 E 705 12.6 1250 * 74.8 81 * 77.8 36 47 71.3 616 11.0 1650 6C 1454C 28.0 * 72.5 E 705 12.6 1250 * 74.8 81 * 77.8 36 47 71.3 616 11.0 1650 6C 1454C 28.0 * 72.5 E 705 12.6 1250 * 74.8 81 * 77.8 36 47 71.3 616 11.0 1650 6C 1454C 28.0 * 72.5 E 705 12.6 1250 * 74.8 81 * 74.6 13.7 81 81 81 81 81 81 81 81 81 81 81 81 81	SITE	RATING		STREA	IM WATE	R QUALI		100-YR P	RIN SPW	Y DESI	GN STORM	RUNG	= 5	80 IN,	PEAK FL	= MO	348	CFS
47.7 0 0.0 C 3 5.6 * 63.5 E 232 4.1 2640 * 66.0 45 * 69.1 27 22 59.0 100 1.7 8360 20 42390 17.0 * 61.5 E 167 3.0 5010 * 63.9 37 * 66.9 25 18 59.0 100 1.7 8360 20 42390 17.0 * 61.5 E 167 3.0 5010 * 67.3 50 * 70.3 28 25 5.6 66.8 3.3 4430 31 26260 20.4 * 64.9 E 285 5.1 2890 * 67.3 50 * 70.3 28 25 56.8 358 6.4 2310 48 17160 24.7 * 69.3 E 499 8.8 1660 * 71.6 68 * 74.6 33 34 47 70.0 530 9.5 1660 60 14540 28.0 * 72.5 E 705 12.6 1250 * 74.8 81 * 77.8 36 47 71.3 616 11.0 1660 66 15470 29.2 * 73.8 E 804 14.3 1270 * 76.1 87 * 79.1 37 53 47 77.8 8 6 15470 29.2 * 73.8 E 804 14.3 1270 * 76.1 87 * 79.1 37 53 84 84 84 84 84 84 84 84 84 84 84 84 84							•				*		*				*	
59.0 100 1.7 8360 20 4239C 17.0 * 61.5 E 167 3.0 5010 * 63.9 37 * 66.9 25 18 62.4 186 3.3 4430 31 2626C 20.4 * 64.9 E 285 5.1 2890 * 67.3 50 * 70.3 28 25 66.8 358 6.4 2310 48 1716C 24.7 * 69.3 E 499 8.8 1660 * 71.6 68 * 74.6 33 36 70.0 530 9.5 1660 6C 1454C 28.0 * 72.5 E 705 12.6 1250 * 74.8 81 * 77.8 36 71.3 616 11.0 160 66 1547C 29.2 * 73.8 E 804 14.3 1270 * 76.1 87 * 79.1 37 53 ************************************	47.7	0	0.0		3		5.6	* 63.5		4.1	2640 *	0.99	45 #	69.1		22	* * *	****
62.4 186 3.3 4430 31 2626C 20.4 * 64.9 E 285 5.1 2890 * 67.3 50 * 70.3 28 25 66.8 35 66.8 35 66.4 2310 48 1716C 24.7 * 69.3 E 499 8.8 1660 * 71.6 68 * 74.6 33 36 47 70.0 530 9.5 1660 6C 1454C 28.0 * 72.5 E 705 12.6 1250 * 74.8 81 * 77.8 36 47 71.3 616 11.0 1660 66 1547C 29.2 * 73.8 E 804 14.3 1270 * 76.1 87 * 79.1 37 53 8***********************************	59.0	100	1.7	8360	20	4239C	17.0	* 61.5	E 167	3.0	\$010	63.9		6.99		18	*	16
66.8 358 6.4 2310 48 1716C 24.7 * 69.3 E 499 8.8 1660 * 71.6 68 * 74.6 33 36 70.0 530 9.5 1660 60 1454C 28.0 * 72.5 E 705 12.6 1250 * 74.8 81 * 77.8 36 47 71.3 616 11.0 1660 66 1547C 29.2 * 73.8 E 804 14.3 1270 * 76.1 87 * 79.1 37 53 8***********************************	62.4	186	3.3	4430	31	2626C	20.4	6.49	E 285	5.1	2890 *	67.3		70.3		25	*	. 26
70.0 530 9.5 1660 6C 1454C 28.0 * 72.5 E 705 12.6 1250 * 74.8 81 * 77.8 36 47 71.3 616 11.0 1660 66 1547C 29.2 * 73.8 E 804 14.3 1270 * 76.1 87 * 79.1 37 53 83 47.1 87 * 79.1 37 53 84 84.4 *******************************	8.99	358	6.4	2310	48	1716C	24.7	* 69.3		8.8	1660 *	71.6		74.6		36	*	40
71.3 616 11.C 1660 66 1547C 29.2 * 73.8 E 804 14.3 1270 * 76.1 87 # 79.1 37 53 *********************************	70.0	530	9.5	1660	29	1454C	28.0	* 72.5		12.6	1250 *	74.8		77.8		47	*	533
**************************************	71.3	919	11.0	1660	99		29.2	* 73.8		14.3	1270 #	76.1		79.1		53	•	0.58
NOTES - (1) COSTS ARE BASED ON 1973 S.C.S. DESIGN CRITERIA AND COST DATA. (2) EMERGENCY SPILLWAY STORAGE AND COSTS ARE BASED ON TOTAL STORAGE, INCLUDING BENEFICIAL POOL. (3) EMERGENCY SPILLWAY TYPE CODE- C=CONCRETE CHUTE, D=CONCRETE DROP, E=EXCAVATED, T= TWO SPILLWAYS, N= (4) TABULAR DATA ARE BASED ON PRELIMINARY INFORMATION. FIGURES SHOWN ARE PRIMARILY FOR COMPARISON PIRE	* * * * * * * * * * * * * * * * * * * *	****	***	***	***	****	******	********	*****	****	******	*****	*****	****	******	*****	****	* * * * *
(3) EMERGENCY SPILLWAY TYPE CODE— C=CONCRETE CHUTE, D=CONCRETE DROP, E=EXCAVATED, T= TWO SPILLWAYS, N= (4) TABULAR DATA ARE BASED ON PRELIMINARY INFORMATION. FIGURES SHOWN ARE PRIMARILY FOR COMPARISON PURP.	NOTES -	(1) (2)	ERGENC	Y SPILL	WAY ST	73 S.C.	S. DEST	GN CRITER S ARF RAS	TED CN T	COST D	ATA. TORAGE.	TACLUDIA	ממממ	FICIAL	1000			
(4) TABULAR DATA ARE BASED ON PRELIMINARY INFORMATION, FIGURES SHOWN ARE PRIMARILY FOR COMPARISON DIRECTOR		(3) EM	ERGENC	Y SPILL	WAY TY	PE CODE	- C=CON	CRETE CHU	TE, D=C	ONCRET	E DROP.	E=FXCAV	ATED. I	THU S	PILLWAY	=N = 2'	I NON	
		(4) TA	BULAR	DATA AR	E BASE	D ON PR	FLIMINA	RY INFORM	ATION	FIGURE	NACHS	ARE PRI	MARTIY	FUB COM	DARTON	PHED	CEC	

(4) TABULAR DATA ARE BASED UN PRELIMINARY INFORMATION. FIGURES SHOWN ARE PRIMARILY FOR COMPARISON PURPOSES.
(5) ELEVATIONS ARE SHOWN TO THE NEAREST 0.1 FOOT TO SHOW VARIATION BETWEEN DEVELOPMENTS ONLY, AND ARE NOT TO BE CONSIDERED ACCURATE TO THAT DEGREE.
** DO NOT USE FOR FINAL SITE SELECTION OR LAND ACCUISITION. **

Existing Site SS-2701 (Turner Pond)

On Second Herring Brook about 2,000 feet downstream from Location:

Cross Street in Norwell, Mass.

Cohasset, Mass. USGS quadrangle

Latitude: 42°10'21" Longitude: 70°47'14"

Surface Area

Height of Drainage Area Dam (Ft.) (Acres)

 $\frac{\text{(Acres)}}{400} \qquad \frac{\text{(Sq. Miles)}}{0.63}$

Potential for

Please refer to Site Data and Design Summary Table for Potential Site SS-2701

Expansion:

Remarks:

The dam is an earthfill structure. The spillway is a stonewalled structure with concrete floor. The weir is about 5.5 feet long and 2.5 feet deep and has provision for flashboards. Trees and brush are growing on the dam. Seepage was noted

along the downstream toe of the dam.

Ownership and

The pond is owned by Mr. Cummings and is used for recreation.

Use:

Existing Site SS-2716

On Pudding Brook about 4,800 feet upstream from Route 53 in Location: Pembroke, Mass.

Hanover, Mass. USGS quadrangle

Latitude: 42⁰05'09"

Longitude: 70^o45'33"

Surface Area

Height of (Acres) Dam (Ft.)

Drainage Area

(Acres) (Sq. Miles) 2,300 3.59

Potential for

Please refer to Site Data and Design Summary Table for Potential Site SS-2716.

Expansion:

Remarks:

The dam is a long earthfill structure constructed of sandy material. The spillway is a two-bay concrete structure with flashboard control. Each bay is about 4 feet wide and 4 feet deep. There is also a concrete weir about 4 feet long and four feet deep with flashboard control which delivers water to a cranberry bog. Concrete in the main spillway is spalled and broken in places.

Ownership The pond is owned by David Mann and is used for cranberry bog and irrigation and recreation.

Use:

Existing Site SS-2717 (Magoun Pond)

Location: On an unnamed tributary to the North River about 2,300 feet upstream from Union Street in Marshfield, Mass.

Hanover, Mass. USGS quadrangle

Latitude: 42^o07'04" Longitude: 70^o45'42"

(Acres)

Surface Area Height of Drainage Area
(Acres) Dam (Ft.) (Acres) (Sq. Miles)

10 15 650 1.02

Potential Please refer to Site Data and Design Summary Table for Potential Site SS-2717.

Expansion:

Remarks:

The dam is an earthfill structure. The spillway is a concrete weir about 6 feet long and 4 feet deep. The spillway has provision for flashboards. The downstream channel has stone walls for about 50 feet. Concrete in the spillway is spalled and broken in places. Trees are growing on the dam.

Ownership The pond is owned by Dr. Hilda Gerry and is used for recreation. and Use:

Existing Site SS-2719 (Wampatuck Pond)

Location: On Indian Head Brook at Liberty Street in Hanson, Mass.

Hanover, Mass. USGS quadrangle

Latitude: 42⁰03'53"

Longitude: 70°51'56"

Surface Area Height of Drainage Area
(Acres) Dam (Ft.) (Acres) (Sq. Miles)

10 1,570 2.45

Potential Limited. A sewage disposal plant located upstream would be affected. A large area of shallow water would be created by expansion. . for Expansion:

Liberty Street forms the dam. A culvert carries water under Liberty Remarks: Street. Two bays of flashboards control the water level.

Ownership The pond is owned by the town of Hanson and is used for recreation. and Use:

Existing Site SS-2720 (Trout Pond)

On Rocky Run about 1,750 feet upstream from Indian Head River Location: on the Pembroke-Hanson town line.

Hanover, Mass. USGS quadrangle

Latitude: 42^o05'27" Longitude: 70^o50'44"

Surface Area Height of Drainage Area

(Acres) Dam (Ft.) (Acres) (Sq. Miles)

3 20 300 0.47

Potential for

Raising the pond level by 10 feet would creat a 30 acre pool. Washington Street would be affected.

Expansion:

Remarks:

The dam is an old stone structure which has recently been capped with concrete. The principal spillway is an 8-foot long weir notch in the concrete cap. Water leaks through the stone work beneath the concrete cap.

Ownership and

The pond is owned by Dr. Bruce Roberts and is not used for a specific purpose.

Use:

Existing Site SS-2721 (Factory Pond)

Location: On Drinkwater River about 700 feet upstream from Center Street on the Hanson-Hanover town line.

Hanover, Mass. USGS quadrangle

Latitude: 42⁰05'20"

Longitude: 70°52'04"

Surface Area Height of Drainage Area (Acres) Dam (Ft.) (Acres) (Sq. Miles) 12,300 19.22

Potential

for Expansion:

Limited. About 25 buildings would be affected by expansion.

Remarks:

The dam is an earthfill structure. The principal spillway is a stone masonry and concret weir about 125 feet long. There is also a 7-foot wide sluiceway which delivered water to an old mill.

Ownership and

Ownership of the pond was not determined. The pond is used for recreation.

Use:

Existing Site SS-2722 (Forge Pond)

Location: On the Drinkwater River just downstream from King Street in Hanover, Mass.

Whitman, Mass. USGS quadrangle

Latitude: 42^o06'13" Longitude: 70^o52'42"

Surface Area Height of Drainage Area

(Acres) Dam (Ft.) (Acres) (Sq. Miles)

17 4 10,700 16.72

Potential Raising the pond level by 5 feet would create an 80-acre pool. for King Street and at least 8 buildings would be affected. Expansion:

Remarks: The dam is an earthfill structure. The spillway is a reinforced concrete structure with six bays of flashboards, each 5 feet wide Water level can be varied about 5 feet by the use of flashboards.

Ownership The pond is owned by the town of Hanover and is used for recreaand tion. Use:

Existing Site SS-2723 (Studley Pond)

Location: On French Stream at Route 123 in Rockland, Mass.

Whitman, Mass. USGS quadrangle

Latitude: 42°07'09" Longitude: 70°55'02"

Surface Area Height of Drainage Area

(Acres) Dam (Ft.) (Acres) (Sq. Miles)

28 8 2,250 3.52

Potential Limited. The pond is surrounded by streets, houses, and cemeteries for which would be affected.

Expansion:

Remarks: Route 123 forms the dam. The spillway is a concrete box culvert with flashboard control at the inlet. The upstream face of the dam is stone masonry and concrete. Gravel is being excavated from the pond at the present time and the pond has been drained.

Ownership The pond is owned by the town of Rockland and is used for and recreation.
Use:

Existing Site SS-2724 (Hackett Pond)

Location: On Longwater Brook about 1,000 feet downstream from Webster Street in Hanover, Mass.

Cohasset, Mass. USGS quadrangle

Latitude: 42°08'35" Longitude: 70°52'16"

Surface Area Height of Drainage Area

(Acres) Dam (Ft.) (Acres) (Sq. Miles)

7 10 1,225 1.91

Potential Limited. Route 123 and severl houses would be affected. for Expansion:

The dam is an earthfill structure. The upstream slope adjacent to Remarks: the spillway has been riprapped. The spillway is a concrete ogee weir about 30 feet long and 3 feet deep which leads to a concrete chute which is 8 feet wide, 5 feet deep, and 30 feet long.

The pond is owned by the town of Hanover and is not used for Ownership and a specific purpose. Use:

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Existing Site SS-2725 (Mill Pond)

Location: On Third Herring Brook about 1,200 feet downstream from Mill Street in Hanover, Mass.

Cohasset, Mass. USGS quadrangle

Latitude: 42^o08'28" Longitude: 70^o50'07"

Surface Area Height of Drainage Area
(Acres) Dam (Ft.) (Acres) (Sq. Miles)

5 12 1,525 2.38

Potential Limited by an upstream shopping plaza and a sewage treatment plant. for Expansion:

Remarks: The dam is an earthfill structure. The fill material has been eroded around both sides of the principal spillway structure. All flow is around the spillway structure. The dam and spillway are in poor condition.

Ownership Ownership of the pond was not determined. The pond is not used and for a specific purpose. Use:

Existing Site SS-2726 (Jacobs Pond)

Location: On Third Herring Brook at Route 123 in Norwell, Mass

Cohasset, Mass. #ECS quadrangle

Latitude: 42^o09 30 Longitude: 70^o50'51"

Surface Area Height of Drainage Area
(Acres) Dam (Ft.) (Acres) (Sq. Miles)

55 15 950 1.48

Potential Limited. The east side of the pond and expansion area is a residential for area which would be affected.

Expansion:

Remarks: Route 123 forms the dam. The spillway is a 72-inch concrete culvert with flashboard control at the inlet. It appears that water may overtop Route 123 in periods of high flow.

Ownership The pond is owned by the town of Norwell and is used for recreation. and Use:

Existing Site SS-2727

On Second Herring Brook about 1,000 feet upstream from Cross Location: Street in Norwell, Mass.

Cohasset, Mass. USGS quadrangle

Latitude: 42°10'39"

Longitude: 70°46'43"

Surface Area Height of Drainage Area
(Acres) Dam (Ft.) (Acres) (Sq. Miles)

10 12 85 0.13

Potential The small drainage area limists expansion potential. for Expansion:

Remarks:

The dam is an earthfill structure. The spillway is a concrete weir about 8 feet long and 2 feet deep. There is also a spillway near the right abutment to control flow to cranberry bogs. Seepage was noted along the downstream slope of the dam.

Ownership The pond is owned by Coles and Leclair and is used for cranberry and bog irrigation. Use:

Existing Site SS-2728 (Tack Factory Pond)

Location: On Herring Brook just upstream from Route 3-A in Scituate, Mass.

Cohasset, Mass. USGS quadrangle

Latitude: 42°11'11"

Longitude: 70°45'41"

Surface Area Height of Drainage Area

(Acres) Dam (Ft.) (Acres) (Sq. Miles)

7 6 2,450 3.83

Potential Raising the pond level by 10 feet would create a 90 acre pool. for Expansion:

Remarks:

An abandoned section of Route 3-A forms the dam. Water from Existing Site SS-2729(New Water Supply Reservoir) creates a tailwater condition against the dam. The spillway is a concrete weir. Concrete in the spillway is spalled and cracked. There is very little freeboard on the dam.

Ownership The pond is owned by the town of Scituate and is used for water and supply. Use:

Existing Site SS-2729 (New Water Supply Reservoir)

Location: On Herring Brook about 2,900 feet downstream from Route 3A in Scituate, Mass.

Cohasset, Mass. USGS quadrangle

Latitude: 42°11'07" Longitude: 70°45'11"

Surface Area Height of Drainage Area
(Acres) Dam (Ft.) (Acres) (Sq. Miles)

80 25 2,850 4.45

Potential Limited by housing projects on both abutments. Expansion would affect Route 3A. for Expansion:

The dam is an earthfill structure. The upstream slope of the dam Remarks: is riprapped. The principal spillway is an 18-inch cast iron pipe to handle normal flow. The emergency spillway is a concrete ogee weir about 25 feet long and 5 feet deep. There is also a concrete

fish ladder. The dam and spillways are in good condition.

Ownership The reservoir is owned by the town of Scituate and is used for water supply. and

Use: Existing Site SS-2730 (Old Oaken Bucket Pond)

Location:

On Herring Brook the intersection of Route 3A and 123 in Scituate, Mass.

Cohasset, Mass. USGS quadrangle

Latitude: 42°10'40" Longitude: 70°45'01"

Surface Area Height of Drainage Area
(Acres) Dam (Ft.) (Acres) (Sq. Miles)

5 3,400 5.31

Potential for

Limited. Development around the pond and a pumping station would be affected. Scituate's New Water Supply Reservoir Expansion: is located immediately upstream.

Remarks:

The dam is an earthfill structure. The spillway is a series of concrete drop structures. The spillway on the right abutment was recently constructed. There is also a fish ladder near the left abutment. Large trees are growing on the dam. Water has overtopped the center area of the dam.

Ownership and

Use:

The pond is owned by the town of Scituate and is used for water supply.

Existing Site SS-2732 (Torrey Pond)

Location:

On Second Herring Brook at Mill Lane in Norwell, Mass.

Cohasset, Mass. USGS quadrangle

Latitude: 42^o09'56" Longitude: 70^o47'25"

Surface Area Height of (Acres) 15

Dam (Ft.)
10

Drainage Acres

(Acres) (Sq. Miles)

Potential for Expansion: Limited. A large area of shallow water would be created by expansion into Dead Swamp.

Remarks:

The dam is an earthfull structure. The spillway is a concrete weir with flashboards. The weir is about 4 feet long and 4 feet deep. Concrete in the spillway is cracked and spalled. Trees and brush are growing on the dam.

Ownership and

The pond is owned by Robert Zildjin and is used for recreation.

Use:

Existing Site SS-2733 (Hatch Pond)

Location:

On an unnamed tributary to the North River about 400 feet downstream from Union Street in Marshfield, Mass.

Hanover, Mass. USGS quadraangle.

Latitude: 42°07'24" Longitude: 70°46'07"

Surface Area Height of Drainage Area
(Acres) Dam (Ft.) (Acres) (Sq. Miles)
1 8 1,200 1.88

Potential for

Expansion:

Limited. Pine Street, Union Street, and several houses would be affected. Sandy soils may limit expansion potential.

Remarks:

The dam is an earthfill structure. The spillway is a twobay timber flume. There is also a mill race near the right abutment. Trees are growing on the dam and seepage through the dam was noted.

Ownership and Use:

The pond is owned by the Marshfield Historical Society. pond and mill are historical landmarks.

Existing Site SS-2734 (Oakman Pond)

Location:

On an unnamed tributary to the North River about 750 feet upstream from Union Street in Marshfield, Mass.

Hanover, Mass. USGS quadrangle

Latitude: 42°07'14" Longitude: 70°45'59"

Surface Area (Acres) Height of Dam (Ft.)
10 Drainage Area

(Acres) (Sq. Miles) 825 1.29

Potential for Expansion: Raising the pond level by 20 feet would create a 20 acre pool. Several gravel roads would be affected.

Remarks:

The dam is an earthfill structure. The downstream face ia a vertical stone wall. The spillway inlet is a concrete weir structure about 4 feet wide, which outlets to twin 24-inch concrete culverts. The spillway inlet has provision for flashboards. There is also an old mill weir located to the right of the principal spillway.

Existing Site SS-2734 (Oakman Pond) (cont'd)

Ownership and

The pond is owned by Dorothy Thompson and is used for recreation.

Use:

Existing Site SS-2736

Location:

On an unnamed tributary to Pudding Brook near Arnold School in Pembroke, Mass.

Hanover, Mass. USGS quadrangle

Latitude: 42°05'28" Longitude: 70°45'25"

Surface Area

Height of Drainage Area

Dam (Ft.) (Acres) (Sq. Miles)

75 0.12

Potential for

The small drainage area limits expansion potential.

Expansion:

Remarks:

The dam is an earthfill structure. The upstream face is a concrete retaining wall. The spillway is a concrete drop structure with flashboards which outlets to a concrete box culvert. The dam and spillway are in good condition.

Ownership and

The pond is owned by the Association for Cultural Interchange and is used for recreation.

Use:

Existing Site SS-2737 (Randall Pond)

Location:

On Pudding Brook about 800 feet upstream from Spring Street in Pembroke, Mass.

Hanover, Mass. USGS quadrangle

Latitude: 42°05'17" Longitude: 70°45'30"

Surface Area Height of Drainage Area
(Acres) Dam (Ft.) (Acres) (Sq. Miles)
775 1.21

Potential for Expansion: Expansion would create a large area of shallow water as the pond would be expanded into a large wetland area.

Remarks:

A gravel driveway forms the dam. The spillway is a concrete sluiceway with two bays of flashboards. Each bay is about 3 feet wide and 3 feet deep.

Ownership and

Use:

The pond is owned by the Association for Cultural Interchange and is not used for a specific purpose.

Existing Site SS-2739 (Mill Pond)

Location:

On Herring Brook about 400 feet downstream from Hobomock Street in Pembroke, Mass.

Hanover, Mass. USGS quadrangle

Latitude: 42°03'21"

Longitude: 70°48'13

Surface Area Height of Drainage Area
(Acres) Dam (Ft.) (Acres) (Sq. Miles)
7 5 2,700 4.22

Potential for Expansion: Topography limits any significant increase in surface area. Sandy soils may limit expansion potential.

Remarks:

The dam is an earthfill structure. The spillway is a concrete weir with flashboard control. The weir is about 4 feet long and 3 feet deep. Trees are growing on the dam. Water has eroded the dam near the left abutment. Concrete in the spillway is crumbling.

Ownership and

Use:

The pond is owned by George Gould and is used for cranberry bog irrigation.

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Existing Site SS-2740 (Indian Head River Dam)

Location:

On Indian Head River about 300 feet upstream from Elm Street on the Hanover-Pembroke town line.

Hanover, Mass. USGS quadrangle

Latitude: 42^o06'00" Longitude: 70^o48'59"

Surface Area Height of Drainage Area
(Acres) Dam (Ft.) (Acres) (Sq. Miles)

15 12 27,100 42.37

Potential for Expansion:

Topography limits any significant increase in surface area without affecting many facilities.

Remarks:

The dam is an earthfill structure. The upstream face is a concrete wall. The spillway is a concrete weir about 75 feet long and four feet deep. Water falls about 10 feet to a wide channel through a series of bays, each about 10 feet wide. Each bay has been filled with rocks. There is also a concrete fish ladder. The dam was renovated in 1970. Dam and spillway system are in good condition.

Ownership and Use:

The dam is owned by the towns of Hanover and Pembroke and is used for recreation.



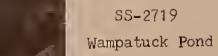
SS-2701 Turner Pond

SS-2720 Trout Pond



SS-2717 Magoun Pond







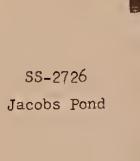
SS-2721
Factory Pond







SS-2722 Forge Pond





SS-2723 Studleys Pond

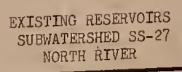




SS-2724 Hackett Pond

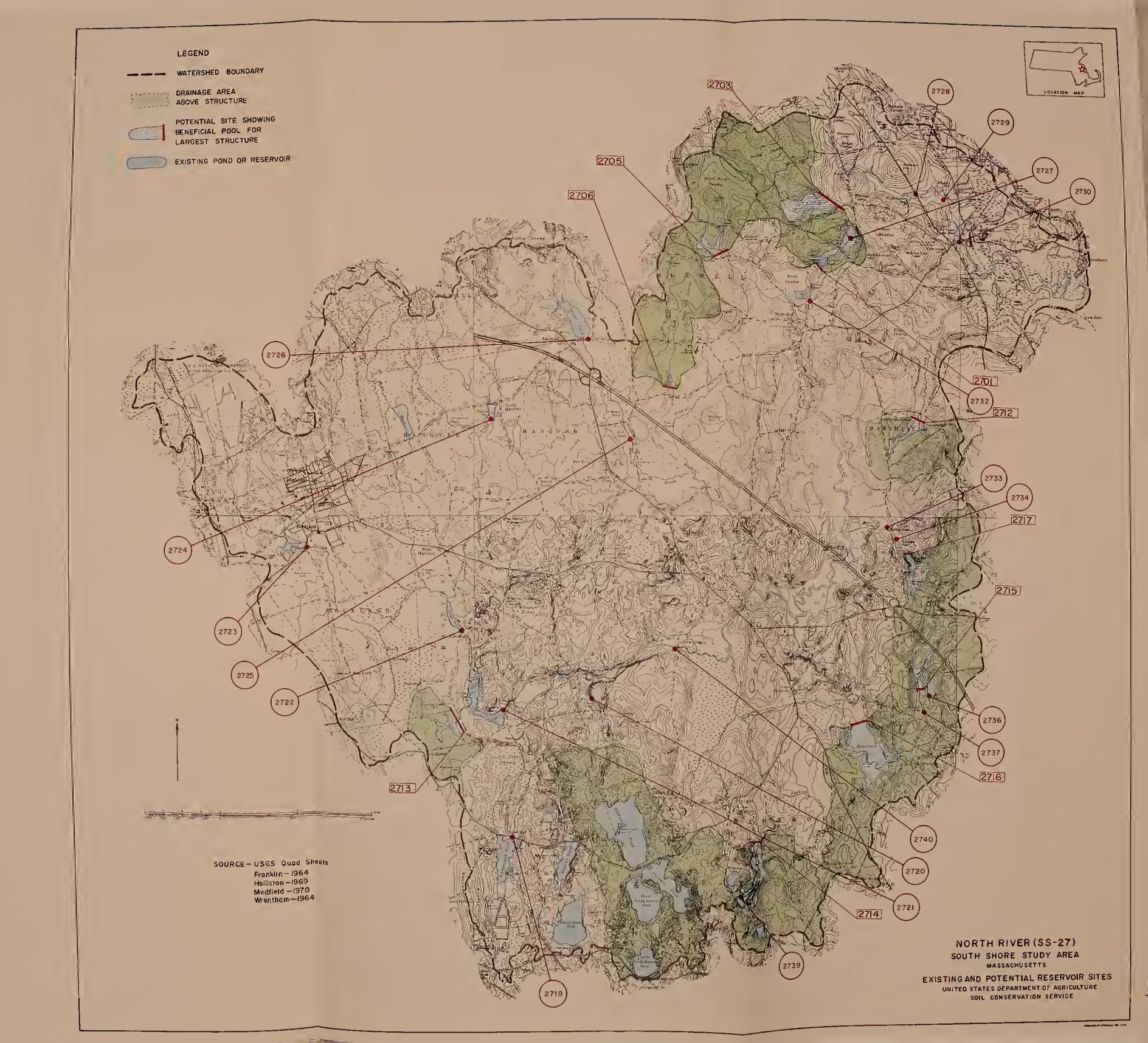


SS-2728
Tack Factory Pond











SOUTH SHORE STUDY AREA SITE DATA FOR

Subwatershed SS-28, South River

This subwatershed covers about 20,900 acres in Duxbury, Marshfield and Pembroke, in Plymouth County.

The major streams in this subwatershed are the South River and Green Harbor River. The South River originates in Duxbury and flows generally northeasterly through Marshfield, then turns northwesterly and flows into Massachusetts Bay. Green Harbor River originates in Marshfield and flows generally easterly into Massachusetts Bay. Elevations range from a high of about 254 feet on Carolina Hill in Marshfield to tide water in Massachusetts Bay.

Geology in this subwatershed is characterized by granite bedrock overlain by 20 to 80 feet of outwash sand and gravel or glacial till.

Seven potential reservoir sites and 5 existing reservoirs were studied.

POTENTIAL SITE SS-2801

Location:

On Hannah Eames Brook about 500 feet upstream from Summer Street in Marshfield, Mass.

Scituate, Mass. USGS quadrangle

Latitude: 42⁰09'05"

Longitude: 70°44'03"

Facilities Affected:

Facility
Industrial building
Route 3-A

Elevation 55

Geologic Conditions:

Both abutments are glacial till with cobbles and boulders. Depth to granite bedrock in the foundation is estimated to be from 20 to 25 feet. Waterholding capabilities appear to be good. Borrow material for dam construction was located near the site.

Engineering Notes:

The right abutment is recommended for the excavated emergency spillway location.

POTENTIAL SITE SS-2802

Location:

On Littles Creek about 1600 feet upstream from Summer Street in Marshfield, Mass.

Scituate, Mass. USGS quadrangle

Latitude: 42°07'53" Longitude: 70°42'43"

Facilities Affected:	Facility Church Street Ferry Street Grove Street 6 houses Industrial building Garage 2 sheds Swimming pool Tennis court	Elevation 55 55 55 45 45 45 45 45 45

Geologic Conditions:

Both abutments are poorly graded sand and gravel outwash with some silt. Depth to bedrock in the foundation is estimated to be from 50 to 60 feet. Waterholding capabilities appear to be poor. Leakage is expected through both abutments and the foundation. Pervious borrow material for dam construction was located near the site; impervious material was not located.

Engineering Notes:

The right abutment is recommended for the excavated emergency spillway location.

POTENTIAL SITE SS-2803

Location:

On Furnace Brook about 800 feet upstream from School Street in Marshfield, Mass.

Duxbury, Mass. USGS quadrangle

Latitude: 42°07'13" Longitude: 7.0°44'31"

POTENTIAL SITE SS-2803 (continued)

Facilities	Facility	Elevation
Affected:	House	120
	Pine Street	120
	3 houses	115
	Church and rectory	115
	2 sheds	115
	House	110
	Commercial building	110
	2 houses	105
	Main Street-Route 3-A	105
	Shed	105
	2 houses	100
	2 sheds	100
	House	95
	shed	95

Geologic Conditions:

Both abutments are poorly graded sand and gravel outwash. Depth to bedrock in the foundation is estimated to be from 50 to 60 feet. Waterholding capabilities appear to be poor. Leakage is expected through both abutments and the foundation. Pervious borrow material for dam construction was located near the site; impervious material was not located.

Engineering Notes:

The right abutment is recommended for the excavated emergency spillway location.

POTENTIAL SITE SS-2804

Location:

On an unnamed tributary to Chandlers Pond and the South River about 2300 feet upstream from Mt. Skirgo Street in Marshfield, Mass.

Duxbury, Mass. USGS quadrangle

Latitude: 42°05'55" Longitude: 70°43'53"

Facilities	Facility	Elevation
Affected:	8 houses	85
	Swimming pool	85
	2 houses	80
	Industrial building	80

Geologic Conditions:

Both abutments are poorly graded sand and gravel outwash. Depth to granite bedrock in the foundation is estimated to be from 15 to 25 feet. Waterholding capabilities appear to be poor. Leakage is expected through the left abutment. Pervious borrow material for dam construction was located near the site; impervious

material was not located.

Engineering Notes:

The right abutment is recommended for the excavated emergency spillway location.

POTENTIAL SITE SS-2805

Location:

On Keene Brook about 200 feet upstream from Union Street in Duxbury, Mass.

Hanover, Mass. USGS quadrangle

Latitude: 42°04'21" Longitude: 70°45'21"

Facilities Affected:

None below elevation 70.

Geologic Conditions: Both abutments are poorly graded sand and gravel outwash. Depth to bedrock in the foundation is estimated to be from 50 to 60 feet. Waterholding capabilities appear to be poor. Leakage is expected through both abutments and the foundation. Pervious borrow material for dam construction was located near the site; impervious material was not located.

Engineering Notes:

The left abutment is recommended for the excavated emergency spillway location.

POTENTIAL SITE SS-2806

Location:

On Furnace Brook at Furnace Pond about 1400 feet downstream from Furnace Street in Marshfield, Mass.

Duxbury, Mass. USGS quadrangle

Longitude: 70°43'36" Latitude: 42⁰06'19"

Facilities	3
Affected:	

Facility	Elevation
House	80
2 sheds	80
3 houses	75
4 houses	70
VFW Hall	70
Main Street-Route 3-A	70
5 houses	65
Shed	65
3 houses	60
Furnace Street	60
House	55
Shed	55
Pumping station	45

POTENTIAL SITE SS-2806 (Furnace Pond) (cont'd)

Geologic Conditions:

Both abutments are poorly graded sand and gravel outwash. Depth to bedrock in the foundation is estimated to be from 70 to 80 feet. Waterholding capabilities appear to be poor to fair. Leakage is expected through both abutments and the foundation. Pervious borrow material for dam construction was located near the site; impervious material was not located.

Engineering Notes:

The right abutment is recommended for the excavated emergency spillway location.

POTENTIAL SITE SS-2807

Location:

On Bares Brook about 800 feet upstream from Summer Street in Marshfield, Mass.

Scituate, Mass. USGS quadrangle

Latitude: 42⁰08'53" Longitude: 70⁰44'00"

Facilities Facility Elevation
Affected: 2 houses 95
Garage 95
House 90

Geologic Conditions:

Both abutments are poorly graded sand and gravel outwash. Depth to granite bedrock in the foundation is estimated to be from 40 to 50 feet. Waterholding capabilities appear to be poor. Leakage is expected through both abutments and the foundation. Borrow material for dam construction was located near the site.

Engineering Notes:

The right abutment is recommended for the excavated emergency spillway location.

SUMMARY DATA FOR POTENTIAL UPSTREAM RESERVOIR SITES

STUCY AREA-SOUTH SHCRE ***********************************

SUMMARY DATA FOR POTENTIAL UPSTREAM RESERVOIR SITES

STUBY AREA-SOUTH SHORE			STUDY AREA-SOUTH	REA-SI	OUTH SHORE	JRE			SUBMATERS	SUBMATERSHED	SOUTH RIVER	IVER				
BENEFICIAL POOL		BENEF I	BENEFICIAL POOL	ب			E E E R	EMERGENCY S	SPILLWAY	>	DESIGN HIGH WATER	TER +	D,	DAM		* SAFE * YIELD
ELEV (MSL) /	STORAGE AC FT I	AGE In	COST PER AC FT (\$)	AREA (AC)	COST/ SURF AC (\$)	DEPTH AT DAM (FT)			AGE REST IN	COST * PER * AC FT *	ELEV (MSL)	AREA *	TOP ELEV (MSL)	HGT FT	FILL VOL (1000 CY)	*PERCENT *CHANCE * (MGD)
SITE RATING (3) STREAM WATER QUALITY (2804 AT ING	(3)	**************************************	MATER	**************************************	****** 384 AC TY (B)	USGS CI 100-YR PI	######################################	**************************************	ASS ***********************************	* * * * * * * * * * * * * * * * * * *	***** TITUD = 5	******* E 42-05-55 •70 IN, PE	* M M	**************************************	70-43-53 121 CFS
43.7 58.5 63.5 70.8 76.5	0 100 158 274 448		5300 4040 3120 2570	110 113 40	51040 48010 42620 28660	36.5 * * * 36.5 * 36.5 *	61.0 61.0 66.0 73.3 79.0	E 133 E 133 E 198 E 340 E 563	4.1 4.1 6.1 0 10.6 3 17.6	3630 * 3990 * 3230 * 25520 * 2050 * 3	63.3 63.4 68.4 75.6 81.1	13 13 34 56 8 8	66.3 66.5 71.4 78.6 84.1	26 26 31 39 44	37 38 56 94 131	* * * * * * * * * * * * * * * * * * *
**************************************	2805 AT ING		**************************************	13 SQ	**************************************	531 AC TY (B)	USGS Q1 100-YR P	QUAD-HAND	######################################	ASS GN STORM	*	**************************************	************* E 42-04-21 LONGITU •70 IN, PEAK FLUW	***** 1 LON EAK FL	**************************************	**************************************
53.3 58.4 60.0 61.1 62.3	0 100 155 210 266	0 0 0 0 4 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	4310 3250 2700 2400	6 W 4 4 W 0 W 0 W 0 W 0 W 0 W 0 W 0 W 0 W	13240 12490 11910 11750	1.2 6.4 8.0 9.2 10.3	660 600 600 600 600 600 600 600 600 600	E 184 E 206 E 285 E 355 F 426	4 4 9 8 6	1820 * 2100 * 1770 * 1600 * 1490 *	62.8 63.4 64.9 66.1 67.1	57 61 70 78 84 84 84	65.8 66.4 67.9 69.1 70.8	14 16 17 19	16 18 23 29 38	* * * * * * * * * * * * * * * * * * *
**************************************	2806 ATING	(3)	**************************************	1 SQ	****** MI = 1	******* 158 AC TY (B)	USGS C 100-YR P	**************************************	KBURY AY DESIGN	**************************************	*	**************************************	**************************************	***** 9 LON EAK FL	.*************************************	******** 70-43-36 567 CFS
45.5 54.0 59.6 66.1	0 100 309 726 1351	0.0 1.0 3.2 7.5 14.0	11550 4930 2580 1650	25 48 82 114	4636C 3198C 2278C 1951C	5.5 14.0 19.7 26.2	61.1 56.5 62.1 68.6 75.0	E 401 E 191 E 461 E 965 E 1664	4.1 2.0 1 4.8 1 4.8 4 17.2	3060 * 6040 * 3300 * 1940 * 1340 *	63.5 59.0 64.6 71.1	68 * 45 * 108 * 1108 * 1137 *	56.6 62.0 67.6 74.1 80.0	22 22 28 40 40	44 29 48 78 113	* * * * * * * * * * * * * * * * * * *
**************************************	(1) (0)	(1) CCSTS ARE	**************************************	**************************************	**************************************	******* S* DESIGN	**************************************		**************************************	**************************************	*	* *	*	*	* * * *	*

(2) EMERGENCY SPILLWAY STORAGE AND COSTS ARE BASED ON TOTAL STORAGE, INCLUDING BENEFICIAL POOL.

(3) EMERGENCY SPILLWAY TYPE CODE— C=CCNCRETE CHUTE, D=CCNCRETE DROP, E=EXCAVATED, T= TWO SPILLWAYS, N= NONE

(4) TABULAR DATA ARE BASED ON PRELIMINARY INFORMATION. FIGURES SHOWN ARE PRIMARILY FOR COMPARISON PURPOSES.

(5) ELFVATIONS ARE SHOWN TO THE NEAREST 0.1 FCOT TO SHOW VARIATION BETWEEN DEVELOPMENTS ONLY, AND ARE NOT TO BE

CONSIDERED ACCURATE TO THAT UEGREE.

** DO NOT USE FOR FINAL SITE SELECTION OR LAND ACQUISITION. **

SUMMARY DATA FOR POTENTIAL UPSTREAM RESERVOIR SITES

STUDY AREA-SOUTH SHORE		STUDY	AREA-S	STUDY AREA-SOUTH SHORE	IORE				S	UBWAT	ERSHED	SUBWATERSHED SOUTH RIVER	RIVER	1	1	1		1	1
	BENEF	BENEFICIAL POOL)OL				EME	EMERGENCY SPILLWAY	SPIL	ILLWAY		DESIGN HIGH WATER	GN			DAM		* SAFE * YIELD	<u>س</u>
COST COST DEPT	STORAGE	COST PER	AREA	COST/ SURF	COST/ DEPTH SURF AT		CREST ELEV	STC	STORAGE AT CREST		COST *	ELEV	•	**************************************	TOP ELEV	HGT	FILL	,	95 ENT CE
(MSL) AC FT IN (\$) (AC) (\$) (FT)	Z	(\$)	(s) (AC) (s)	(3)	(FT)		(MSL)	AC F	AC FT IN		(\$)	(MSL) (AC) + (MSL)	(AC)		HSL)	FT	(X)		(0)
SITE-SS-2807 SITE RATING (3)	(3)	DA= 0. STREA	58 SQ	DA= 0.58 SQ MI = 371 AC STREAM WATER QUALITY (B)	371 AC (TY (B)	=	USGS 00-YR	QUAD-S	CITUA PWY D	TE, MA	USGS QUAD-SCITUATE, MASS 100-YR PRIN SPWY DESIGN STORM	RUND	LATITUDE 42-08 JNOFF = 5.70 IN,	DE 4 5.70	LATITUDE 42-08-53 LONGITUC RUNOFF = 5.70 IN, PEAK FLOW =	3 LO	LATITUDE 42-08-53 LONGITUDE 70-44-00 FF = 5.70 IN, PEAK FLOW = 189 CFS	70-44-0 189 CFS	4-00 CFS
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(2) EMERGENCY SPILLWAY STORAGE AND COSTS ARE BASED ON TOTAL STORAGE, INCLUDING BENEFICIAL POOL.

(3) EMERGENCY SPILLWAY TYPE CODE- C=CONCRETE CHUTE, D=CONCRETE DROP, E=EXCAVATED, T= TWO SPILLWAYS, N= NONE

(4) TABULAR DATA ARE BASED ON PRELIMINARY INFORMATION. FIGURES SHOWN ARE PRIMARILY FOR COMPARISON PURPOSES.

(5) ELEVATIONS ARE SHOWN TO THE NEAREST 0.1 FOOT TO SHOW VARIATION BETWEEN DEVELOPMENTS ONLY, AND ARE NOT TO BE
CONSIDERED ACCURATE TO THAT DEGREE.

** DO NOT USE FOR FINAL SITE SELECTION OR LAND ACQUISITION. **

Existing Site SS-2806 (Furnace Pond)

Location:

On Furnace Brook about 1,400 feet downstream from Furnace Street in Marshfield, Mass.

Duxbury, Mass. USGS quadrange

Latitude: 42°06'20" Longitude 70°43'38"

Surface Area Height of Drainage Area (Acres) Dam (Ft.) (Acres) (Sq. Miles) 1,150 1.80

for

Potential Please refer to Site Data and Design Summary Table for Potential Site SS-2806.

Expansion: Remarks:

The dam is an earthfill structure with vertical slopes and stone walls both up and downstream. The principal spillway is a concrete weir about 15 feet long and 2 feet deep. The emergency spillway is a vegetated channel on the left abutment. The principal spillway is not operating as water is flowing under the concrete. Large trees are growing on the

The pond is owned by the town of Marshfield - Conservation Ownership Commission and is used for wildlife habitat. and Use:

Existing Site SS-2808 (Murdocks Pond)

On Macombers Creek about 900 feet downstream from Summer Location: Street in Marshfield, Mass.

Scituate, Mass. USGS quadrangle

Latitude: 42⁰09'05"

Longitude: 42⁰43'45"

Surface Area Height of Drainage Area
(Acres) Dam (Ft.) (Acres) (Sq. Miles)

10 1.64

for Expansion:

Potential Topography limits any significant increase in surface area. Several streets and houses would be affected by expansion.

Remarks:

Damon Point Road forms the dam. The spillway is a concrete hox drop inlet. Flow is controlled with flashboards. A stone box culvert carries flow through the road embankment. The spillway inlet is about 4 feet wide, 7 feet long and has a 5 foot drop to the structure floor.

The pond is owned by the town of Marshfield and is used for Ownership recreation. and Use:

Existing Site SS-2810 (Parsons Pond)

On Furnace Brook at Old Plain Street in Marshfield, Mass. Location:

Duxbury, Mass. USGS quadrangle

Latitude: 42°05'49" Longitude: 70°43'23"

Surface Area
(Acres)

Bam (Ft.)

Contract Area
(Acres)

Contract Area
(Acres)

Contract Area
(Sq. Miles)

Contract Area
(Sq. Miles)

Contract Area
(Sq. Miles)

Contract Area
(Sq. Miles)

for

Potential Raising the pond level by 15 feet would create a 50 acre pool. Sandy soils may limit expansion potential.

Expansion:

Remarks: Old Plain Street forms the dam. The spillway is a concrete weir with flashboard control. The weir is about 4 feet long and 4 feet deep. Flow passes under Old Plain Street in a 4 foot by 5 foot concrete box culvert. There is about one foot of freeboard between the water level and Old Plain Road.

The pond is owned by the town of Marshfield - Conservation Ownership Commission and is used for wildlife habitat. and Use:

Existing Site SS-2811 (Chandlers Pond)

On the South River about 600 feet upstream from Mt. Skirgo Location: Street in Marshfield, Mass.

Duxbury, Mass. USGS quadrangle

Latitude: 42⁰05'33"

Longitude: 70^o43'23"

Surface Area Height of Drainage Area
(Acres) Dam (Ft.) (Acres) (Sq. Miles)

4 5,600 8.75

Potential Raising the pond level by 10 feet would create a 90 acre pool. for Three streets and several houses would be affected. Expansion:

Remarks: The dam is an earthfill structure. The spillway is a series of three bays, each 4 feet wide and 2 feet deep with provision for flashboards.

Ownership The pond is owned by Camp Millbrook and is used for recreation. and Use:

Existing Site SS-2812 (Keene Pond)

On Keene Brook about 1,200 feet upstream from Keene Street Location: in Duxbury, Mass.

Hanover, Mass. USGS quadrangle

Latitude: 42^o04'42" Longitude: 70^o45'02"

Surface Area (Acres)

Height of Dam (Ft.)

Drainage Area (Acres) 650

Potential Raising the pond level by 5 feet would create a 30 acre pool. Camp Wing buildings might be affected. Sandy soils may limit Expansion: expansion potential.

The dam is an earthfill structure. The downstream face is Remarks: constructed of concrete and rubble. The principal spillway is a gate-controlled 30-inch boiler-plate pipe. The emergency spillway is another 30-inch boiler-plate pipe. Leakage was noted near an old spillway which has been covered with fill material. Trees are growing on the dam. Erosion has

occurred on the dam.

The pond is owned by Camp Wing and is used for recreation. Ownerhip and Use:

Existing Site SS-2813

On Green Harbor River about 4,400 feet upstream from Location: Webster Street in Marshfield, Mass.

Duxbury, Mass. USGS quadrangle

Latitude: 42°04'21"

Longitude: 70°41'31"

Surface Area (Acres)

Height of <u>Dam (Ft.)</u>

Drainage Area (Acres) 1,350

Potential Sandy soils may limit expansion potential. for Expansion:

Existing Site SS-2813 (cont'd)

Remarks:

The dam is an earthfill structure. The spillway is a concrete weir with flashboard control. There is also a side channel controlled by flashboards which delivers water to a cranberry bog. The dam is constructed of sandy material and seepage was noted along the downstream toe in many places.

Ownership and Use:

The pond is owned by John Garretson and is used for cranberry bog irrigation.

Existing Site SS-2814 (Wrights Reservoir)

Location: On Green Harbor River about 2,700 feet upstream from Enterprise Street in Duxbury, Mass.

Duxbury, Mass.USGS quadrangle

Latitude: 42^o03'43" Longitude: 70^o42'12"

Surface Area Height of Drainage Area (Acres) Dam (Ft.) (Acres) (Sq. Miles) 500 0.78

Potential

Sandy soils may limit expansion potential.

for

Expansion:

Remarks:

The dam is an earthfill structure. The spillway is a concrete weir with flashboard control which outlets through a concrete pipe. The upstream slope of the dam has eroded due to wave action.

Ownership and Use:

The reservoir is owned by Lawrence Pink and is used for recreation and cranberry bog irrigation.



SS-2806 Furnace Pond



SS-2810 Parsons Pond



SS-2811 Chandlers Pond

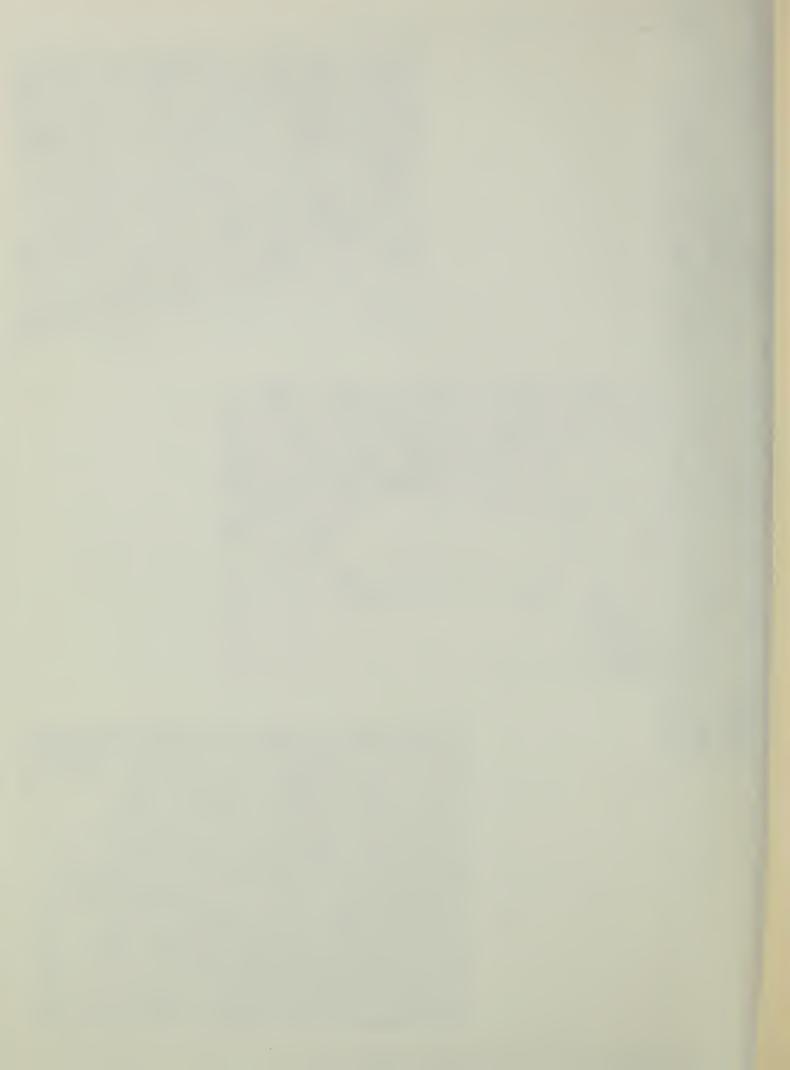


SS-2813 Garretson Bog Reservoir



SS-2814 Wright Reservoir

EXISTING RESERVOIRS SUBWATERSHED SS-28 SOUTH RIVER





SS-2729



SS-2740



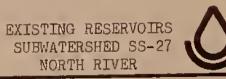


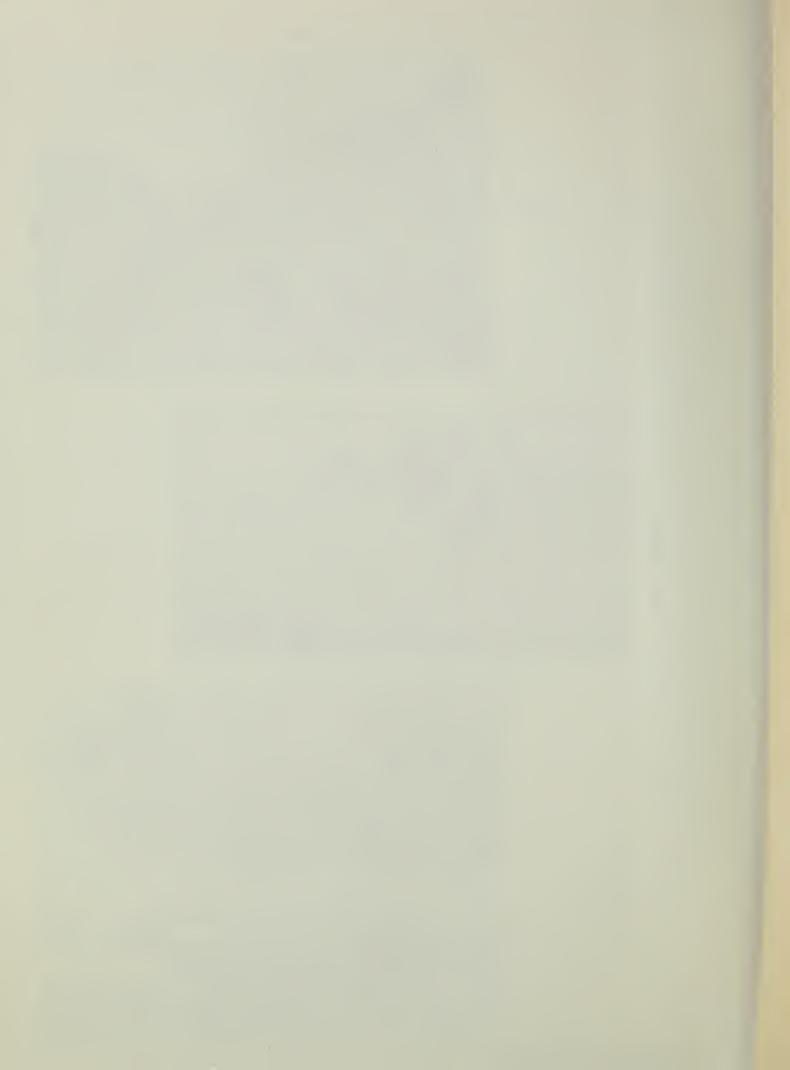


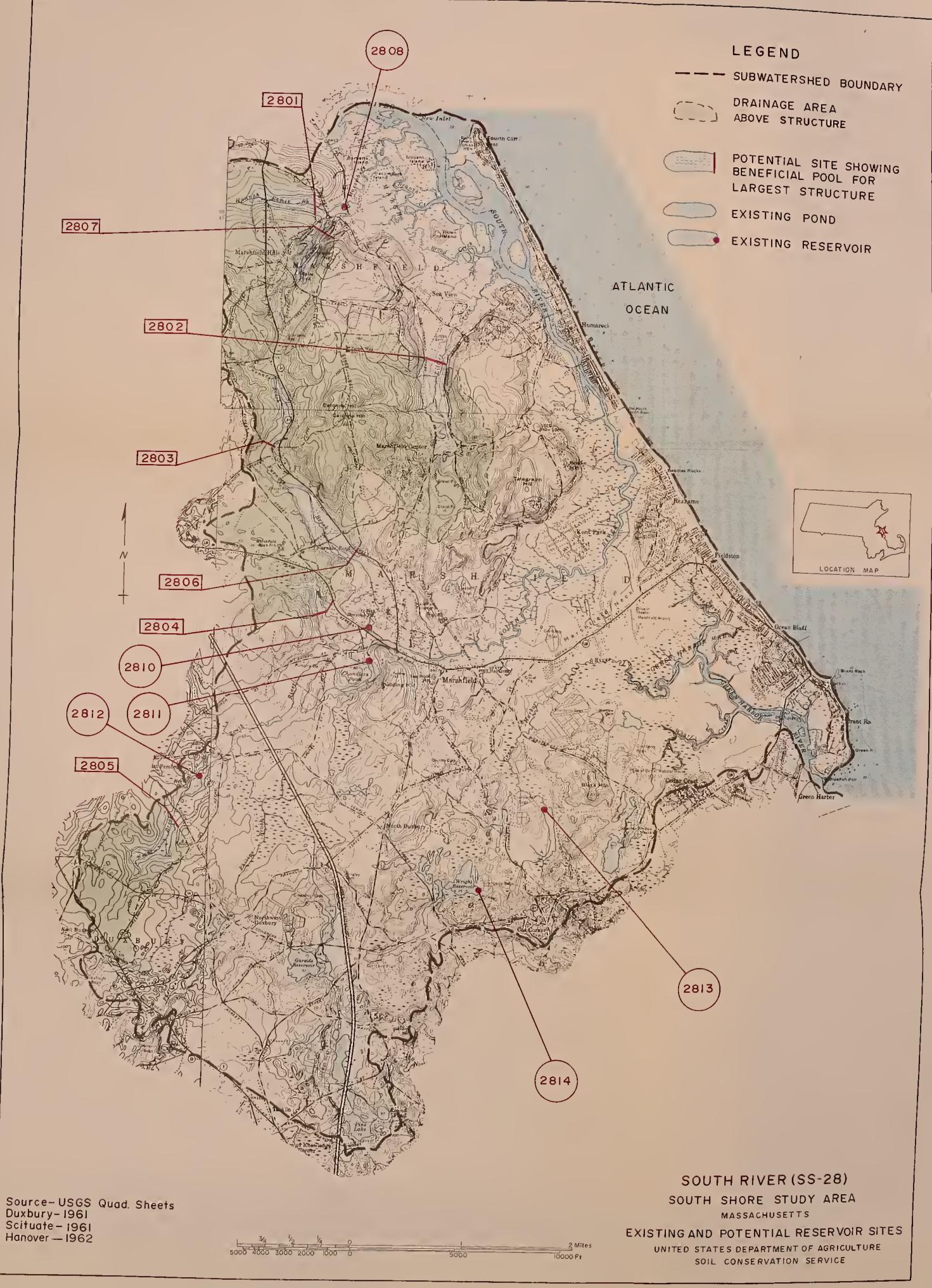
SS-2739 Mill Pond

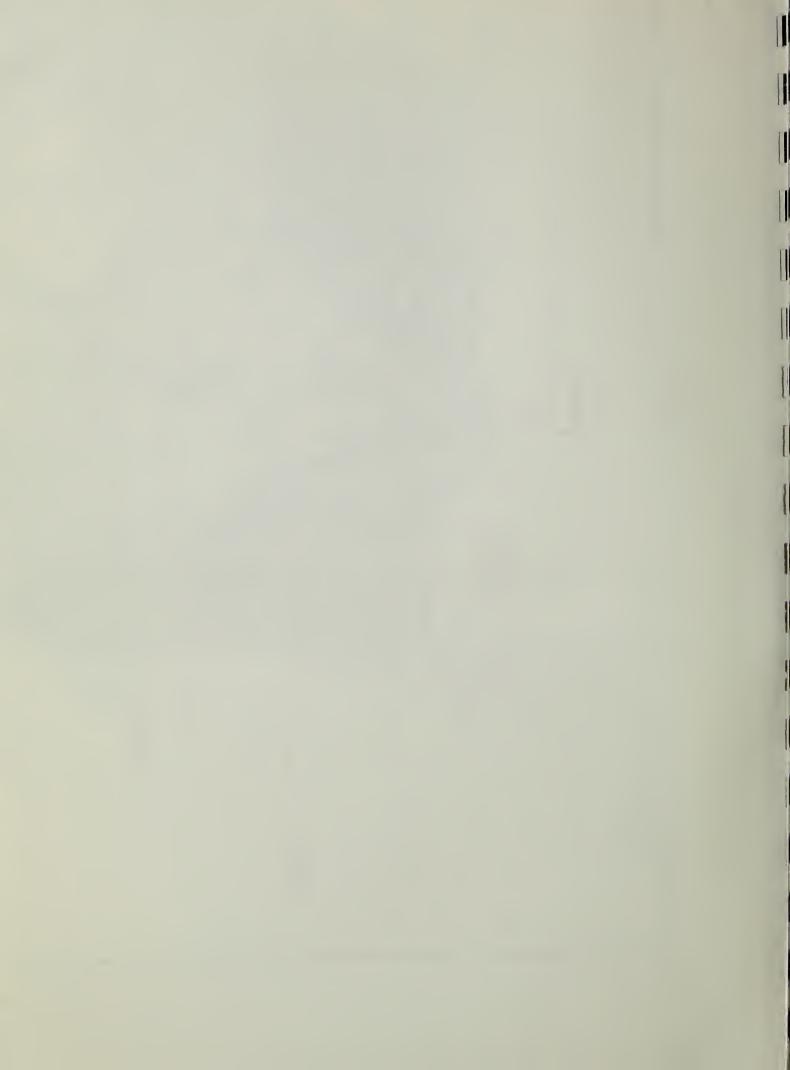


SS-2730 Old Oaken Bucket Pond









SOUTH SHORE STUDY AREA SITE DATA FOR

Subwatershed SS-29, Jones River

This subwatershed covers about 28,341 acres in Duxbury, Halifax, Kingston, Marshfield, Plymouth and Plympton, in Plymouth County. There is a U.S. Geological Survey stream gage on the Jones River in Kingston.

The major stream in this subwatershed is the Jones River, which originates in Silver Lake in Kingston and flows generally easterly into Kingston Bay. The main tributary to the Jones River is Pine Brook, which originates in Duxbury and flows southerly to the confluence in Kingston. Elevations range from a high of about 280 feet in Plymouth to tidewater in Kingston Bay. Geology in this subwatershed is characterized by granite bedrock overlain by 5 to 80 feet of outwash sand and gravel.

Five potential reservoir sites and 9 existing reservoirs were studied.

POTENTIAL SITE SS-2901

Location:

On West Brook about 200 feet upstream from Tremont Street in Duxbury, Mass.

Duxbury, Mass. USGS quadrangle

Latitude: 42°03'12" Longitude: 70°41'37"

Facilities	Facility	Elevation
Affected:	4 houses	35
	5 garages	35
	2 garages	30
	House	25
	Commercial building	25
	Water wells	20
	Garage	20

Geologic Conditions:

Both abutments are poorly graded sand and gravel outwash. Depth to granite bedrock in the foundation is not known but may be from 70 to 80 feet. Waterholding capabilities appear to be poor. Leakage is expected through both abutments and the foundation. Pervious borrow material for dam construction was located near the site; impervious material was not located.

Engineering Notes:

The right abutment is recommended for the excavated emergency spillway location.

POTENTIAL SITE SS-2903

Location:

On the Jones River about 4500 feet upstream from Wapping Road, Route 106, in Kingston, Mass.

Plympton, Mass. USGS quadrangle

Latitude: 41°59'54" Longitude: 70°45'50"

Facilities	
Affected:	

Facility	Elevation
21 houses	45
3 garages	45
11 houses	40
West Street	40
Grove Street	40
Shed	40
2 houses	35
2 garages	35
Swimming pool	45

Geologic Conditions: Both abutments are poorly graded sand and gravel outwash. Depth to granite bedrock in the foundation is estimated to be from 50 to 65 feet. Waterholding capabilities appear to be fair. Leakage is expected through both abutments and the foundation. Pervious borrow material for dam construction was located near the site; impervious material was not located.

Engineering Notes:

The left abutment is recommended for the excavated emergency spillway location. Inflow to the site may be sufficient to maintain a pool in spite of losses through the abutments and foundation.

POTENTIAL SITE SS-2904

Location:

On Smelt Brook about 2100 feet upstream from Route 3 in Kingston, Mass.

Plymouth, Mass. USGS quadrangle

Latitude: 41°58'26" Longitude: 70°43'09"

Facilities Affected:

Facility	Elevation
5 houses	115
Shed	115
5 houses	110
Garage	110
Raboth Road	105

Geologic
Conditions:

Both abutments are poorly graded sand and gravel outwash. Depth to bedrock in the foundation is estimated to be from 60 to 75 feet. Waterholding capabilities appear to be fair. There appears to be enough fine material to prevent excessive leakage through the abutments. Pervious borrow material for dam construction was located near the site; impervious material was not located.

POTENTIAL SITE SS-2904 (continued)

Engineering Notes:

The left abutment is recommended for the excavated emergency spillway location.

POTENTIAL SITE SS-2905

Location:

On Furnace Brook about 600 feet upstream from the Jones River in Kingston, Mass.

Plymouth, Mass. USGS quadrangle

Latitude: 41°59'26" Longitude: 70°44'42"

Facilities Affected:

Facility Pumphouse

Elevation 45

Geologic Conditions: Both abutments are poorly graded sand and gravel outwash probably underlain by bedrock. Depth to granite bedrock in the foundation is estimated to be from 5 to 15 feet. Waterholding capabilities appear to be poor. Leakage is expected through both abutments. Pervious borrow material for dam construction was located near the site; impervious material was not located.

Engineering Notes:

The right abutment is recommended for the excavated emergency spillway location. Waterholding capabilities might be improved if a cut-off to bedrock can be made.

POTENTIAL SITE SS-2906

Location:

On Fountainhead Brook about 2300 feet upstream from Wapping Road, Route 106, in Kingston, Mass.

Plympton, Mass. USGS quadrangle

Latitude: 41°59'10" Longitude: 70°45'46"

Facilities Affected:

Facility.
Utility poles

Elevation

Geologic Conditions: Both abutments are poorly graded sand and gravel outwash. Depth to granite bedrock in the foundation is estimated to be from 35 to 45 feet. Waterholding capabilities appear to be poor. Leakage is expected through both abutments and the foundation. Pervious borrow material for dam construction was located near the site; impervious material was not located.

Engineering Notes:

The right abutment is recommended for the excavated emergency spillway location.

SUMMARY CATA FOR POTENTIAL UPSTREAM RESERVOIR SITES

1 1 1 1 1 1	SAFE	PERCENT *CHANCE	++++++ 70-41-37 401 CFS 40.17 0.28 0.37 0.45	70-45-50 1579 CFS **** 0.21 1.83 2.99 4.28	**************************************	NONE ISES. IOT TO BE
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		TOP ELEV (MSL)	### 442-4 42-03-1 931-5 931-5 933-0	41-59- 50 IN: 48.2 49.9 48.5 50.0	**************************************	
RIVER	ER +	AREA *	**************************************	TITUDE	LATITUDE FF = 5.4 1 68 * 1 0 57 * 0 57 * 0 6 90 * 1 1 103 * 1	BENEF (ED) T= (RILY F) (ELOPME
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		CUST COST COST OEP ELEV STORAGE PER AREA SURF AT AC FT AC FT AC (MSL) AC FT IN (\$) (AC) (\$) (FT	SITE—SS_2901 SITE RATING (3) STREAM WATER QUALITY (1) 16.7 0 0.0 22.1 100 1.4 7530 46 1651C 10 23.7 189 2.5 4140 67 1166C 11 24.9 279 3.8 2890 83 965C 12 25.9 368 5.0 23C0 97 875C 13	CA= 16.13 SQ MI = 10323 STREAM WATER QUALITY 88 16280 121 13400 3370 285 12410 1 1960 466 8380 1 1340 806 5290 1	##### QUALI QUALI 221C 179C 930C 785C	COSTS ARE BASED ON 1973 S.C.S. DESIGN CR. EMERGENCY SPILLWAY STORAGE AND COSTS ARE EMERGENCY SPILLWAY TYPE COCE— C=CONCRETE TABULAR DATA ARE BASED ON PRELIMINARY IN ELEVATIONS ARE SHOWN TO THE NEAREST 0.1 ICONSICERED ACCURATE TO THAT CEGREE.
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STUDY	BENEFICIAL POOL	COST PER AC FT (\$)	DA= 1.38 SQ MI = STREAM WATER QUAL 4 7530 46 1651 4140 67 1166 2890 83 965 2300 97 875	DA= 16. STREA 3370 1960 1340	######################################	201
1 1 1 1	BENEFI	AGE IN	* (8) * (9) * (1) * (1) * (2) * (2) * (3) * (4) * (5) * (6) * (7) * (7) * (8) * (9) *	(2) 0.0 0.1 1.2 2.3 3.6	2904 2904 ATING (2) 0 0.0 100 1.9 137 2.5 212 3.9 286 5.3 323 6.0 ************************************	COSTS ARE E EMERGENCY S EMERGENCY S TABULAR DAI ELEVATIONS CONSIDERED
1 1 1 2 1		STORAGE AC FT I	RATING 100 189 279 368	SITE RATING 32.4 0 33.4 100 38.0 1048 40.5 1996 42.5 3184	######################################	(1) CO (2) EM (3) EM (4) TA (5) EL
3		ELEV (MSL)	SITE-S\$-2901 SITE-S\$-2901 SITE RATIN 16.7 22.1 100 23.7 180 24.9 27 24.9 27	SITE-SS-2903 SITE RATING (2) STREAM WATER QUALITY (32.4 0 0.0 888 4 33.4 100 0.1 16280 121 13400 5 38.0 1048 1.2 3370 285 12410 10 40.5 1996 2.3 1960 466 8380 12 42.5 3184 3.6 1340 806 5290 14	######################################	NOTES -

SUMMARY DATA FOR POTENTIAL UPSTREAM RESERVOIR SITES

5535

EMERGENCY SPILLWAY STORAGE AND COSTS ARE BASED ON TOTAL STORAGE, INCLUDING BENEFICIAL POOL.

EMERGENCY SPILLWAY STORAGE AND COSTS ARE BASED ON TOTAL STORAGE, INCLUDING BENEFICIAL POOL.

EMERGENCY SPILLWAY TYPE CODE- C=CONCRETE CHUTE, D=CCNCRETE DROP, E=EXCAVATED, T= TWO SPILLWAYS, N= NONE
TABULAR DATA ARE BASED ON PRELIMINARY INFORMATION. FIGURES SHOWN ARE PRIMARILY FOR COMPARISON PURPOSES.

ELEVATIONS ARE SHOWN TO THE NEAREST 0.1 FCOT TO SHOW VARIATION BETWEEN DEVELOPMENTS ONLY, AND ARE NOT TO BE
CONSIDERED ACCURATE TO THAT CEGREE.

** DO NOT USE FOR FINAL SITE SELECTION OR LAND ACQUISITION. **

Existing Site SS-2908 (Upper Chandler Pond)

Location:

On Pine Brook about 1,000 feet downstream from Valley Street on the Duxbury-Pembroke town line.

Hanover, Mass. USGS quadrangle

Latitude: 42^o02'03" Longitude: 70^o46'04"

Surface Area Height of Drainage Area (Acres) Dam (Ft.) (Acres) (Sq. Miles) 825 1.29

Potential for Expansion:

Expansion would create a large area of shallow water as the pond would expand into a wetland area.

Remarks:

The dam is an earthfill structure. The spillway is a sheet-steel riser with flashboard control. Two 36-inch pipes carry flow through the embankment. The spillway has two bays, each 4 feet wide and 2 feet deep. There is also a three-foot long weir with flashboards located near the right abutment.

Ownership and

Use:

The pond is owned by Ingrid, Lorrie, John and Eino Hill and is used for cranberry bog irrigation.

Existing Site SS-2909 (Lower Chandler Pond)

Location:

On Pine Brook at Lake Shore Drive in Pembroke, Mass.

Hanover, Mass. USGS quadrangle

Latitude: 42⁰01'40" Longitude: 70⁰46'03"

Surface Area
(Acres)

Dam (Ft.)

15

Height of Drainage Area
(Acres)
(Sq. Miles)
1,450
2.27

Potential for

Raising the pond level by 10 feet would create a 110-acre pool. Sandy soils may limit expansion potential.

Expansion:

Remarks:

Lake Shore Drive forms the dam the spillway is a corrugated metal half-round riser and metal pipe conduit. The riser is 6 feet in diameter; the conduit is 5 feet. The pond has about one foot or freeboard.

Ownership and

The pond is owned by the town of Pembroke and is used for water supply and recreation.

Use:

Existing Site SS-2910 (Reeds Mill Pond)

On Pine Brook at Route 27 in Kingston, Mass. Location:

Hanover, Mass. USGS quadrangle

Latitude: 42°00'50" Longitude: 70°46'14"

Surface Area Height of Drainage Area
(Acres) Dam (Ft.) (Acres) (Sq. Miles)
7 10 1,900 2.97

Potential for

Raising the pond level by 10 feet would create a 110-acre pool.

Expansion:

Remarks:

Route 27 forms the dam. Two culverts with flashboards serve as the spillway. Erosion has occurred at the spillway outlets.

Ownership

The pond is owned by Robert Davidson and is not used for a specific purpose.

and Use:

Existing Site SS-2911 (Dennetts Pond)

Location:

On the Jones River just upstream from Ring Road in Plympton Mass.

Plympton, Mass. USGS quadrangle

Latitude: 41°58'27" Longitude: 70°47'56"

Surface Area Height of Drainage Area
(Acres) Dam (Ft.) (Acres) (Sq. Miles)

12 1,125 1.76

Potential for

Expansion would create a large area of shallow water as the pond would expand to a large wetland area.

Expansion:

Remarks:

The dam is an earthfill structure with a vertical stone wall on the downstream face. The spillway is a concrete weir with provision for flashboards. The spillway has two bays; one 6.5 feet wide and the other 4.5 feet wide. The dam was leaking in several places.

Ownership

and Use: The pond is owned by John Osborn and is used for recreation.

Existing Site SS-2912 (Russell Pond)

Location:

On Furnace Brook between Elm Street and Indian Pond Road in Kingston, Mass.

Plymouth, Mass. USGS quadrangle

Latitude: 41°58'43" Longitude: 70°44'50"

Surface Area Height of Drainage Area (Acres) Dam (Ft.) (Acres) (Sq. Miles) 475 0.74

Potential

Topography limits any significant increase in surface area.

for

Expansion:

Remarks:

The dam is an earthfill structure. The downstream slope is nearly vertical. The spillway is a concrete weir about 3 feet long and 2 feet deep. There is a fish ladder adjacent to the spillway. Large trees are growing on the dam. Many

seepage areas were noted along the downstream toe.

Ownership and

The pond is owned by Horace Weston and is not used for a

specific purpose.

Use:

Existing Site SS-2913 (Stone Pond)

Location:

On an unnamed tributary to Plymouth Bay about 1,200 feet upstream from Route 3A in Plymouth, Mass.

Plymouth, Mass. USGS quadrangle

Latitude: 41°58'29" Longitude: 70°41'32"

Surface Area
(Acres)

Dam (Ft.)

4

Height of Drainage Area
(Acres)
(Sq. Miles)

0.26

Potential for

Topography limits any significant increase in surface area. The small drainage area limits expansion potential. Sandy

Expansion: soils may limit expansion potential.

Remarks:

The dam is an earthfill structure. The spillway is a concrete weir inlet about 6 feet long and a 30-inch cast iron conduit. A trash rack on the inlet restricts flow.

Ownership and

The pond is owned by the town of Plymouth and is used for

recreation.

Use:

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Existing Site SS-2914 (Spooner Pond)

On an unnamed tributary to Plymouth Bay at Route 3A in Location:

Plymouth, Mass.

Plymouth, Mass. USGS quadrangle

Latitude: 41°58'36" Longitude: 70°41'24"

Surface Area Height of Drainage Area
(Acres) Dam (Ft.) (Acres) (Sq. Miles)

5 75 0.12

Potential for

The small drainage area limits expansion potential. The pond is surrounded by streets and buildings which would be affected

Expansion: by expansion of the pond.

Remarks:

Route 3-A forms the dam. The spillway is a concrete weir inlet with flashboards and a concrete block conduit. The inlet weir is about 6 feet long The conduit is 4 feet wide and 4 feet deep. Concrete in the spillway is broken in places.

Ownership and

The pond is owned by Saltwater Trust and is used for industrial purposes.

Use:

Existing Site SS-2915 (Foundry Pond)

Location:

On Smelt Brook about 700 feet upstream from Route 3-A in Kingston, Mass.

Plymouth, Mass. USGS quadrangle

Latitude: 41°59'08" Longitude: 70°42'37"

Surface Area Height of Drainage Area (Acres) Dam (Ft.) (Acres) (Sq. Miles) 1,000 1.56

Potential for

Raising the pond level by 20 feet would create a 30-acre pool.

Expansion:

Remarks:

The dam is an earthfill structure with a mill building at the downstream face. There is some riprap on the upstream slope. The spillway is a concrete weir and flume with flashboard control.

Ownership and

The pond is owned by C. Drew Company and is used for industrial purposes.

Use:

Existing Site SS-2916 (Mill Pond)

Location: On Island Creek at Tremont Street in Duxbury, Mass.

Duxbury, Mass. USGS quadrangle

Latitude: 42°01'00"

Longitude: 70°42'40"

Surface Area (Acres)

Height of Dam (Ft.)

Drainage Area

(Acres) (Sq. Miles) 900 1.41

Potential for

Raising the pond level by 15 feet would create a 110-acre pool. Tobey Garden Street and 5 houses would be affected.

Expansion: Sandy soils may limit expansion potential.

Remarks:

Route 3 A (Tremont Street) forms the dam. The spillway is a 4-foot by 5-foot concrete culvert with flashboard control. The downstream channel is a series of 6-inch high concrete steps which serve as a fish ladder.

Ownership and

The pond is owned by the town of Duxbury and is used for

fire protection and recreation.

Use:



SS-2908
Upper Chandler Pond

SS-2912 Russell Pond



SS-2909 Lower Chandler Pond

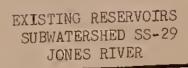


SS-2911 Dennetts Pond



SS-2913 Stone Pond











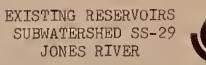
SS-2914
Spooner Pond



SS-2915
Foundry Pond

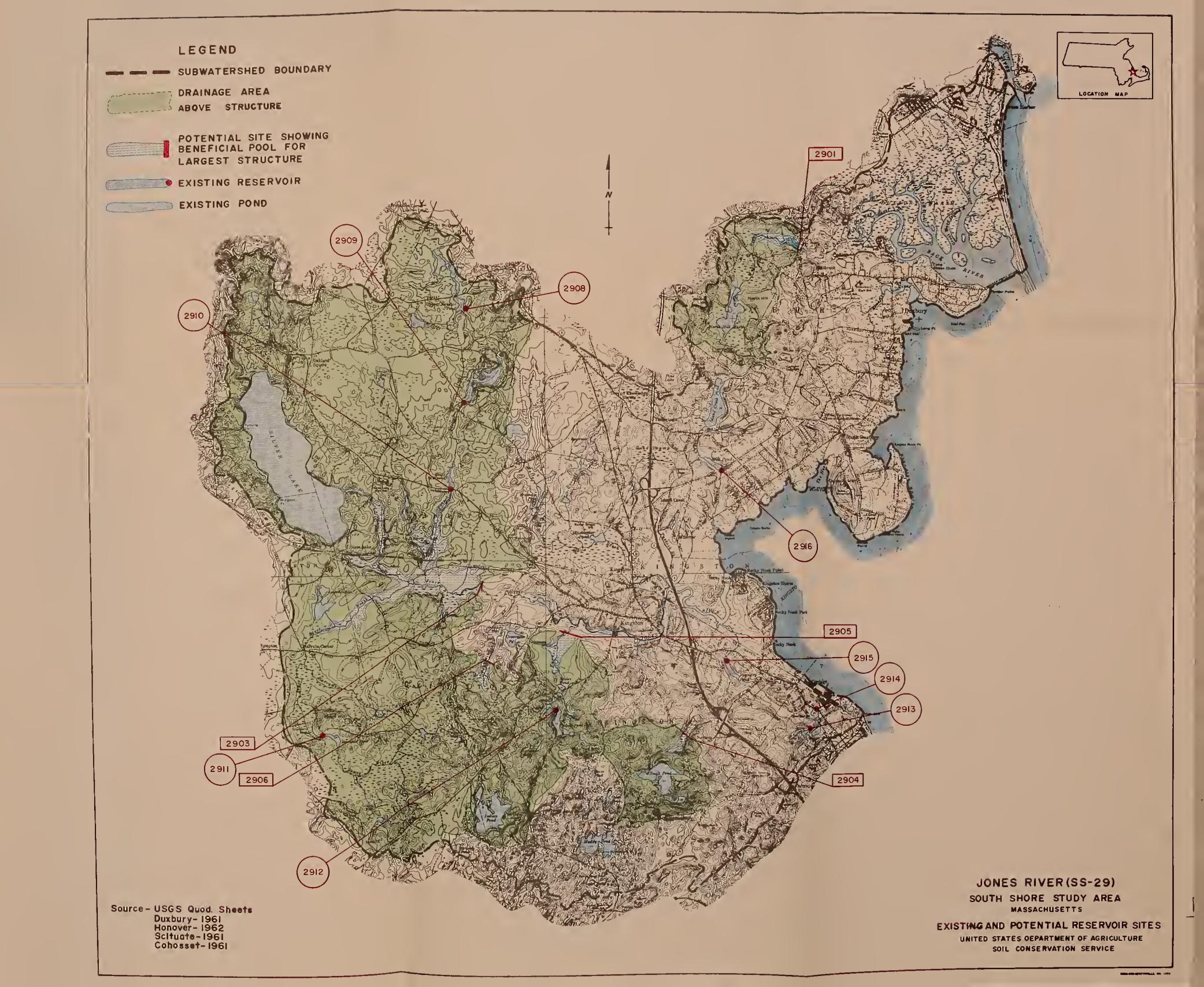


SS-2916 Mill Pond











SOUTH SHORE STUDY AREA SITE DATA FOR

Subwatershed SS-30, Eel River

This subwatershed covers about 27,700 acres in Kingston and Plymouth, in Plymouth County. There is a U.S. Geological Survey Stream gage on the Eel River in Plymouth.

Major streams in this subwatershed include the Eel River, Town Brook and Beaver Dam Brook. The Eel River originates in Plymouth and flows northeasterly into Plymouth Bay. Town Brook originates in Billington Sea in Plymouth and flows northeasterly into Plymouth Harbor. Beaver Dam Brook also originates in Plymouth and flows northerly to Cape Cod Bay. Elevations range from a high of about 395 feet on Manomet Hill to tidewater in Cape Cod Bay.

Geology in this subwatershed is characterized by granite bedrock overlain by 40 to 80 feet of outwash sand and gravel.

Seven potential reservoir sites and 9 existing reservoirs were studied.

POTENTIAL SITE SS-3001

Location:

On the Eel River at Russell Millpond about 1300 feet upstream from Route 3 in Plymouth, Mass.

Plymouth, Mass. USGS quadrangle

Latitude: 41°55'01" Longitude: 70°31'38"

Facilities	<u>Facility</u>	Elevation
Affected:	3 houses	80
	5 garages	80
	2 houses	75
	3 garages	75
	2 houses	70
	2 garages	70
	Gravel road	70
	6 houses	65
	Garage	65
	Utility towers	65
	2 houses	60
	2 garages	60

Geologic Conditions: Both abutments are poorly graded sand and gravel outwash. Depth to bedrock in the foundation is estimated to be from 60 to 75 feet. Waterholding capabilities appear to be poor. Leakage is expected through both abutments. Pervious borrow material for dam construction was located near the site; impervious material was not located.

Engineering Notes:

Preliminary design information indicates that a concrete monolithic conduit emergency spillway may be needed at this site.

Inflow to the site may be sufficient to maintain a pool in spite of losses through abutments and the foundation.

POTENTIAL SITE SS-3002

Location:

On the Eel River at the pond about 700 feet upstream from Sandwich Road in Plymouth, Mass.

Manomet, Mass. USGS quadrangle

Latitude: 41°55'27" Longitude: 70°37'18"

Facilities Affected: Route 3

Elevation

Geologic Conditions: Both abutments are poorly graded sand and gravel outwash. Depth to bedrock in the foundation is estimated to be from 50 to 60 feet. Waterholding capabilities appear to be poor. Leakage is expected through both abutments. Pervious borrow material for dam construction was located near the site; impervious material was not located.

Engineering Notes:

The left abutment is recommended for the excavated emergency spillway location. Inflow to the site may be sufficient to maintain a pool in spite of losses through the abutments.

POTENTIAL SITE SS-3003

Location:

On the Eel River about 1400 feet upstream from River Street in Plymouth, Mass.

Manomet, Mass. USGS quadrangle

Latitude: 41°55'58" Longitude: 70°37'07"

Facilities	Facility	Elevation
Affected:	2 houses	50
	4 houses	45
	Doten Road	45
	2 houses	40
	4 garages	40
	2 houses	35
	3 garages	35
	4 houses	30
	2 garages	30
	12 houses	25
	5 garages	25 25
	Garage	20
	Clifford Road	20
	Sandwich Road	15

Geologic Conditions: Both abutments are poorly graded sand and gravel outwash. Depth to bedrock in the foundation is estimated to be from 70 to 80 feet. Waterholding capabilities appear to be poor. Leakage is expected through both abutments. Pervious borrow material for dam construction was located near the site; impervious material was not located.

POTENTIAL SITE SS-3003 (continued)

Engineering Notes:

The left abutment is recommended for the excavated emergency spillway location. Inflow to the site may be sufficient to maintain a pool in spite of losses through the abutments.

POTENTIAL SITE SS-3004

Location:

On an unnamed tributary to the Eel River about 6000 feet upstream from Forge Pond Dam in Plymouth, Mass.

Manomet, Mass. USGS quadrangle

Latitude: 41°54'15" Longitude: 70°36'15"

Facilities	Facility	Elevation
Affected:	Beaver Dam Road	100
	Garage	95
	Sandwich Road	85
	House	80

Geologic Conditions:

Both abutments are poorly graded sand and gravel outwash. Depth to bedrock in the foundation is estimated to be from 40 to 50 feet. Waterholding capabilities appear to be poor. Leakage is expected through both abutments. Pervious borrow material for dam construction was located near the site; impervious material was not located.

Engineering Notes:

The left abutment is recommended for the excavated emergency spillway location. Inflow to the site may be sufficient to maintain a pool in spite of losses through the abutments.

POTENTIAL SITE SS-3006

Location:

On an unnamed tributary to the Eel River about 1200 feet upstream from Clifford Road in Plymouth, Mass.

Manomet, Mass. USGS quadrangle

Latitude: 41°55'24" Longitude: 70°36'40"

Facilities	Facility	Elevation
Affected:	House	60
	2 houses	55
	2 houses	50
	2 garages	50
	Doten Road	45
	2 garages	40
	Garage	35

POTENTIAL SITE SS-3006 (continued)

Geologic Conditions:

Both abutments are poorly graded sand and gravel outwash. Depth to granite bedrock in the foundation is estimated to be from 70 to 80 feet. Waterholding capabilities appear to be poor. Leakage is expected through both abutments and the foundation. Pervious borrow material for dam construction was located near the site; impervious material was not located.

Engineering Notes:

The right abutment is recommended for the excavated emergency spillway location. Inflow to the site may be sufficient to maintain a pool in spite of losses through the abutments and foundation.

POTENTIAL SITE SS-3007

Location: On Indian Brook about 1100 feet upstream from Route 3A in

Plymouth, Mass.

Manomet, Mass. USGS quadrangle Latitude: 41°53'11" Longitude 70°32'31" Longitude:

Facilities Elevation Facility Affected: 50 3 cottages 45 House 45 Garage 70 7 garages 40 2 water wells

Geologic Conditions: Both abutments are fine sand and gravel. Depth to bedrock in the foundation is estimated to be from 50 to 60 feet. Waterholding capabilities appear to be fair. Pervious borrow material for dam construction was located near the site; impervious material was not located.

Engineering Notes:

The left abutment is recommended for the excavated emergency spillway location. Inflow to the site may be sufficient to maintain a pool in spite of losses through the abutments.

POTENTIAL SITE SS-3008

On an unnamed tributary to the Eel River, about 3000 feet Location: upstream from the Russell Mill Pond dam in Plymouth, Mass.

Plymouth, Mass. USGS quadrangle

Longitude: 70°32115" Latitude: 41°55'11"

Elevation Facilities Facility 75 Affected: House 75 70 Garage Garage 65 House 65 Utility poles

Geologic Conditions:

Both abutments are poorly graded sand and gravel outwash. Depth to granite bedrock in the foundation is estimated to be from 60 to 75 feet. Waterholding capabilities appear to be fair. Leakage is expected through both abutments. Pervious borrow

material for dam construction was located near the site;

impervious material was not located.

The left abutment is recommended for the excavated emergency Engineering Notes: spillway location.

SUMMARY DATA FOR POTENTIAL UPSTREAM RESERVOIR SITES

			STUDY A	REA-S	STUDY AREA-SOUTH SHCRE	PRE			SUBWA	SUBWATERSHED-EEL	-EEL RIVER	ER	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	1 1 1 1 1	1	1 1 1 1 1
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**************************************	STOF AC FT	**************************************	######################################	AREA (AC)	****** COST/ SURF AC AC (\$)	DEPTH AT CAM	•	STORAGE AT CREST AC FT IN	AGE REST IN	* * * * *	ELEV AR	E A	TOP ELEV (MSL)	F1LL HGT VOL (1000 FT CY)		
**************************************	3001 ATING	(2)	**************************************	14 SQ	**************************************	******* 498 AC TY (B)	USGS (**************************************	MOUTH,	**************************************	LATITUD RUNOFF = 5	**************************************	41-55-	**************************************	* Ш	70-31-38 656 CFS
40.0 57.3 63.3 67.6 71.0	500 957 1415 1872	0.0 4.0 7.6 111.3	3380 2020 1520 1260	56 94 121 148	2998C 2047C 1773C 1589C	21.2 27.2 31.5 35.0		0 11 11 11 0	4.1 7.8 11.5 11.5	NEHEN	6.1001	:	69.6 74.3 77.1 79.5	34. 441 444 1	71 * 95 * 1111 * 126 * 126 *	* * * * * * * * * * * * * * * * * * *
SITE RATING (2) STREAM WATER QUALITY	3002 ATING	(2)	######################################	7 SQ	**************************************	773 AC TY (B)	USGS (######################################	**************************************	SS SN STORM	LATITUDE RUNDFF = 5.1	TITUDE = 5.	41-55-27 10 IN, PE	41-55-27 LONGITUDE 0 IN, PEAK FLOW =		70-37-18 651 CFS
16.9 24.9 30.2 42.5	0 100 251 552 841		9930 5950 3280 2370	6 5 5 5 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6	46540 43100 36030 31810	6.8 14.8 20.2 27.4 32.5	38.2 24.9 40.8 42.9	E 613 T 122 E 760 E 953 T 863	40000	1920 * 8130 * 1960 * 1900 * 2310 *	40.5 443.3 47.4 41.4	388 388 492 493 494 494 494 494 494 494 494 494 494	7 6 6 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8	34 255 36 10 40	91 ** 37 ** 1114 ** 169 **	**** 0.18 0.41 0.74 0.98
SITE-SS-3003 SITE RATING (3003 AT ING		**************************************	4 SQ MATE	**************************************	******* 554 AC TY (B)	USGS (**************************************	10MET # MASS 4Y DESIGN S	15S NS STORM	LA RUNOFF	**************************************	41-55-58 10 IN, PEAK	**************************************	rube 7 = 19	70-37-07 1934 CFS
8.8 24.2 35.0 42.5	100 1546 4438 7548	0.0 0.2 2.8 8.1 13.7	23540 2630 1060 740	32 51 190 355 472	46360 21370 13300 11820	4.9 7.3 20.2 31.0 38.5	31.2 11.3 34.8 41.5 42.5	E 3329 T 182 E 4468 E 7157 T 7629	6.1 0.3 8.2 14.0	1010 * 12940 * 910 * 660 * 730 *	33.7 24.2 37.3 43.9 46.7	335 * * 391 * * 492 * * 535	42.9 30.5 43.4 49.8 50.0	39 26 39 46 1	88 * 40 * 90 * 125 * 126 *	***** 0.21 2.20 4.68 6.41
**************************************	(1) C(2)	CUSTS ARE EMERGENCY	######################################	ON 1973	1973 S.C.S. STORAGE AND	S. DESIGN	**************************************		**** COST OTAL	**************************************	**************************************	***** G BENE	ada .	P00L.		* * * * * *

(3) EMERGENCY SPILLWAY TYPE CODE— C=CONCRETE CHUTE, D=CONCRETE DROP, E=EXCAVATED, T= TWO SPILLWAYS, N= NONE
(4) TABULAR DATA ARE BASED ON PRELIMINARY INFORMATION. FIGURES SHOWN ARE PRIMARILY FOR COMPARISON PURPOSES.
(5) ELEVATIONS ARE SHOWN TO THE NEAREST 0.1 FOOT TO SHOW VARIATION BETWEEN DEVELOPMENTS ONLY, AND ARE NOT TO BE CONSIDERED ACCURATE TO THAT DEGREE.

** DO NOT USE FOR FINAL SITE SELECTION OR LAND ACQUISITION. **

1	LD CD	ENT ICE	**** 6-15 CFS	* * * * * * * * * * * * * * * * * * *	-36-40 7 CFS	**** 0.17 0.48 0.87 1.11	+**** -32-31 7 CFS	* * * * * * * * * * * * * * * * * * *	96
1	SAFE	* PERCENT *CHANCE * (MGD)	70-36-1 663 CFS	* 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	70-36-40 417 CFS	***** 0°17 0°48 0°87 1°11	70-32-3 1227 CFS	***************************************	***** NONE SES. OT TO
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1		FILL VOL (1000 CY)	* -		* -	40.6	LONGITUDE FLOW =		* W
		HGT FT	15 LONGI PEAK FLOW	223 26 40 40		20 116 25 43 46	1 LONGI EAK FLOW	22 23 24 24 28	LEWAY:
1	DAM		115 PE	11580	5-24 PE	w4w-0w	3-11 PE/	L 8 7 6 6	****** L POOL. SPILLW OMPARIS
		TOP ELEV (MSL)	DE 41-54- 5.00 IN,	85.1 83.1 86.5 92.8 99.6	41-55-24 .0 IN, PEA	34.3 30.4 38.5 57.0 59.5	41-53-11 0 IN, PE	44 44 45 45 45 45 45 45	TWO DR CC
			LATITUDE 41-54-1 FF = 5.00 IN, P			* * * * * * *		* * * * * *	**************************************
/ER	N E R	AREA (AC)	LATITU OFF =	70 56 79 124 246	LATITUDE FF = 5.	48 39 59 87 114 126	LATITUDE FF = 5.	280 300 330 371 463	46 BE ATED, ARIU
EEL RIVER	DESIGN HIGH WATER	ELEV MSL)		82.1 80.1 83.5 96.1	LA RUNOFF	31.0 22.4 35.5 55.5 55.6	LA RUNOFF	39.8 40.4 41.3 42.5 45.3	CAVA
-EEL	H16	ELEV	k 8		k k	w 0/ w 4. rv rv	*	W4444	INCL EEEX ARE
SUBWATERSHED-EEL		F F	MANOMET, MASS SPNY DESIGN STORM	1660 2130 1730 1140 860	STORM	1800 3210 1670 1290 1160	STORM	1470 1330 1170 1050 710	A. BE
ATER	>	COST PER AC FT (\$)	IASS GN ST	16 21 21 17 11 8		32 32 16 11 11	IASS GN S	113	
SUBWATER	SPILLWAY	, Z	QUAD-MANOMET, MASS	4 8 8 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9	QUAD-MANOMET, MASS PRIN SPWY DESIGN	4.1 2.4 7.1 115.7 24.5 28.7	QUAD-MANCMET, MASS PRIN SPWY DESIGN	4.1 4.6 5.8 7.1 2.0	± 111 ~ ~
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RE		DEPTH AT DAM (FT)	42 AC Y (B)	3.5 9.3 16.6 24.9 31.5	_	2.5 16.7 26.7 34.0	× ~	6.3 9.5 12.3 15.6 18.7	
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OUTH		COST/ SURF AC (\$)	MI.	36470 23490 14730 11850	MI I	2329C 1888C 21440 2287C 2277C	M I M	2391C 1119C 770C 621C	73 S 0RAG PE C D ON
EA-S		AREA (AC)	SOWATE	10 25 46 89 155	DA= 1.43 SQ MI STREAM WATER (8 25 48 72 95	SQ	14 60 138 222 314	N 19 Y ST Y TY BASE
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		STO AC FT	1004 TING	100 362 885 1671	1 I NG	100 401 1003 1606 1907	1007 TING	100 100 388 965 1830	(2) E (2) E (2) E (3) E (5) E (5) E
			TE-SS-3004 SITE RATING	NW OON	TE-SS-3006 SITE RATING	δ rν - 800	TE-SS-3007 SITE RATING	\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\	1
STUDY AREA-SOUTH SHORE	BENEFICIAL POOL	ELEV (MSL)	SITE-SS-3004 DA= 2.41 SQ MI = 1542 AC SITE RATING (2) STREAM WATER QUALITY (B)	63.5 0 0.0 10 25 3647C 76.6 362 2.8 2970 46 2349C 1 84.9 885 6.8 1480 89 1473C 2 91.5 1671 13.0 11C0 155 11850 3	SITE-SS-3006 SITE RATIN	16.6 0 0.0 8 23.29C 22.5 100 1.2 5900 25 23.29C 30.7 401 5.3 2250 48 1888C 1 40.8 1003 13.2 1550 72 21440 2 48.0 1606 21.1 1350 95 2287C 3 51.0 1907 25.0 1260 106 2277C 3	SITE-SS-3007 SITE RATIN	28.2 31.5 34.4 37.5 40.8	NOTES - (1) COSTS ARE BASED ON 1973 S.C.S. (2) EMERGENCY SPILLWAY STORAGE AND (3) EMERGENCY SPILLWAY TYPE CODE-C (4) TABULAR DATA ARE BASED ON PRELI (5) ELEVATIONS ARE SHOWN TO THE NEA
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CONSIDERED ACCURATE TO THAT DEGREE.

** DO NOT USE FOR FINAL SITE SELECTION OR LAND ACQUISITION. **

SUMMARY DATA FOR POTENTIAL UPSTREAM RESERVOIR SITES

SUBMATERSHED-EEL RIVER	* *	######################################	中央市市市市市市市市市市市市市市市市市市市市市市市市市市市市市市市市市市市市	DA= 0.53 SQ MI = 339 AC USGS QUAD-PLYMOUTH (2) STREAM WATER QUALITY (B) 100-YR PRIN SPWY DESIGN STORM RUNDFF = 5.10 IN, PEAK FLOW = 155 CFS	* * * * * * * * * * * * * * * * * * *	3.5 5120 25 20470 8.1 * 68.6 E 176 6.1 2910 * 70.9 40 * 73.9 16 25 * 0.14 4.8 4060 29 18930 9.3 * 69.9 E 220 7.8 2490 * 72.0 44 * 75.0 17 28 * 0.17	3400 32 1778C 10.6 * 71.1 E 265 9.3 2170 * 73.3 48 * 76.6 19 33 *	*	本本本本本本本本本本本本本本本本本本本本本本本本本本本本本本本本本本本本
STUDY	EFICIAL PO	**************************************	*******	CA	0.				中市市市市市市市市市市市市市市市市市市市市市市市市市市市市市市市市市市市市
	BEN	ELEV STORAGE	中央市市市市市市市市市市市市市市市市市市市市市市市市市市市市市市市市市市市市	SITE-SS-3008 SITE RATING (2)	59.3 0 0	66.1 100 3	170		市市市市市市市市市市市市市市市市市市市市市市市市市市市市市市市市市市市市市

COSTS ARE BASED ON 1973 S.C.S. DESIGN CRITERIA AND COST DATA.

EMERGENCY SPILLMAY STORAGE AND COSTS ARE BASED ON TOTAL STORAGE, INCLUDING BENEFICIAL POOL.

EMERGENCY SPILLMAY TYPE COCE— C=CONCRETE CHUTE, D=CONCRETE DROP, E=EXCAVATED, T= TWO SPILLWAYS, N= NONE
TABULAR DATA ARE BASED ON PRELIMINARY INFORMATION. FIGURES SHOWN ARE PRIMARILY FOR COMPARISON PURPOSES.

ELEVATIONS ARE SHOWN TO THE NEAREST 0.1 FOOT TO SHOW VARIATION BETWEEN DEVELOPMENTS ONLY, AND ARE NOT TO BE
CONSIDERED ACCURATE TO THAT DEGREE.

** DO NOT USE FOR FINAL SITE SELECTION OR LAND ACQUISITION. ** 26.53

Existing Site SS-3001 (Russell Mill Pond)

Location:

On the Eel River about 1,200 feet upstream from Route 3 in Plymouth, Mass.

Plymouth, Mass. USGS quadrangle

Latitude: 41°55'01" Longitude: 70°37'38"

Surface Area Height of Drainage Area (Acres) Dam (Ft.) (Acres) (Sq. Miles) 24 1,550 2.42

Potential for

Please refer to Site Data and Design Summary Table for Potential

Site SS-3001.

Expansion:

Remarks:

The dam is an earthfill structure. The spillway is a concrete weir and flume with flashboard control. There are several old concrete structures along the dam including a non-functional

fish ladder.

Ownership and

The pond is owned by Russell Mill Pond Inc. and is used

for recreation

Use:

Existing Site SS-3002 (Eel River Pond)

Location:

On the Eel River about 1,800 feet downstream from Route 3 in Plymouth, Mass.

Manomet, Mass. USGS quadrangle

Latitude: 41°55'26" Longitude: 70°37'19"

Surface Area Height of Drainage Area (Acres) Dam (Ft.) (Acres) (Sq. Miles) 1,800 2.81

Potential

Expansion:

Please refer to Site Data and Design Summary Table for

Potential Site SS-3002.

Remakrs:

The dam is an earthfill structure. A portion of the downstream face is a vertical stone wall. The spillway is a 6-foot corrugated metal riser and conduit. The spillway has provision for flashboards. There is also a concrete sluice and fish ladder near the right abutment. Trees and brush are growing on the dam.

Ownership and

The pond is owned by the Chilton Trust and is used for recreation and irrigation.

Use:

Existing Site SS-3009 (Arms House Pond)

Location: On Town Brook at Spring Lane in Plymouth, Mass.

Plymouth, Mass. USGS quadrangle

Latitude: 41°57'12" Longitude: 70°39'56"

Surface Area Height of Drainage Area (Acres) Dam (Ft.) (Acres) (Sq. Miles) 2,100 3.28

Potential For

Limited. The pond is located in Plymouth Center.

Expansion:

Remarks:

Spring Lane forms the dam. The downstream face is a vertical masonry wall. The spillway is a concrete structure with flashboards. There is also a fish ladder which outlets to the left of a rustic grist mill.

Ownership and Use:

The pond is owned by the town of Plymouth and is not used for a specific purpose.

Existing Site SS-3011

Location:

On an unnamed tributary to the Eel River at Clifford Road in Plymouth, Mass.

Manomet, Mass. USGS quadrangle

Latitude 41°55'33"

Longitude: 70°36'50"

Surface Area Height of Drainage Area
(Acres) Dam (Ft.) (Acres) (Sq. Miles)

8 10 4,050 6.33

Potential for Expansion: Raising the pond level by 10 feet would create a 50-acre pool. Further expansion would affect Sandwich Road and new houses.

Remarks:

Clifford Road forms the dam. The downstream face adjacent to the spillway is a vertical stone wall. The spillway is a concrete weir with flashboards which outlet through a concrete bridge opening.

Ownership and Use:

The pond is owned by Sherman Whipple Trust and is not used for a specific purpose.

Existing Site SS-3012 (Forge Pond)

Location: On an unnamed tributary to the Eel River about 1,400 feet upstream from Sandwich Road in Plymouth, Mass.

Manomet, Mass. USGS quadrangle

Latitude: 41°55'10" Longitude: 70°36'40"

Surface Area Height of Drainage Area (Acres) Dam (Ft.) (Acres) (Sq. Miles) 3,000 4.69

Potential Raising the pond level by 25 feet would create a 120-acre pool. for Cranberry bogs would be affected. Sandy soils may limit ex-Expansion: pansion potential.

Remarks: The dam is an earthfill structure. The spillway a two bay concrete structure with flashboards. Trees and brush are growing on the dam.

Ownership The pond is owned by Dorothy Illig and is used for irrigation and aesthetics.
Use:

Existing Site SS-3013 (The Arm Pond)

Location: On an unnamed tributary to Beaver Dam Brook between Beaver Dam Road and Bartlett Road in Plymouth, Mass.

Manomet, Mass. USGS quadrangle

Latitude: 41°54'07" Longitude: 70°33'57"

Surface Area Height of Drainage Area (Acres) Dam (Ft.) (Acres) (Sq. Miles) 35 0.05

Potential The small drainage area limits expansion potential. for Expansion:

Remarks: The dam is an earthfill structure constructed of clean sand.

The spillway is a 5-foot diameter, steel half-round riser and conduit. Some seepage was noted along the downstream toe.

Ownership The pond is owned by United Cape Cod Cranberry Corporation and is used for cranberry bog irrigation.
Use:

Existing Site SS-3014 (Beaver Dam Pond)

Location:

On Beaver Dam Brook about 7,500 feet upstream from Route 3A in Plymouth, Mass.

Manomet, Mass. USGS quadrangle

Latitude: 41^o54'03"

Longitude: 70°34'16"

Surface Area

Height of

Drainage Area

(Acres) 30

Dam (Ft.) 14

(Acres) (Sq. Miles)

Potential

for Expansion: The small drainage area limits expansion potential.

Remarks:

The dam is an earthfill structure with some riprap on the upstream face. The principal spillway is a concrete drop structure with provision for flashboards. There is also a similar smaller structure which outlets to a by-pass channel. Trees are growing on the dam.

Ownership and

The pond is owned by United Cape Cod Cranberry Corporation and is used for cranberry bog irrigation.

Use:

Existing Site SS-3015 (Indian Brook Pond)

Location:

On Indian Brook at Route 3-A in Plymouth, Mass.

Manomet, Mass. USGS quadrangle

Latitude: 41⁰53'15"

Longitude: 70°32'31"

Surface Area (Acres)

Height of Dam (Ft.) Drainage Area

(Acres)

(Sq. Miles)

Potential for

Limited. Many dikes would be required. Several businesses would be affected.

Expansion:

Remarks:

The dam is formed by a short section of road fill. The spillway is composed of four 24-inch metal culverts with a timber riser and 3 bays of flashboards. Dam and spillway

are in good condition.

Ownership and

The pond is owned by George Briggs and is not used for a specific purpose.

Use:

Existing Site SS-3016 (Briggs Reservoir)

On Indian Brook between Sandwich Road and Ship Pond Road in Location: Plymouth, Mass.

Manomet and Sagamore, Mass. USGS quadrangles

Latitude: 41^o52'31"

Longitude: 70°33'37"

Surface Area (Acres)

Height of Dam (Ft.) Drainage Area (Acres)

(Sq. Miles) 0.66

Sandy soils may limit expansion potential. Potential

for

Expansion:

Remarks:

The dam is an earthfill structure. The principal spillway is a 3-bay concrete drop structure with provision for flashboards. The spillway is in good condition except for the concrete cap which is deteriorating. A secondary spillway delivers water to a canal.

and

Ownership The reservoir is owned by George Briggs and is used for irrigation.

Use:



SS-3001 Russell Mill Pond



SS-3002



SS-3009 Arms House Pond



SS-3011





SS-3012 Forge Pond

SS-3015 Indian Brook Pond



SS-3013 The Arm Pond

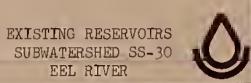




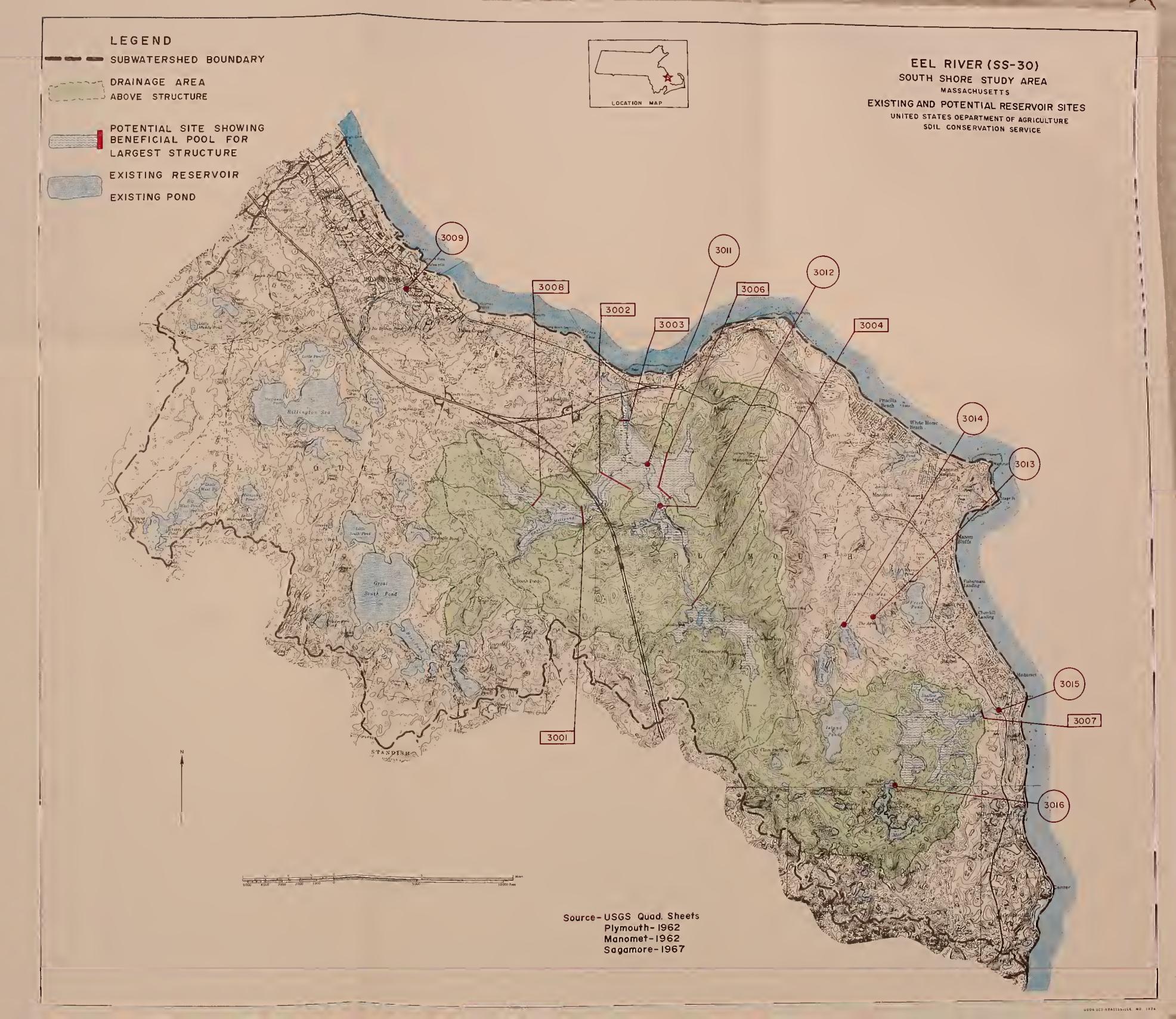
SS-3014 Beaver Dam Pond

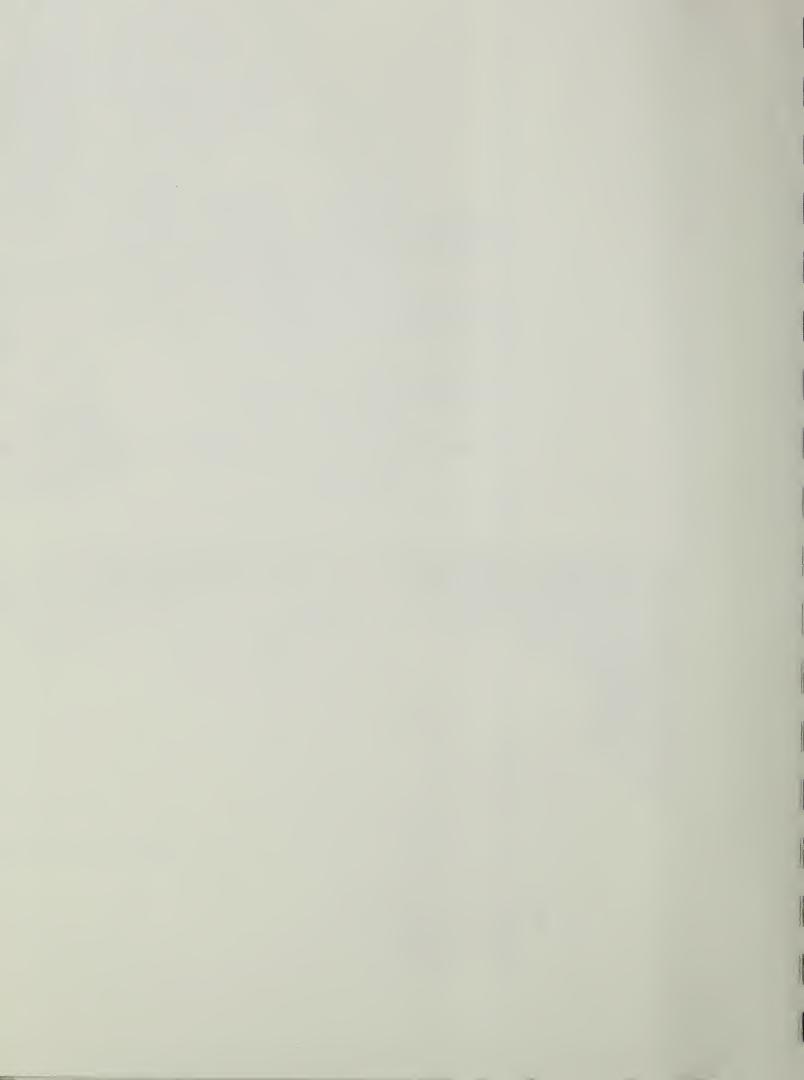


SS-3016 Briggs Reservoir









South Shore Study Area

Site Data For Subwatershed SS-31 Cape Cod Canal

This subwatershed covers about 10,700 acres in Plymouth in Plymouth County and Bourne and Sandwich in Barnstable County.

The major stream in the subwatershed is Herring River which originates in Plymouth and flows south through two Great Ponds to the Cape Cod Canal in Bourne.

Three existing reservoirs were studied.

Existing Site SS-3101 (Little Herring Pond)

Location: On the Herring River west of Long Pond Road in Plymouth, Mass.

Sagamore, Mass. USGS quadrangle

Latitude: 41°49'08" Longitude: 70°34'23"

Surface Area Height of Drainage Area
(Acres) Dam (Ft.) (Acres) (Sq. Miles)

83 4 250 0.39

Potential Sandy soils may limit expansion potential. The small for drainage area limits expansion. Expansion:

Remarks: The dam is an earthfill structure. The spillway is an open concrete flume with no means of controlling the water level. Top width of the dam is about 3 feet. Concrete in the spillway has some deterioration.

Ownership The pond is an enhanced Great Pond and is used for irrigation and and recreation.
Use:

Existing Site SS-3102 (Great Herring Pond)

Location:

On the Herring River at the Bourne-Plymouth town line in Plymouth, Mass.

Sagamore, Mass. USGS quadrangle

Latitude: 41^o47'11"

Longitude: 70°33'55"

Surface Area (Acres)

410

Height of Dam (Ft.)
4

Drainage Area (Acres) 1,385

Potential for

Expansion:

Sandy soils may limit expansion potential. The pond is already quite large in relation to the drainage area.

Remarks:

The dam is an earthfill structure. The spillway is a concrete weir with flashboard control. Trees are growing on the dam. Concrete in the spillway has spalled.

Ownership and Use:

The pond is an enhanced Great Pond and is used for irrigation and recreation.

Existing Site SS-3103 (Foundry Pond)

Location:

On the Herring River about 200-feet upstream from Route 6E in Bourne, Mass.

Sagamore, Mass. USGS quadrangle

Latitude: 41°46'24" Longitude: 70°33'47"

Surface Area (Acres)

Height of Dam (Ft.) (Acres) (Sq. Miles) 2.54

Drainage Area

Potential for

Expansion:

Limited by streets, houses, and Great Herring Pond located upstream.

Remarks:

The dam is an earthfill structure. The downstream face is a vertical stone wall. The spillway consists of two concrete bays with flashboard control. Each bay is about 3 feet wide. The left bay of the spillway is a concrete fish ladder. The downstream channel has concrete and masonry sidewalls to the Cape Cod Canal.

Ownership and

Ownership of the pond was not determined. The pond is used for recreation.

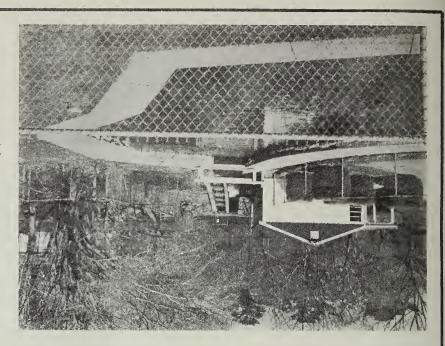
Use:





CAPE COD CAUAL
CAPE COD CAUAL

Foundary Pond SS-3103



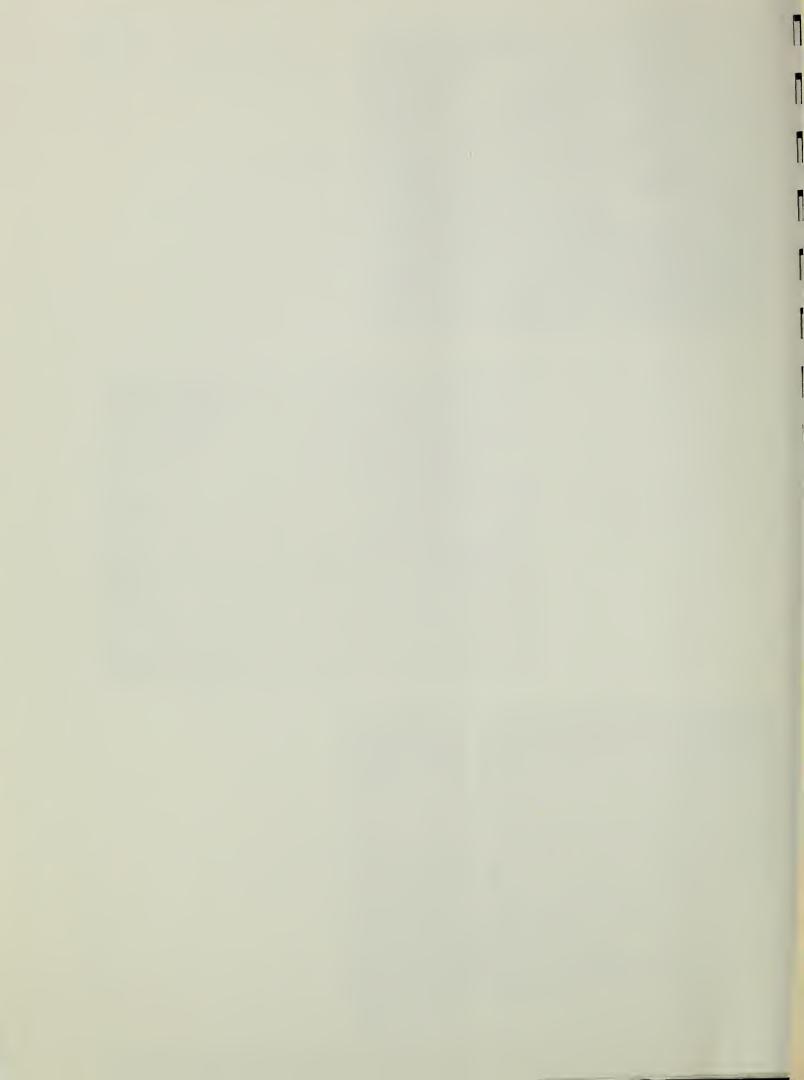


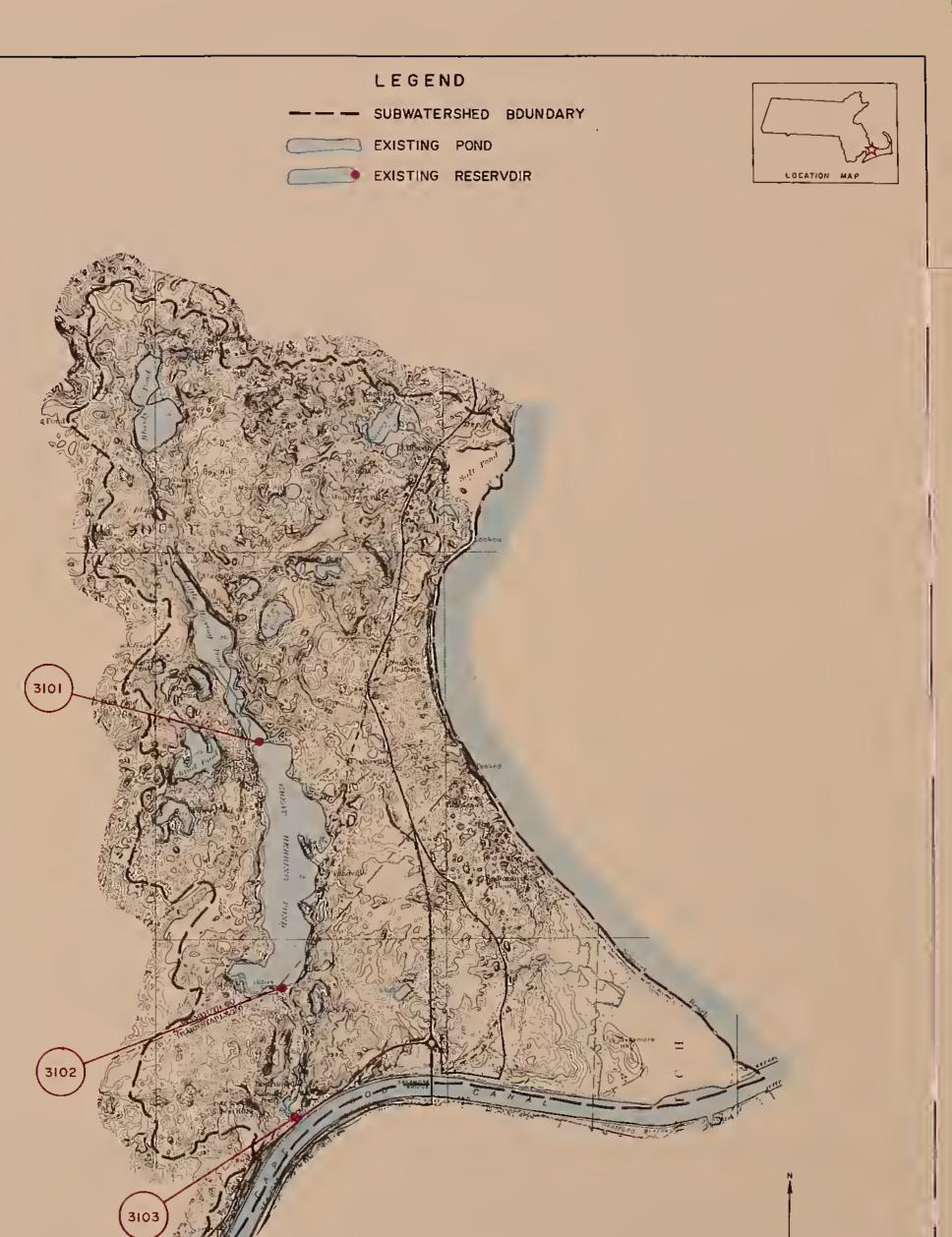
SS-3102 Great Herring Pond



Piffle Herring Pond







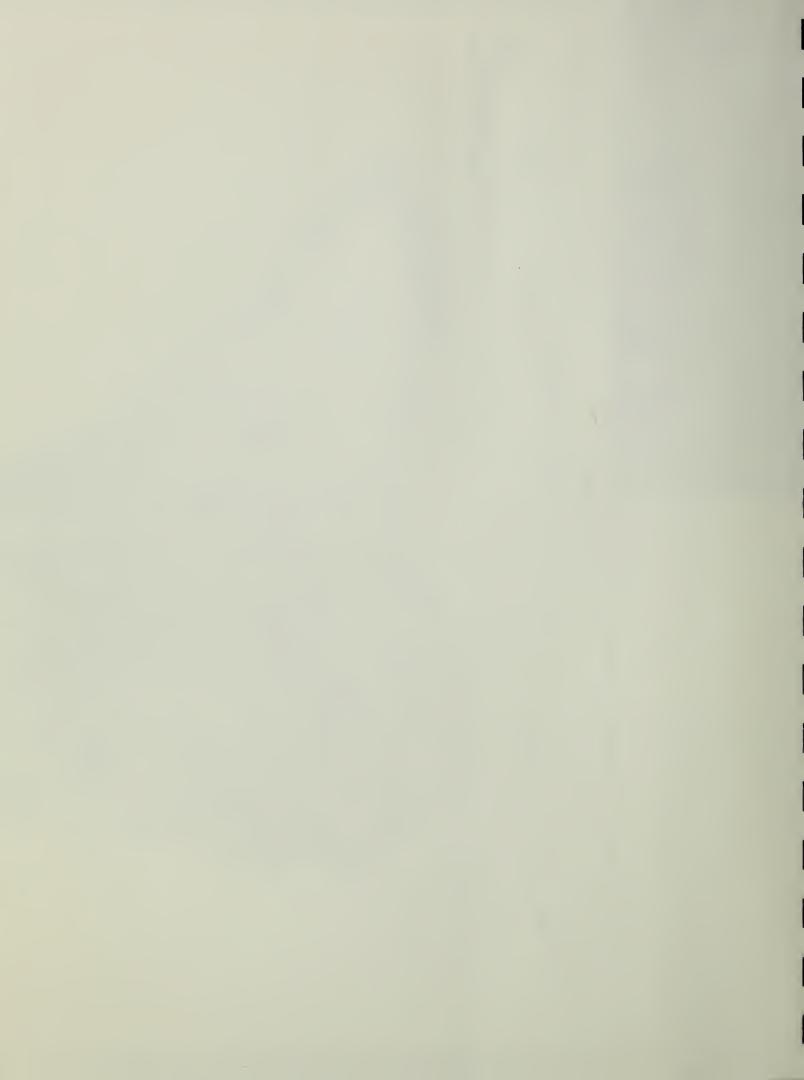
CAPE COD CANAL (SS-31)
SDUTH SHORE STUDY AREA
MASSACHUSETTS

EXISTING AND POTENTIAL RESERVOIR SITES

UNITED STATES DEPARTMENT OF AGRICULTURE

SOIL CONSERVATION SERVICE

PERSONAL PROPERTY



CAPE COD STUDY AREA SITE DATA FOR

Subwatershed CC-32, North Shore-Cape

The subwatershed covers about 35,700 acres in Barnstable, Bourne, Dennis, Sandwich, and Yarmouth in Barnstable County.

The area contains a number of small streams which originate in the central portion of Cape Cod and flow north to Cape Cod Bay.

Geology of the subwatershed is characterized by shale bedrock overlain by about 150 feet of outwash sand and gravel.

Four existing reservoirs were studied. No potential reservoir sites which met study criteria were located.

Site CC-3201(Upper Shawme Lake)

Location:

On Mill Creek about 2600 feet upstream from Route 130 in Sandwich, Mass.

Pocasset, Mass. USGS quadrangle

Surface Area	Height of	Drainage Area	
(Acres)	Dam(Ft.)	(Acres)	Sq. Mi.
20	10	250	0.4

Potential for

for Expansion:

The surface area could be nearly tripled without affecting any major facilities. The sandy soils may limit expansion potential. The small drainage area also limits expansion.

Remarks:

The dam is an earthfill structure with a vertical stone downstream face. The spillway is a timber flume with flashboard control. The outlet channel is riprapped to Shawme Lake. Large trees are growing at the downstream toe and small pine trees are growing along the top of the dam.

Ownership and Use:

The lake is owned by Peter Cook and is used for recreation.

Site CC-3202(Shawme Lake)

Location:

On Mill Creek about 50 feet upstream from Route 130 in Sandwich, Mass.

Sagamore, Mass. USGS quadrangle

rface Area Height of Drainage Area (Acres) Dam(Ft.) (Acres) Sq. Mi. Surface Area

Potential

for Expansion: Limited. Raising the water level by 4 feet would affect a cemetery and several houses. Surface area would not be appreciably increased. Sandy soils may limit expansion potential. Site CC-3201 is located immediately upstream.

Remarks:

The dam is an earthfill structure with a vertical masonry upstream face. The principal spillway is a concrete weir with flashboard control. The weir is about 8 feet wide. There is also an auxiliay spillway to an old mill waterwheel. A concrete fish ladder is located to the right of the principal spillway. Large trees are growing on the dam. There is about 2 feet of freeboard between the lake level and top of dam. All concrete and masonry in the structure is in good condition.

Ownership and Use:

The lake is owned by the Town of Sandwich and is used for recreation.

Site CC-3203

Location:

On a tributary to Cape Cod Bay between the Penn-Central railroad and Route 6A in Sandwich, Mass.

Sandwich, Mass. USGS quadrangle

urface Area Height of Drainage Area
(Acres) Dam(Ft.) (Acres) Sq. Mi. Surface Area

Potential for

Limited. The small drainage area limits expansion. Route 6A would be affected by expansion. Sandy soils may limit expansion potential.

Expansion:

Remarks:

The railroad embankment forms the dam. The spillway is a sheet-steel riser with flashboard control and pipe conduit. Weir length on the riser is about 4 feet.

Ownership and

The pond is owned by the Commonwealth of Massachusetts and is used for irrigation water.

Use:

Site CC-3204(Nye Pond)

Location:

On an unnamed tributary to Scorton Creek at Old County Road in Sandwich, Mass.

Sandwich, Mass. USGS quadrangle

Surface Area
(Acres)

Height of Dam(Ft.)

Drainage Area (Acres)

Sq. Mi.

Potential for Expansion:

The surface area could be nearly tripled without affecting major facilities. The small drainage limits expansion potential. Sandy soils may limit expansion potential.

Remarks:

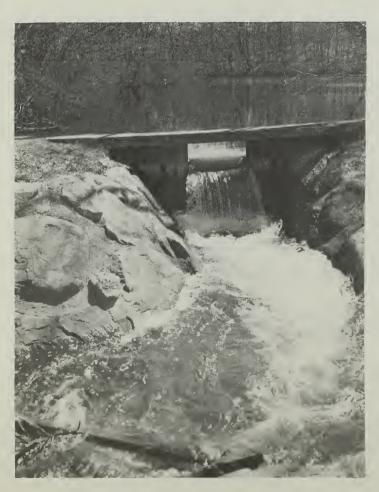
Old County Road forms the dam. The upstream face of the dam is a vertical granite-block wall. The spillway is a concrete pipe with flashboard control. Immediately upstream from the spillway inlet is a series of timber trash racks. There is also an auxiliary pipe spillway to deliver water to downstream trout pools. The dam and spillways are in good condition.

Ownership and Use:

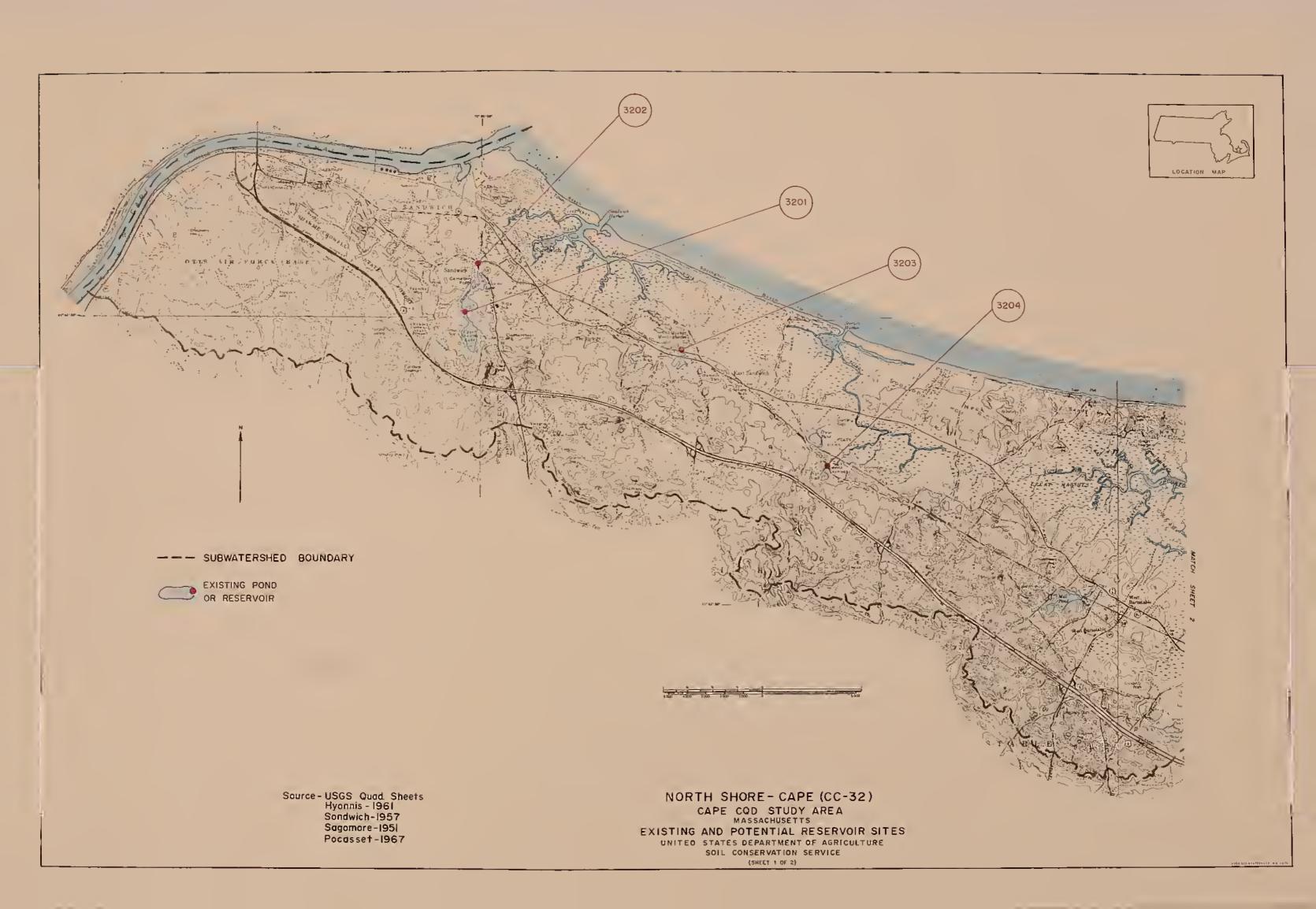
The pond is owned by the Mass. Division of Fisheries and Game and is used in connection with a fish rearing station.



Existing Site CC-3201 (Upper Shawme Lake)



Existing Site CC-3204 (Nye Pond)







LEGEND

- - SUBWATERSHED BOUNDARY

THERE WERE NO EXISTING OR POTENTIAL RESERVOIR SITES WHICH MET STUDY CRITERIA LOCATED IN THIS PORTION OF THE SUBWATERSHED.



Source - USGS Quad. Sheets Dennis - 1961 Hyannis - 1961 Sondwich - 1957 (SHEET 2 OF 2)

NORTH SHORE - CAPE (CC-32)

CAPE COD STUDY AREA

MASSACHUSETTS

EXISTING AND POTENTIAL RESERVOIR SITES
UNITED STATES DEPARTMENT OF AGRICULTURE
SOIL CONSERVATION SERVICE



Subwatershed CC-33, North Shore-Cape

The subwatershed covers about 14,900 acres in Brewster, Dennis, Harwich, and Orleans in Barnstable County.

The area contains a number of small streams which originate in the central portion of Cape Cod and flow north to Cape Cod Bay.

Geology of the subwatershed is characterized by shale bedrock overlain by about 150 feet of outwash gand and gravel.

One existing reservoir was studied. No potential reservoir sites which met study criteria were located.

Site CC-3301(Lower Mill Pond)

Location:

On Stony Brook about 100 feet upstream from Stony Brook Road in Brewster, Mass.

Harwich, Mass. USGS quadrangle

Surface Area Height of Drainage Area (Acres) Dam(Ft.) (Acres) Sq. Mi.

Potential for

Expansion:

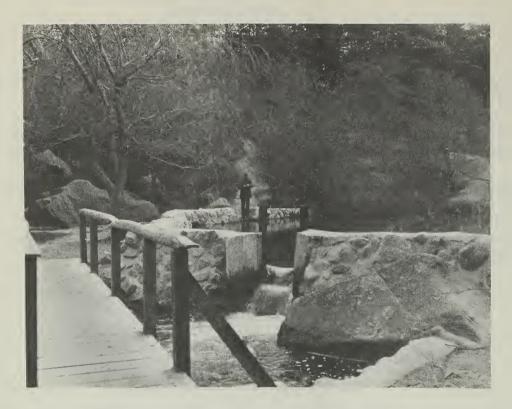
Limited. The much larger Upper Millpond is located immediately upstream. Sandy soils limit expansion potential.

Remarks:

The dam is an earthfill structure. A masonry wall has been built to a height two feet above the earth fill. There are two spillways through the masonry wall. Each spillway is a weir with flashboards; one weir is four feet wide, the other is two feet wide. The dam and spillways are in good condition. The channel downstream of the dam is a series of steps which serve as a fish ladder for herring.

Ownership and Use:

The pond is owned by the town of Brewster and is used for recreation.



Existing Site CC-3301 (Lower Mill Pond)

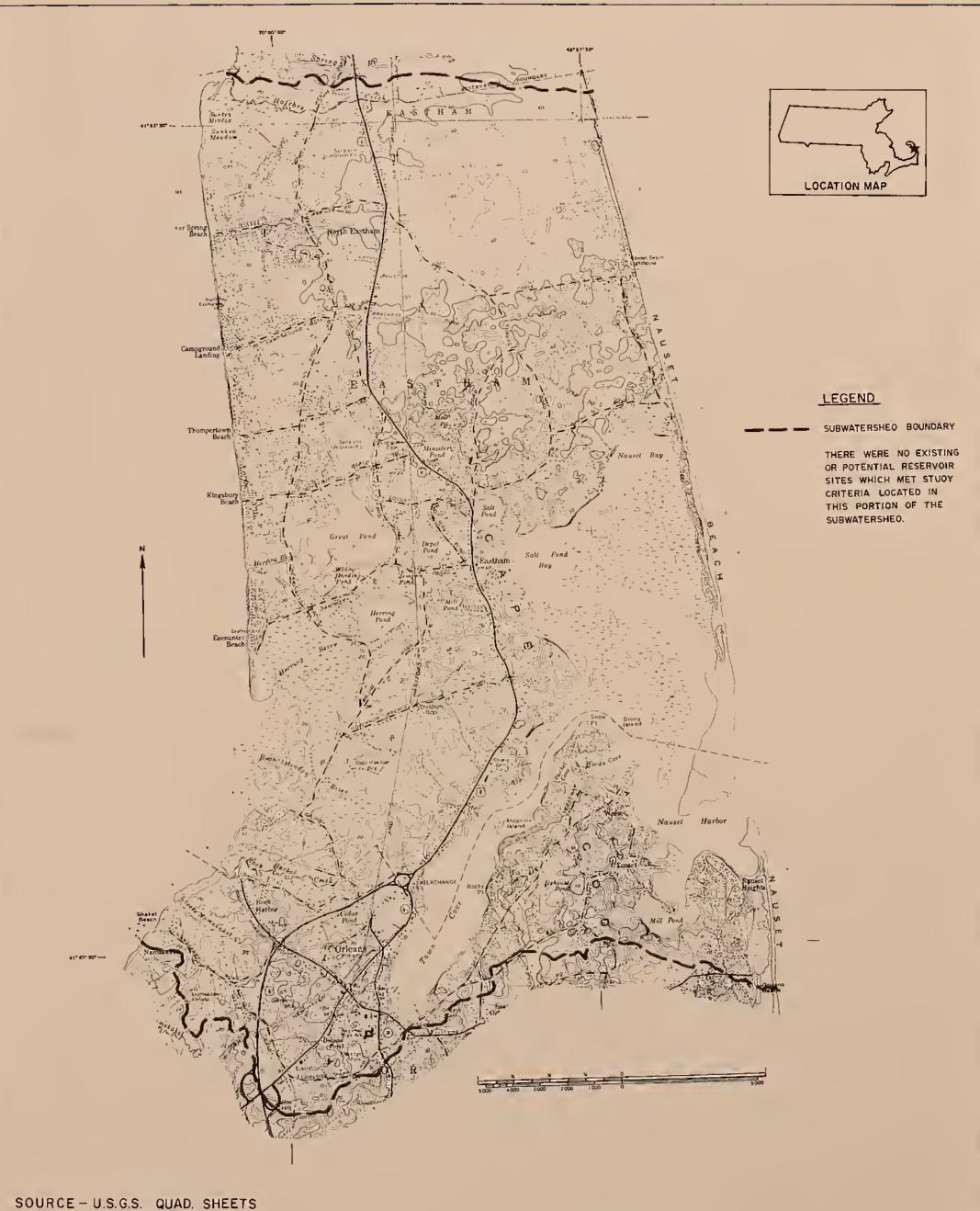
Subwatershed CC-34, Cape Cod

The subwatershed covers about 13,300 acres in Eastham, Orleans, and Wellfleet in Barnstable County.

No potential or existing reservoir sites which met inventory criteria were located.







WELLFLEET - 1958 ORLEANS - 1962

CAPE COD (CC-34)

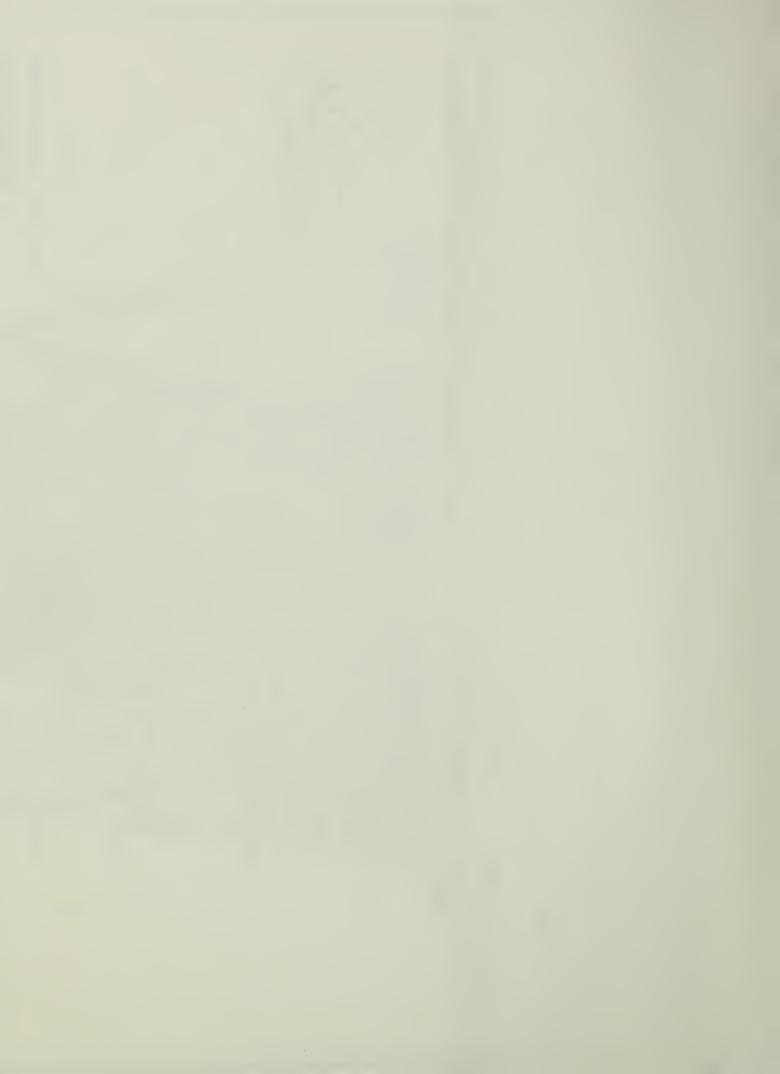
CAPE COD STUDY AREA

MASSACHUSETTS

EXISTING AND POTENTIAL RESERVOIR SITES

UNITED STATES DEPARTMENT OF AGRICULTURE

SOIL CONSERVATION SERVICE



Subwatershed CC-35, Cape Cod

This subwatershed covers about 15,800 acres in the towns of Eastham, Truro and Wellfleet, in Barnstable County.

The major stream in this subwatershed is the Herring River which originates in Wellfleet and flows generally southwesterly into Wellfleet Harbor, a tidal inlet of Cape Cod Bay. Elevations range from a high of about 170 feet in Truro to tidewater in Wellfleet Harbor.

Geology in this subwatershed is characterized by shale bedrock overlain by about 150 feet of outwash sand and gravel. One potential reservoir site was studied. There were no existing reservoirs which met study criteria.

POTENTIAL SITE CC-3501

Location:

On Fresh Brook about 300 feet upstream from GAR Highway, Route 6, in Wellfleet, Mass.

Wellfleet, Mass. USGS quadrangle

Latitude: 41°53'23" Longitude: 69°59'14"

Facilities Affected:

Facility House

Elevation

Geologic Conditions:

Both abutments are outwash sand with thin gravel lenses. Depth to shale bedrock in the foundation is estimated to be about 400 feet. Waterholding capabilities appear to be poor. Leakage is expected through both abutments and the foundation. Pervious borrow material for dam construction was located near the site; impervious material was not located.

Engineering Notes:

The downstream slope of the dam would be subject to tidal effects. Preliminary design information indicates that a concrete monolithic conduit emergency-spillway may be needed at this site.

SUMMARY DATA FOR POTENTIAL UPSTREAM RESERVOIR SITES

	SAFE	AT 95	*PERCENT	*CHANCE		(MGD)	******	9-59-14	197 CFS		****	0.14	0.24	0.32	0.37		******		
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EMERGENCY SPILLWAY STORAGE AND COSTS ARE BASED ON TOTAL STORAGE, INCLUDING BENEFICIAL POOL.

EMERGENCY SPILLWAY STORAGE AND COSTS ARE BASED ON TOTAL STORAGE, INCLUDING BENEFICIAL POOL.

EMERGENCY SPILLWAY TYPE CODE— C=CONCRETE CHUTE, D=CONCRETE DROP, E=EXCAVATED, T= TWO SPILLWAYS, N= NONE
TABULAR DATA ARE BASED ON PRELIMINARY INFORMATION. FIGURES SHOWN ARE PRIMARILY FOR COMPARISON PURPOSES.

ELEVATIONS ARE SHOWN TO THE NEAREST 0.1 FOOT TO SHOW VARIATION BETWEEN DEVELOPMENTS ONLY, AND ARE NOT TO BE 2525

CONSIDERED ACCURATE TO THAT DEGREE.

** DO NOT USE FOR FINAL SITE SELECTION OR LAND ACQUISITION. **

LEGEND

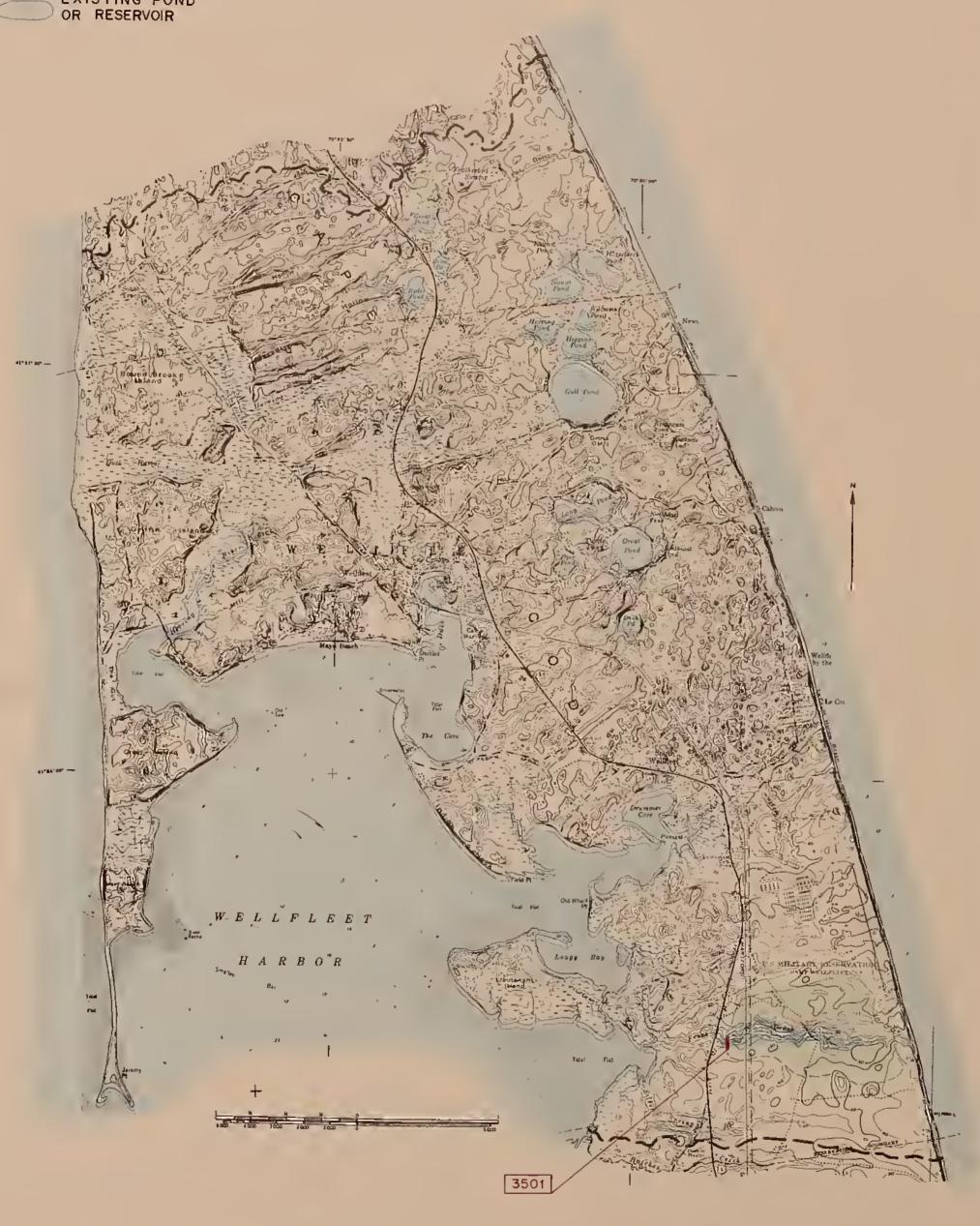
- SUBWATERSHED BOUNDARY

DRAINAGE AREA ABOVE STRUCTURE

POTENTIAL SITE SHOWING BENEFICIAL POOL FOR LARGEST STRUCTURE

EXISTING POND





Source - USGS Quad. Sheets Wellfleet - 1958

CAPE COD (CC-35) CAPE COD STUDY AREA MASSACHUSETTS EXISTING AND POTENTIAL RESERVOIR SITES UNITED STATES DEPARTMENT OF AGRICULTURE SOIL CONSERVATION SERVICE



Subwatershed CC-36, Cape Cod

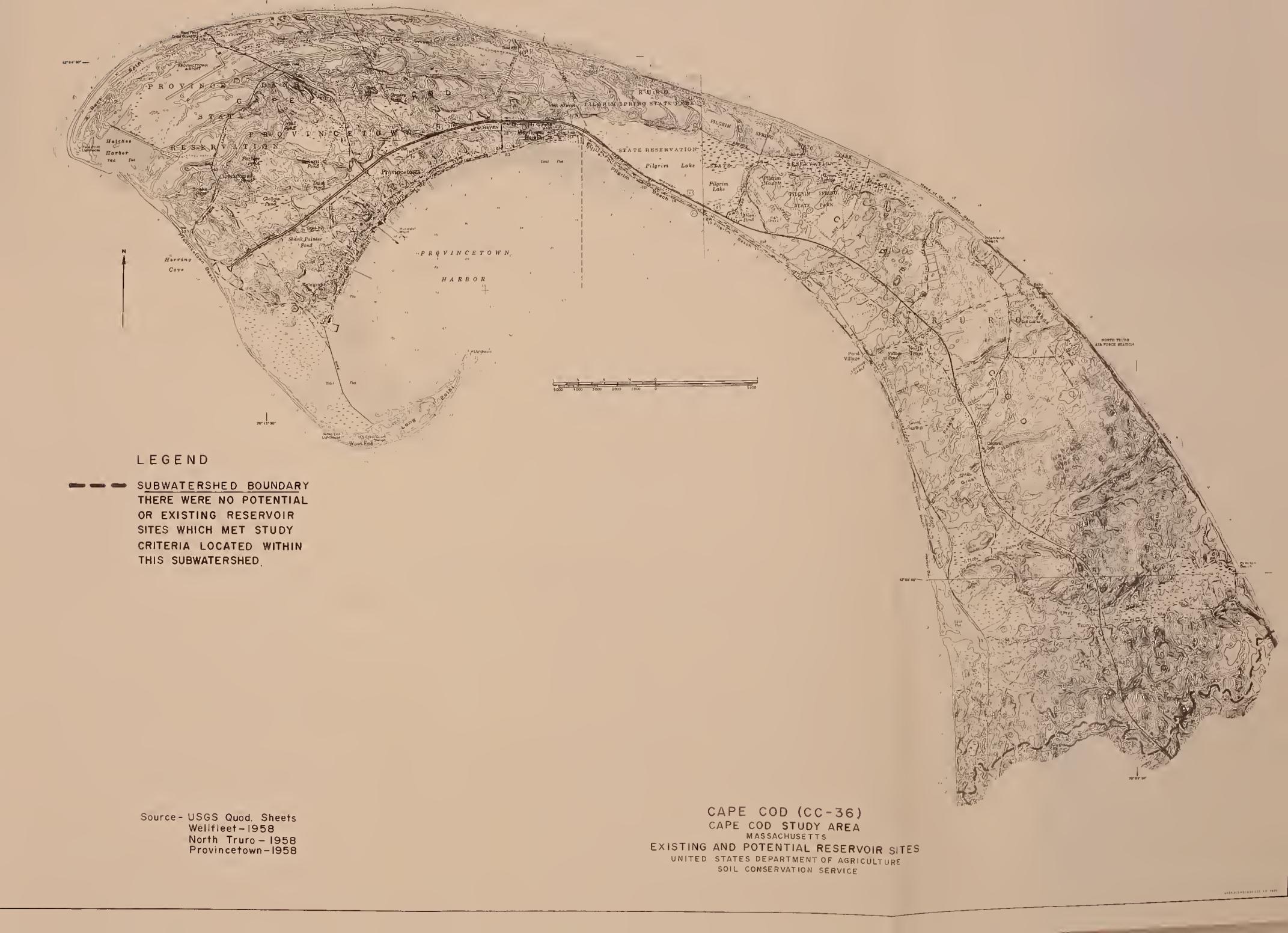
The subwatershed covers about 17,400 acres in Provincetown and Truro in Barnstable County.

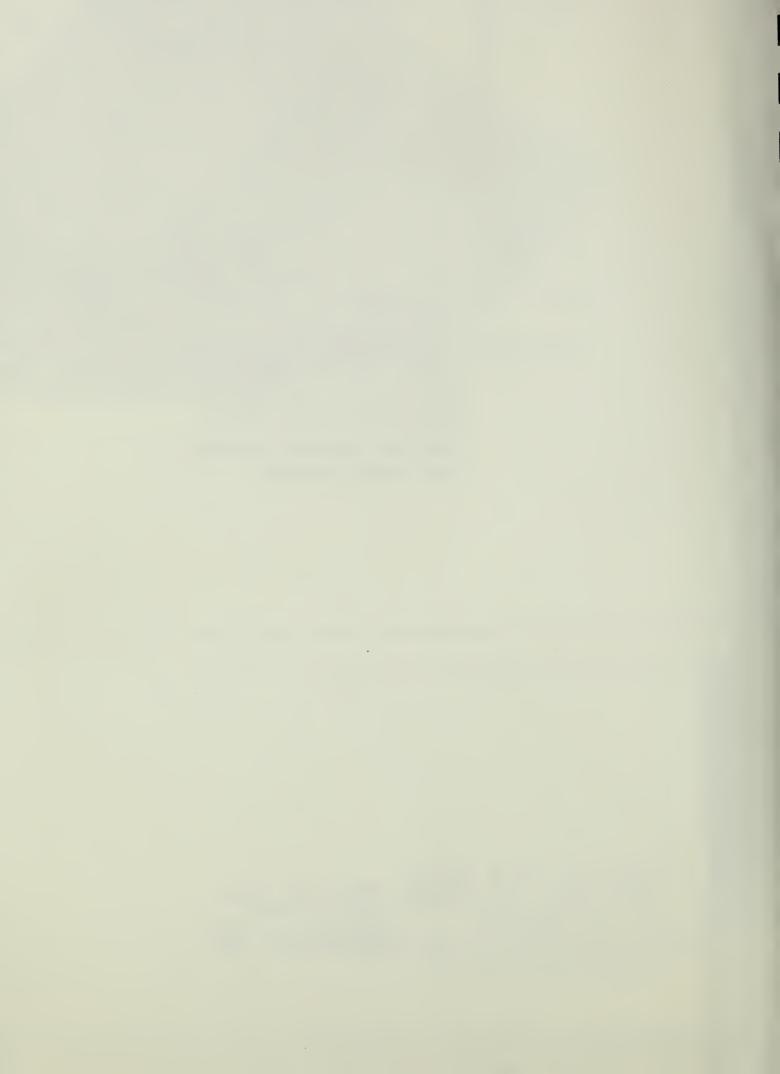
No potential or existing reservoir sites which met inventory criteria were located.

Subwatershed CC-37, South Shore-Cape

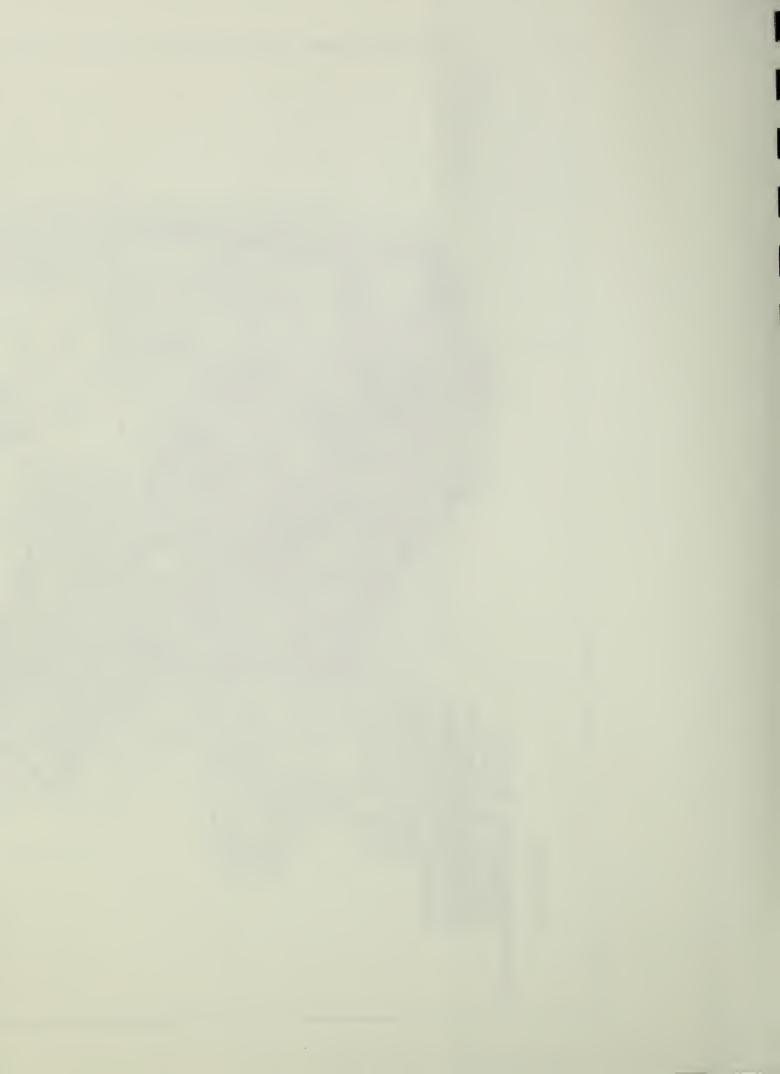
The subwatershed covers about 26,400 acres in Brewster, Chatham, Harwich, and Orleans in Barnstable, Mass.

No potential or existing reservoir sites which met inventory criteria were located.









CAPE COD STUDY AREA SITE DATA FOR Subwatershed CC-38, South Shore-Cape

This subwatershed covers about 44,700 acres in the towns of Barnstable, Brewster, Chatham, Dennis, Harwich and Yarmouth, in Barnstable County. There is a U.S. Geological Survey gaging station on the Herring River in Harwich.

All of the streams in this subwatershed flow into Nantucket Sound. The major stream is the Bass River which originates in Yarmouth and flows southeasterly, then southwesterly along the Dennis-Yarmouth town boundary. Other streams include: Herring River which originates in Harwich and flows generally southwesterly; Swan Pond River which originates in Dennis and flows southwesterly; and Parkers River which originates in Yarmouth and flows southerly. Elevations range from a high of about 150 feet in Dennis to tidewater in Nantucket Sound.

Geology in the subwatershed is characterized by Shale bedrock overlain by about 150 feet of outwash sand and gravel.

One potential reservoir site and one existing reservoir were studied.

POTENTIAL SITE CC-3801

Location:

On Mill Creek about 2400 feet upstream from Route 28 in Yarmouth, Mass.

Hyannis, Mass. USGS quadrangle

Latitude: 41°39'50" Longitude: 70°15'44"

Facilities Affected:

Facility

Elevation 25

Geologic Conditions: Both abutments are outwash sand and gravel with some thin silt lenses. Depth to shale bedrock in the foundation is estimated to be about 150 feet. Waterholding capabilities appear to be poor. Leakage is expected through both abutments and the foundation. Pervious borrow material for dam construction was located near the site; impervious material was not located.

Engineering Notes:

Preliminary design information indicates that a concrete drop structure emergency spillway may be needed at this site.

BE

	* SAFE * YIELD	** AT 95	*PERCENT	*CHANCE		CY) * (MGD)	********	LATITUDE 41-39-50 LONGITUDE 70-15-44	505 CFS		*****	0.18	0.30	1 0.37			1 1 1 1 1 1 1 1
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EMERGENCY SPILLWAY TYPE CODE— C=CONCRETE CHUTE, D=CONCRETE DROP, E=EXCAVATED, T= TWO SPILLWAYS, N= NONE TABULAR DATA ARE BASED ON PRELIMINARY INFORMATION. FIGURES SHOWN ARE PRIMARILY FOR COMPARISON PURPOSES. ELEVATIONS ARE SHOWN TO THE NEAREST 0.1 FOOT TO SHOW VARIATION BETWEEN DEVELOPMENTS ONLY, AND ARE NOT TO CONSIDERED ACCURATE TO THAT DEGREE. 5 (4)

EMERGENCY SPILLWAY STORAGE AND COSTS ARE BASED ON TOTAL STORAGE, INCLUDING BENEFICIAL POOL.

COSTS ARE BASED ON 1973 S.C.S. DESIGN CRITERIA AND COST DATA.

NOTES

** DO NOT USE FOR FINAL SITE SELECTION OR LAND ACQUISITION. **

Site CC-3802(Mill Pond)

Location:

On Mill Creek about 100 feet upstream from Route 28 in Yarmouth, Mass.

Hyannis, Mass. USGS quadrangle

Surface Area (Acres)

Height of Dam(Ft.)

Drainage Area (Acres) 2.350

Sq. Mi. 3.7

Potential

Sandy soils limit expansion potential.

for

Expansion:

Remarks:

The dam is an earthfill structure. The spillway is a concrete weir with flashboard control which discharges through a box culvert to a concrete fish ladder. Concrete in the spillway is spalling in some areas. The dam is in good condition.

Ownership and Use:

Ownership was not determined. The pond is used for fishing.



Existing Site CC-3802 (Mill Pond)





Subwatershed CC-39, South Shore-Cape

This subwatershed covers about 22,200 acres in the towns of Barnstable, Mashpee and Sandwich, in Barnstable County.

All of the major streams in this subwatershed flow generally southerly into Nantucket Sound. The Mashpee River originates in Wakeby Pond in Sandwich and flows through Mashpee. The Santuit River originates in Santuit Pond in Mashpee and forms the Mashpee-Barnstable town boundary south of the village of Santuit. Marstons Mills River originates in Mystic Lake in Barnstable and flows into North Bay, a tidal inlet of Nantucket Sound. The Centerville River originates in Wequaquet Lake in Barnstable and flows into East Bay, also a tidal inlet of Nantucket Sound. Elevations range from a high of about 230 feet in Barnstable to tidewater in Nantucket Sound.

Geology in the subwatershed is characterized by shale bedrock overlain by about 150 feet of outwash sand and gravel.

Three potential reservoir sites and 3 existing reservoirs were studied.

POTENTIAL SITE CC-3901

Location:

On the Santuit River about 1300 feet upstream from Quinaquisset Avenue in Mashpee and Barnstable, Mass. The town line bisects the pool area.

Cotuit, Mass. USGS quadrangle

Latitude: 41°37'12" Longitude: 70°27'01"

Facilities	Facility	Elevation
Affected:	Barn	<u>LiO</u>
	House	35
	House	25
	Barn	25
	House	20
	Barn	20
	Old Mill Road	15

Geologic Conditions:

Both abutments are outwash sand and gravel. Depth to shale bedrock in the foundation is estimated to be about 150 feet. Waterholding capabilities appear to be poor. Leakage is expected through both abutments and possibly through the foundation. Pervious borrow material for dam construction was located near the site; impervious material was not located.

Engineering Notes:

The right abutment is recommended for the excavated emergency spillway location. Inflow to the site may be sufficient to maintain a pool in spite of losses through the abutments and foundation.

POTENTIAL SITE CC-3902

Location:

On Mashpee River about 500 feet upstream from Route 28 in Mashpee, Mass.

Cotuit, Mass. USGS quadrangle

Latitude: 41°37'23" Longitude: 70°28'55"

Facilities Affected:

Facility Wagnoit Road Meetinghouse Road Elevation 50 50

Geologic Conditions: Both abutments are outwash sand and gravel with some thin silt beds. Depth to shale bedrock in the foundation is estimated to be about 150 feet. Waterholding capabilities appear to be poor. Leakage is expected through both abutments and possibly through the foundation. Pervious borrow material for dam construction was located near the site; impervious material was not located.

Engineering Notes:

The right abutment is recommended for the excavated emergency spillway location. Inflow to the site may be sufficient to maintain a pool in spite of losses through the abutments and foundation. The downstream slope of the dam would be subject to tidal effects.

POTENTIAL SITE CC-3903

Location:

On an unnamed tributary to the Bumps River at Lumbert Pond about 50 feet upstream from Lumbert Mill Road in Barnstable, Mass.

Cotuit, Mass. USGS quadrangle

Latitude: 41°39'27" Longitude: 70°22'39"

Facilities Affected:

Elevation Facility 35 Nye Road 35 35 Road Utility poles Shed

Geologic Conditions: Both abutments are outwash sand and gravel. Depth to shale bedrock in the foundation is estimated to be about 150 feet. Waterholding capabilities appear to be poor. Leakage is expected through both abutments and the foundation. Pervious borrow material for dam construction was located near the site; impervious material was not located.

Engineering Notes:

The left abutment is recommended for the excavated emergency spillway location.

Site CC-3904(Santuit Pond)

Location:

On Santuit River about 3200 feet upstream from Route 130 in Mashpee, Mass.

Cotuit, Mass. USGS quadrangle

Surface Area

Surface Area Height of Drainage Area

(Acres) Dam(Ft.) (Acres) Sq. Mi.

7 800 1.3

Potential for

Expansion:

Raising the pond level 10 feet would affect several cranberry bogs. Sandy soils limit expansion potential. The small drainage area limits expansion.

Remarks:

The dam is an earthfill structure. The spillway is a timber flume with flashboard control. The flume is about 6 feet wide and has a wooden fish ladder on one side. There is about 3 feet of freeboard between the pond level and top of dam. Trees are growing on the dam. Santuit Pond is a groundwater pond with water level manipulated by flashboards.

Ownership and Use:

The pond is an enlarged Great Pond which is used for recreation.

Site CC-3905(Mill Pond)

Location:

On the Mills River at Route 149 in Barnstable, Mass.

Cotuit, Mass. USGS quadrangle

Surface Area Height of Drainage Area

(Acres) Dam(Ft.) (Acres) Sq. Mi.

8 1.750 2.7

Potential for

Expansion:

Topography limits any significant increase in surface area. Sandy soils may limit expansion potential.

Remarks:

Route 149 forms the dam. The spillway is a 50-foot long weir. Flow passes under Route 149 in a concrete pipe. There is also a stone fish ladder. The dam and spillway are in good condition.

Ownership and

The pond is owned by the Town of Barnstable and is used for recreation.

Use:

SUMMARY CATA FOR POTENTIAL UPSTREAM RESERVOIR SITES

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Site CC-3906(Lumbert Pond)

Location:

On an unnamed tributary to the Bumps River at Lumbert Mill Road in Barnstable, Mass.

Cotuit, Mass. USGS quadrangle

Surface Area (Acres)

Height of Dam(Ft.)

Drainage Area (Acres)

Sq. Mi.

Potential for

for Expansion:

Raising the pond level 10 feet would affect a cranberry bog. Sandy soils limit expansion potential.

Remarks:

Lumbert Mill Road forms the dam. The spillway is a concrete weir with flashboard control. The weir is about 4 feet long. The conduit under the road is two corrugated metal pipes. There is also a small outlet on the left abutment which is clogged with debris.

Ownership and Use:

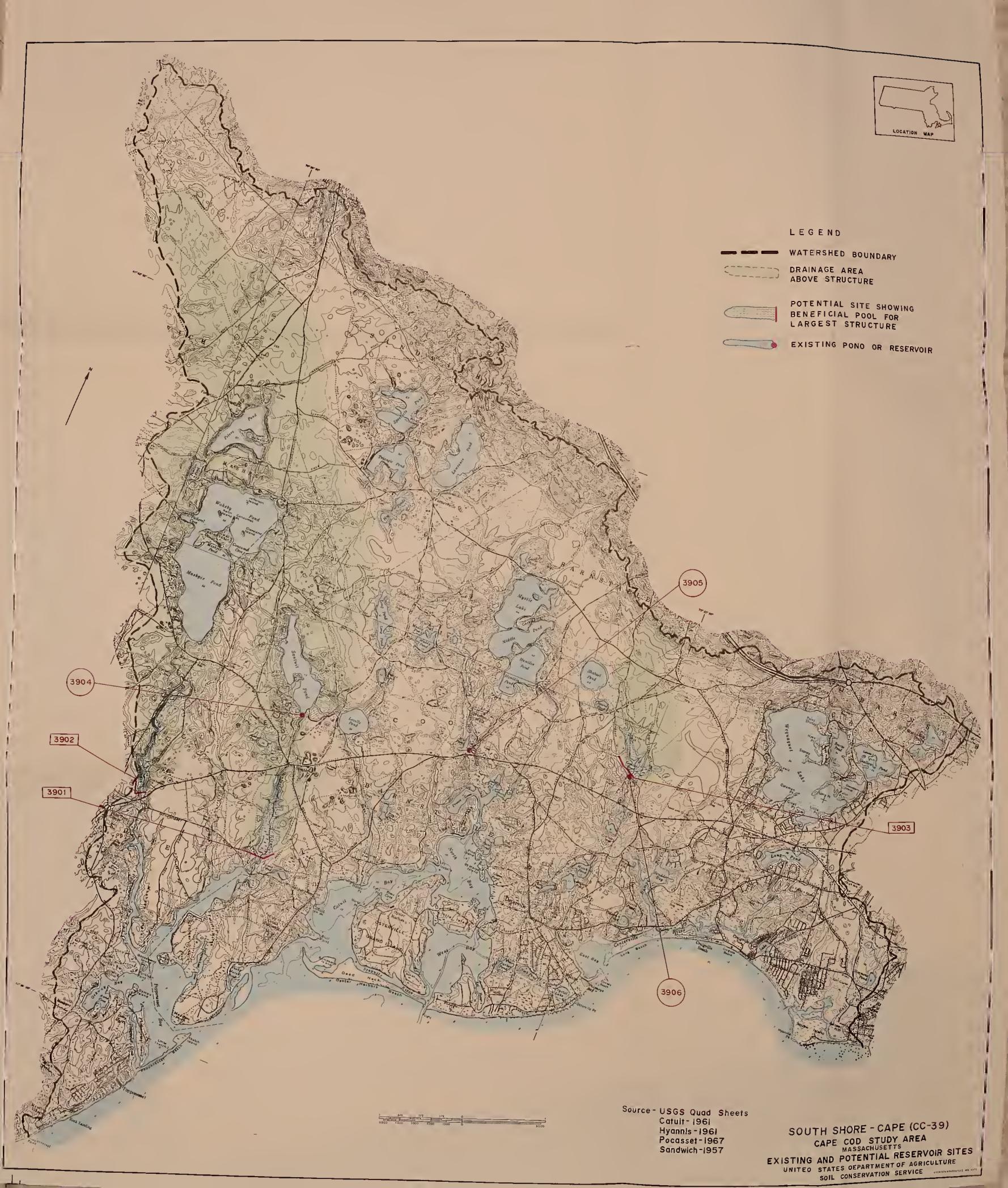
Ownership of the pond was not determined. The pond is for fishing.



Existing Site CC-3904 (Santuit Pond)



Existing Site CC-3905 (Mill Pond)





CAPE COD STUDY AREA SITE DATA FOR

Subwatershed CC-40, South Shore-Cape

The subwatershed covers about 64,100 acres in Bourne, Falmouth, Mashpee, and Sandwich in Barnstable County.

The major streams include the Coonamessett River and the Childs River. The Coonamessett originates in the northern portion of Falmouth and flows south to Vineyard Sound. The Childs River originates in Mashpee and also flows south to Vineyard Sound. Elevations range from a high of about 275 feet in Bourne to sea level.

Geology of the subwatershed is characterized by shale bedrock overlain by about 150 feet of outwash sand and gravel.

Four existing reservoirs were studied.

Site CC-4001(Mill Pond)

Location:

On Pocasset River about 250 feet downstream from County Road in Bourne, Mass.

Pocasset, Mass. USGS quadrangle

Surface Area Height of Drainage Area (Acres) Dam(Ft.) (Acres) Sq. Mi.

Potential for Expansion:

Raising the existing pond level about 10 feet would affect County Road and 5 houses. Surface area would be about 25 acres. Sandy soils limit expansion potential.

Remarks:

The dam is an earthfill structure. The downstream face is a vertical stone wall. The principal spillway is a concrete weir about 2 feet long with flashboard control. There is also a small spillway which also acts as a fish ladder. There is about one foot of free-board between the pond level and top of dam. It appears that the dam had recently washed out. Repairs have been made to the downstream end of the principal spillway and trees and brush have been removed from the dam.

Ownership and Use:

The pond is owned by John Tracy and is used for recreation.

Site CC-4002(Red Brook Pond)

Location:

On Red Brook at Shore Road in Bourne, Mass.

Pocasset, Mass. USGS quadrangle

Surface Area (Acres)___

Height of Drainage Area

Dam(Ft.) (Acres)

200

Potential for

Expansion:

The small drainage area and sandy soils limit expansion potential.

Remarks:

Shore Road forms the dam. The spillway is a concrete box culvert. The downstream end of the spillway has provision for flashboards. There is a concrete fish ladder to the right of the principal spillway. Flow from the principal spillway reaches a tidal pool by means of a concrete chute. Seepage was noted at the downstream toe of the roadfill. Concrete in the spillways is in good condition.

Ownership and Use:

The pond is owned by the town of Bourne and is used for recreation.

Site CC-4003(Flax Pond)

Location:

On an unnamed tributary to the Coonamessett River in Falmouth, Mass.

Falmouth, Mass. USGS quadrangle

Surface Area

rface Area Height of Drainage Area
(Acres) Dam(Ft.) (Acres) Sq. Mi.

Potential for Expansion: The small drainage area and sandy soils limit expansion potential.

Remarks:

The dam is an earthfill structure constructed of sandy materials. The spillway is a concrete chute about 5 feet wide and four feet deep. The pond is a dammed "kettle-hole". Trees and brush are growing on the upstream slope of the dam.

Ownership and Use:

The pond is owned by the Town of Falmouth and is used to store water for cranberry bog irrigation.

Site CC-4004(Mill Pond)

Location:

On the tributary to Green Pond at Route 28 in Falmouth, Mass.

Falmouth, Mass. USGS quadrangle

Surface Area
(Acres)
13

Height of Dam(Ft.)

Drainage Area (Acres)

Sq. Mi.

Potential for Expansion:

Cranberry bogs located immediately upstream would be affected by expansion. Surface area could be nearly doubled without affecting any other facilities. Sandy soils limit expansion potential.

Remarks:

Route 28 forms the dam. The downstream side is granite masonry and concrete. The spillway is a box conduit with a weir inlet about 4 feet long and 4 feet deep. The inlet has provision for flashboards although none are in use at present. Large trees are growing on both slopes of the dam. The pond is a dammed "kettle-hole".

Ownership and Use:

The pond is owned by the Town of Falmouth and is used to store water for cranberry bog irrigation.



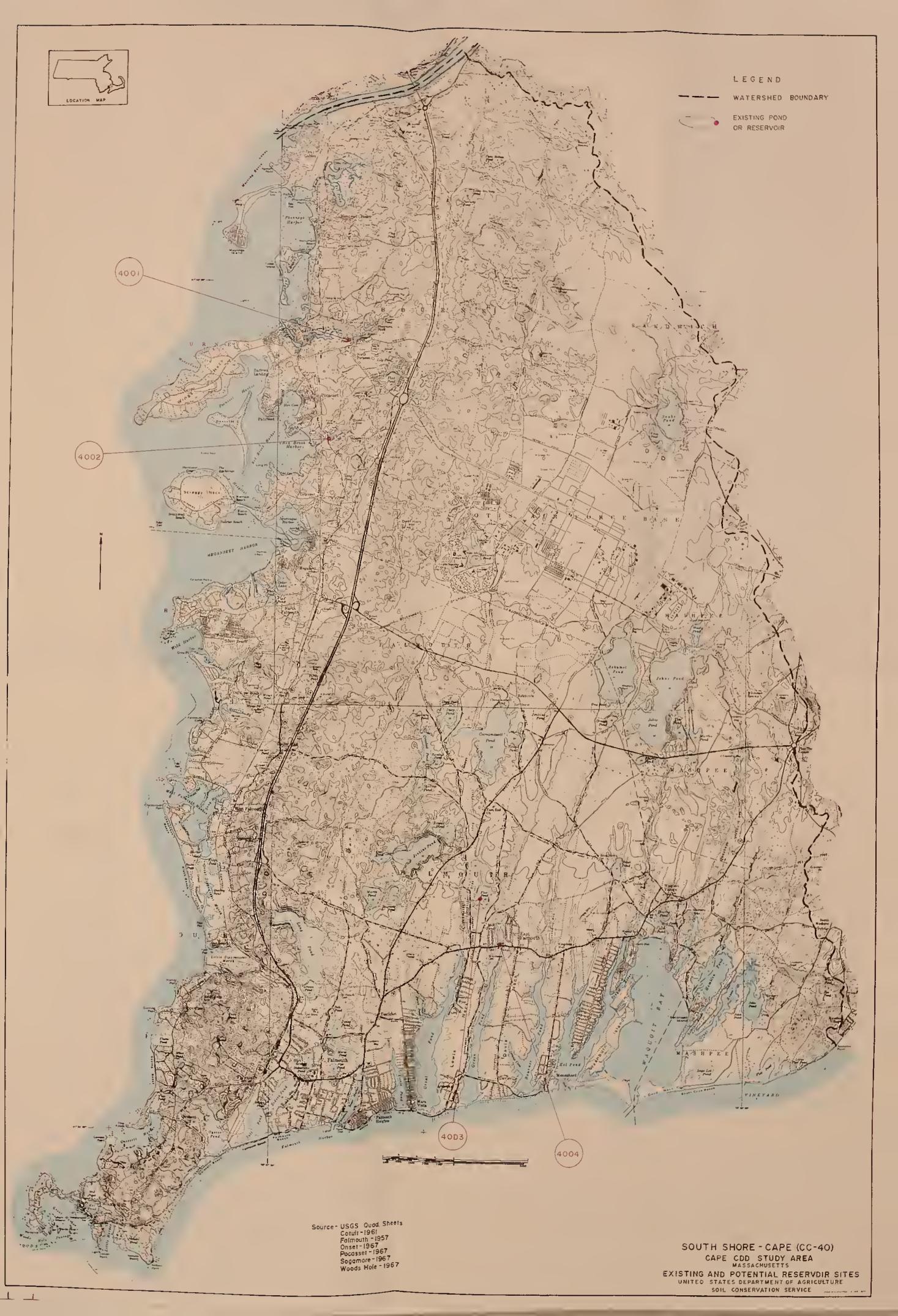
Existing Site CC-4001(Mill Pond)



Existing Site CC-4003 (Flax Pond)



Existing Site CC-4004 (Mill Pond)





BUZZARDS BAY STUDY AREA SITE DATA FOR

Subwatershed BB-41, Agawam River

This subwatershed covers about 48,700 acres in Carver, Plymouth and Wareham, in Plymouth County; and Bourne, in Barnstable County. There are three U.S. Geological Survey partial record stations in the subwatershed. These stations are located at Ellisville and East Wareham on the Agawam River, and at Wareham on the Wankinco River. The subwatershed is bordered on the south by Buzzards Bay, and on the southeast by the Cape Cod Canal.

Major streams in the subwatershed include the Agawam and Wankinco Rivers. The Agawam River originates in Halfway Pond in Plymouth and flows southwesterly to tidewater at the confluence with the Wareham River in Wareham. The Wankinco River originates in East Head Pond in Carver and flows generally southerly along the Carver-Plymouth boundary to its confluence at tidewater with the Wareham River in Wareham. Elevations range from a high of about 210 feet in Plymouth to sea level at Buzzards Bay.

Geology of the subwatershed is characterized by granitic bedrock overlain by 80 to 100 feet of outwash sand and gravel.

Three potential reservoir sites and 20 existing reservoirs were studied.

POTENTIAL SITE BB-4101

Location:

On an unnamed tributary to Harlow Brook about 800 feet upstream from the Wareham-Carver town line in Carver, Mass.

Wareham, Mass. USGS quadrangle

Latitude: 41°48'16" Longitude: 70°41'32"

Facilities Affected:

None below elevation 80

Geologic Conditions:

Both abutments are fine sand with some thin lenses of gravel. Depth to bedrock in the foundation is estimated to be from 80 to 100 feet. Waterholding capabilities appear to be poor; leakage is expected through both abutments and possibly through the foundation. Pervious material for dam construction was located near the site; impervious material was not located.

Engineering Notes:

The left abutment is recommended for the excavated emergency spillway location.

POTENTIAL SITE BB-4103

Location:

On the Agawam River about 1500 feet upstream from the Wareham-Plymouth town line in Plymouth, Mass.

Wareham, Mass. USGS quadrangle

Latitude: 41°48'29" Longitude: 70°38!35"

Facilities	Facility	Elevation
Affected:	House	65
	Barn	65
	5 sheds	60
	Agawam Road	50

Geologic Conditions: Both abutments are bedded outwash fine sand with some silt and gravel lenses. Depth to bedrock in the foundation is estimated to be from 90 to 100 feet. Waterholding capabilities appear to be poor; leakage is expected through both abutments and the foundation. Pervious borrow material for dam construction was located near the site; impervious material was not located.

Engineering Notes:

The right abutment is recommended for the excavated emergency spillway location.

POTENTIAL SITE BB-4104

Location:

On Red Brook about 4400 feet upstream from Red Brook Road in Wareham, Mass. Half of the dam and reservoir area are in Plymouth, Mass.

Wareham, Mass. USGS quadrangle

Latitude: 41°46'23" Longitude: 70°37'56"

Facility	Elevation
2 houses	50
Iouse	45
Shed	45
houses	40
Pipeline	40
	houses louse shed houses

Geologic Conditions:

Both abutments are bedded fine sand with some gravel lenses. Depth to bedrock in the foundation is estimated to be from 90 to 100 feet. Waterholding capabilities appear to be poor; leakage is expected through both abutments and the foundation. Borrow material for dam construction was located near the site.

Engineering Notes:

The right abutment is recommended for the excavated emergency spillway location.

SUMMARY DATA FOR POTENTIAL UPSTREAM RESERVOIR SITES

** DO NOT USE FOR FINAL SITE SELECTION OR LAND ACQUISITION. **

CONSIDERED ACCURATE TO THAT DEGREE.

Location:

On the Agawam River about 1,500 feet upstream from the Wareham-Plymouth town line in Plymouth, Mass.

Wareham, Mass. USGS quadrangle

Latitude: 41°48'29" Longitude: 70°38'35"

Surface Area (Acres)

Height of

Drainage Area Dam (Ft.) (Acres)

1.5 2,600

Sq. Miles

Potential for Expansion: Please refer to Site Data and Design Summary Table for Potential Site BB-4103.

Remarks:

The dam is an earthfill structure. The spillway is a covered concrete flume with provision for three bays of flashboards. There is a wooden fish ladder at the left side of the dam.

Ownership and Use:

The pond is owned by A. D. Makepeace and is used for cranberry bog irrigation and recreation.

EXISTING SITE BB-4105 (White Island Pond)

Location:

On Red Brook about 300 feet downstream from the Plymouth-Wareham town line in Wareham, Mass. The majority of the pond area is in Plymouth.

Sagamore and Wareham, Mass. USGS quadrangles

Latitude: 41°48'17" Longitude: 70°37'12"

Surface Area (Acres) 240

Dam (Ft.)

Height to Drainage Acre (Acres) 1,650

Sq. Miles

Potential for Expansion: Topography limits any significant increase in surface area. Sandy soils may limit expansion potential. The relatively small drainage area limits potential.

Remarks:

The dam is an earthfill structure. The spillway is a three bay concrete flume with flashboard channels.

Ownership and Use:

The pond is owned by the Commonwealth of Massachusetts and is used for irrigation water and recreation.

EXISTING SITE BB-4106 (Cranberry Reservoir)

Location:

On the Agawam River about 5,600 feet downstream from Halfway Pond in Plymouth, Mass.

Wareham and Sagamore, Mass. USGS quadrangles

Latitude: 41°49'58" Longitude:70°37'36"

Surface Area Height of Drainage Area
(Acres) Dam (Ft.) (Acres) (Sq. Miles)
15 15 1,050 1.64

Potential for Expansion: Limited. A relatively large area of shallow water would be created. Halfway Pond, located upstream, is only about 6 feet higher in elevation than this site.

Remarks:

The dam is an earthfill structure. There are four concrete flume spillways with flashboards. The main spillway near the center of the dam also has a wooden fish ladder to the right side of the structure.

Ownership and

The reservoir is owned by A.D. Makepeace and is used for irrigation of cranberry bogs.

Use:

EXISTING SITE BB-4107 (Fawn Pond)

Location:

On an unnamed tributary to the Agawam River about 50 feet upstream from Agawam Road in Plymouth, Mass.

Wareham, Mass. USGS quadrangle

Latitude: 41°49'26" Longitude:70°38'10"

Surface Area Height of Drainage Area
(Acres) Dam (Ft.) (Acres) (Sq. Miles)

45 - 400 0.63

Potential for

The small drainage area limits expansion potential.

Expansion:

Remarks:

This is basically a ground water pond. The outlet structure is a concrete riser with flashboards and a concrete pipe conduit. A pumphouse is located near the outlet structure.

Ownership and

The pond is owned by A. D. Makepeace Co. and is used for cranberry bog irrigation and recreation.

Use:

Location:

On Maple Springs Brook about 3,400 feet downstream from the Plymouth-Wareham town line in Wareham, Mass.

Wareham, Mass. USGS quadrangle

Latitude:41⁰47'48"

Longitude:70°39'21"

Surface Area (Acres)

Height of Dam (Fi.) Drainage Area (Acres)

Potential for Expansion: Raising the present pond level by 10 feet would more than double the surface area. No facilities would be affected. Sandy soils may limit expansion potential.

Remarks:

The dam is an earthfill structure. The principal spillway is a concrete drop inlet with sloping timber flashboards.

Ownership and Use:

The pond is owned by A. D. Makepeace and is used for cranberry bog irrigation.

EXISTING SITE BB-4109 (Glen Charlie Pond)

Location:

On the Agawam River about 3,800 feet southwest of the Wareham Road - Glen Charlie Road intersection in Wareham, Mass.

Wareham, Mass. USGS quadrangle

Latitude: 41^o47'04" Longitude: 70^o39'16"

Surface Area (Acres)

Height of Dam (Ft.)
15

Drainage Area (Acres)

(Sq. Miles)

Potential for Expansion: Topography limits any significant increase in surface area.

Remarks:

The dam is a sand-fill structure with a concrete cut-off wall. The principal spillway is a concrete chute with provision for flashboards along the spillway crest. There is also a small gated spillway with a concrete drop structure located to the left of the principal spillway. There is a concrete and wood fish ladder about 1,000 feet long. Concrete in the principal spillway is cracked and spalled. Wood portions of the fish ladder are deteriorated and broken.

EXISTING SITE BB-4109 (Glen Charlie Pond) cont'd)

Ownership and

The pond is owned by Francis Ortolani and is used for cranberry bog irrigation.

Use:

EXISTING SITE BB-4110 (Mill Pond)

Location:

On the Agawam River at Route 28 in Wareham, Mass.

Wareham, Mass. USGS quadrangle

Latitude: 41°45'45" Longitude: 70°40'36"

Surface Area (Acres)

Height of Dam (Ft.)

Drainage Area (Acres) 6,000

 $(\underline{Sq. Miles})$

Potential for

Expansion:

Limited. The pond is surrounded by residential areas which would be affected by expansion. Upstream cranberry bogs would also be affected.

Remarks:

The main dam is the Route 28 and 6 roadway embankment. There is also a secondary dam perpendicular to Route 28. At the east end of the secondary dam is a small concrete structure with a fish ladder and a pipe spillway under Route 28. There is a reversed concrete flume near the left edge of the pond. Near the right edge is a stone masonry drop inlet spillway with a steel conduit. Mill Pond controls the water level in several upstream ponds.

Ownership and Use:

The pond is owned by Adams and Gould Lumber Corporation and is used for cranberry bog irrigation and recreation

Location:

On an unnamed tributary to Buttermilk Bay about 600 feet upstream from the Bourne-Plymouth town line in Plymouth, Mass.

Sagamore. Mass. USGS quadrangle

Latitude: 41°46'14"

Longitude: 70°37'03"

Surface Area (Acres)

Height of Dam (Ft.)

Drainage Area (Acres)

(Sq. Miles)

Potential for

Expansion:

Expansion would affect upstream cranberry bogs. The sandy soils may also limit expansion potential.

Remarks:

The dam is an earthfill structure. The spillway is a sheet steel riser with a corrugated metal pipe conduit. The riser has provision for flashboards.

Ownership and Use:

The pond is owned by David Mann and is used for cranberry bog irrigation.

EXISTING SITE BB-4112 (Dicks Pond)

Location:

On Gibbs Brook about 900 feet upstream from Routes 6 and 28 in Wareham, Mass.

Wareham, Mass. USGS quadrangle

Latitude: 41^o45'31"

Longitude: 70°39'16"

Surface Area Height of (Acres)

Dam (Ft.)

Drainage Area (Acres)

Potential for Expansion: Limited. A much longer dam would be required. Several cranberry bogs would be affected.

Remarks:

The dam is an earthfill structure. The outlet is a small concrete flume with flashboards. Water is pumped onto adjacent bogs and flows back to the pond by gravity.

Ownership and Use:

Ownership was not determined. The pond is used by Daniel O'Connor for cranberry bog irrigation and recreation.

On Frog Foot Brook about 2,600 feet upstream from the Location:

Wareham-Plymouth town line in Plymouth, Mass.

Wareham. Mass. USGS quadrangle

Latitude: 41°48'26" Longitude: 70°42'11"

Surface Area Height of Drainage Area
(Acres) Dam (Ft.) (Acres) (Sq. Miles)

20 17 550 0.86

Raising the pond level by ten feet would more than double Potential the surface area. An unpaved road would be affected. for Expansion:

Remarks: The dam is an earthfill structure. There are three outlet structures. Two are closed concrete conduits with flashboardcontrolled inlets. The other spillway is a double-bay con-

crete conduit with flashboard control.

The pond is owned by A. D. Makepeace and is used for cran-Ownership and berry bog irrigation.

Use:

i<mark>nicial del del contrato</mark> de la contratorio del contratorio de la contratorio de la contratorio del contratorio de la c

EXISTING SITE BB=4114 (Tihonet Pond)

Location: On the Wankinco River about 200 feet upstream from Tihonet

Road in Wareham, Mass.

Wareham, Mass. USGS quadrangle

Latitude: 41°47'15" Longitude: 70°43'08"

Surface Area Height of Drainage Area (Acres)

85 Dam (Ft.) (Acres)

3,750 (<u>Sq. Miles</u>)

Potential Raising the pond level by 10 feet would nearly double the surface area. Several upstream cranberry bogs would for

be affected. Expansion:

Remarks: The dam is an earthfill structure. There are three outlets. One is a fairly new two-bay timber flume which outlets to a 36-inch corrugated metal pipe. There is also a two-bay concrete drop inlet with flashboards. In addition, there is a two-bay concrete drop inlet which outlets to a steel

conduit and then to a concrete chute.

Ownership The pond is owned by A. D. Makepeace Company and is used and for cranberry bog irrigation and recreation. Use:

EXISTING SITE BB-4115 (Parker Mills Pond)

On the Wankinco River at Route 28 in Wareham, Mass. Location:

Wareham, Mass. USGS quadrangle

Latitude: 41^o46'02"

Longitude: 70°43'22"

Surface Area Height of Drainage Area (Acres)

Dam (Ft.)
18

(Acres) (Sq. Miles) 11,450 17.89

Potential for

Raising the present pond level by 5 feet would more than double the surface area.

Expansion:

Remarks:

Route 28 forms the dam. The upstream slope is faced with stone. The spillway is a concrete drop structure with a concrete chute to the stream. Chute sides are stone masonry. There is also a concrete fish ladder and concrete flume. The drop structure originally had timber steps which were concrete capped. Concrete in the fish ladder is broken, missing, and undermined. Some pieces of stone masonry from the chute spillway have fallen into the channel.

Ownership and Use:

Ownership of the pond was not determined. The pond is used by Tremont Nail Company for industrial use and is used for recreation.

EXISTING SITE BB-4116 (Golden Field Pond)

Location: On an unnamed tributary to the Wankinco River about 2,200 feet west of Federal Street in Carver, Mass.

Wareham, Mass. USGS quadrangle

Latitude: 41°49'09"

Longitude: 70^o43'36"

Surface Area Height of Drainage Area

(Acres) Dam (Ft.) (Acres) (Sq. Miles)

15

9

125

0.20

Potential for Expansion: The small drainage area limits expansion potential.

Remarks:

The dam is an earthfill structure. The spillway is a large three-bay concrete flume with provision for flashboards. There is also a concrete flume at the east side of the pond. A lift pump is used to return water from cranberry bogs to the pond. There is a pumphouse located over the main spillway.

EXISTING SITE BB-4116 (Golden Field Pond) cont'd

Ownership and

The pond is owned by Smith-Hammond Cranberry Company and is used for cranberry bog irrigation.

Use:

EXISTING SITE BB-4117

Location:

On an unnamed tributary to Rose Brook about 500 feet south of the Carver-Wareham town line in Wareham, Mass.

Wareham, Mass. USGS quadrangle

Latitude: 41°47'56"

Longitude: 70°43'56"

Surface Area
(Acres)

Height of Dam (Ft.)

Drainage Area
(Acres)

(Sq. Miles)

Potential for

Expansion:

Expansion would affect several cranberry bogs. Sandy soil may limit expansion potential. A much longer dam would be needed.

Remarks:

The dam is an earthfill structure surrounded on three sides by cranberry bogs. A total of eleven concrete or timber flumes serve the bogs.

Ownership and

The pond is owned by Robert Hammond and is used for cranberry bog irrigation.

Use:

indende de la companda del companda del companda de la companda del companda de la companda del companda de la companda del companda de la companda del companda de la companda de la companda de la companda de la companda del com

Location: On Rose Brook about 7,000 feet downstream from the Carver-Wareham town line in Wareham, Mass.

Wareham, Mass. USGS quadrangle

Latitude: 41°46'54" Longitude: 70°44'12"

Surface Area Height of Drainage Area

(Acres) Dam (Ft.) (Acres) (Sq. Miles)

10 15 1,600 2.50

Potential Limited. Several cranberry bogs are located immediately for upstream. Sandy soils may limit expansion potential. Expansion:

Remarks: The dam is an earthfill structure. The dam is vegetated and has some trees on the upstream slope. There are three concrete outlet structures. One near the left abutment is a single-bay flume. Near the center of the dam is a two-bay flume. The third structure near the right abutment is blocked by silt. Concrete in the right structure is badly spalled. Erosion has occurred near the center structure outlet. The dam and abutments are sand.

Ownership The pond is owned by Cranberry Corporation of America and is and used for cranberry bog irrigation.
Use:

EXISTING SITE BB-4120 (East Head Pond)

Location: On the Wankinco River at East Head Road on the Carver-Plymouth town line.

Wareham, Mass. USGS quadrangle

Latitude: 41°50'20" Longitude: 70°41'25"

Surface Area Height of Drainage Area
(Acres) Dam (Ft.) (Acres) (Sq. Miles)

90 13 350 0.55

Potential Topography limits any significant increase in surface area. for The relatively small drainage area also limits expansion Expansion: potential.

Remarks: East Head Road forms the dam. The upstream slope is faced with stone. There are two concrete drop inlet flume spillways.

Ownership The pond is owned by A. D. Makepeace Company and is used and for cranberry bog irrigation and recreation.

Use:

EXISTING SITE BB-4121 (Mosquito Pond)

Location:

On an unnamed tributary to the Wankinco River about 2,500 feet upstream from Tihonet Road in Wareham, Mass.

Wareham, Mass. USGS quadrangle

Latitude: 41^o47'27"

Longitude: 70°43'46"

Surface Area Height of Drainage Area*

(Acres) Dam (Ft.) (Acres)

7 975

Potential for Expansion: Limited. Several upstream cranberry bogs would be affected. Sandy soils may limit expansion potential.

Remarks:

The dam is an earthfill structure. Mosquito Pond is separated into two bodies of water by a narrow dike. There are two spillways; a timber drop inlet structure and a concrete drop inlet and concrete pipe conduit. Both spillways have provision for flashboards.

Ownership and Use:

The pond is owned by A. D. Makepeace Company and is used for cranberry bog irrigation.

*Drainage area may be inaccurate due to complicated cranberry bog system located upstream.

EXISTING SITE BB-4122

Location:

On an unnamed tributary to Maple Springs Brook 2,600 feet upstream from Maple Springs Road in Wareham, Mass.

Wareham, Mass. USGS quadrangle

Latitude: 41⁰47'21"

Longitude: 70^o40'35"

Surface Area (Acres)

Height of Dam (Ft.)

Drainage Area (Acres)

Potential for Expansion: Sandy soils may limit expansion potential.

Remarks:

The dam is an earthfill structure. Timbers along the upstream face provide some wave-erosion control. The main spillway is a covered timber flume. There are also three other outlets; a concrete conduit, concrete drop inlet and concrete flume which serve cranberry bogs.

Ownership and Use:

The pond is owned by Francis Ortolani and is used for cranberry bog irrigation.

Location:

On an unnamed tributary to the Agawam River about 1.000 feet west of Agawam Road in Wareham, Mass.

Wareham, Mass. USGS quadrangle

Latitude: 41^o47'07"

Longitude: 70°39'34"

Surface Area Height of (Acres)

Dam (Ft.)
15

Drainage Area*

(Acres) 50

Potential

Flat topography limits expansion potential.

for

Expansion:

Remarks:

The dam is an earthfill structure. There are three outlets; a concrete drop inlet with a concrete box conduit, a four-bay concrete flume with flashboards, and another concrete flume with flashboards. A concrete flume inlets water from Glen Charlie Pond.

Ownership and Use:

Ownership of the pond was not determined. The pond is used for cranberry bog irrigation and recreation.

*This is the direct drainage area only. Water is also diverted into the pond from Glen Charlie Pond.

EXISTING SITE BB-4124

Location:

On Rose Brook about 1,400 feet upstream from the Wareham-Carver town line in Carver, Mass.

Wareham, Mass. USGS quadrangle

Latitude: 41⁰48'15"

Longitude: 70⁰44!20"

Surface Area (Acres)

Height of Dam (Ft.)

Drainage Area (Acres) 850

Potential for

Limited very flat topography would cause many upstream cranberry bogs to be affected by expansion.

Expansion: Remarks:

The dam is an earthfill structure. The upstream slope is riprapped near the outlet structure and at water level. The outlet is a capped concrete flume with flashboards.

Ownership and

The pond is owned by A. D. Makepeace Company and is used for cranberry bog irrigation.

Use:



BB - 4103



8B - h107 Fawn Pond



88 - 4105 White Island Pond



BB - 4106 Cranberry Pond



88 - 4108







BB - 4109 Glen Charlie Pond





BB - 4110 Mill Pond

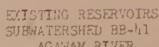






38 - 4111









BB-4114 Tihonet Pond



88-4117



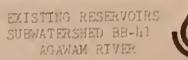
BB-4115 Parker Mills Pond



BB-4118



BB-4116 Golden Field Pond







BB - 4120 East Head Pond



BB - 4121 Mosquito Pond

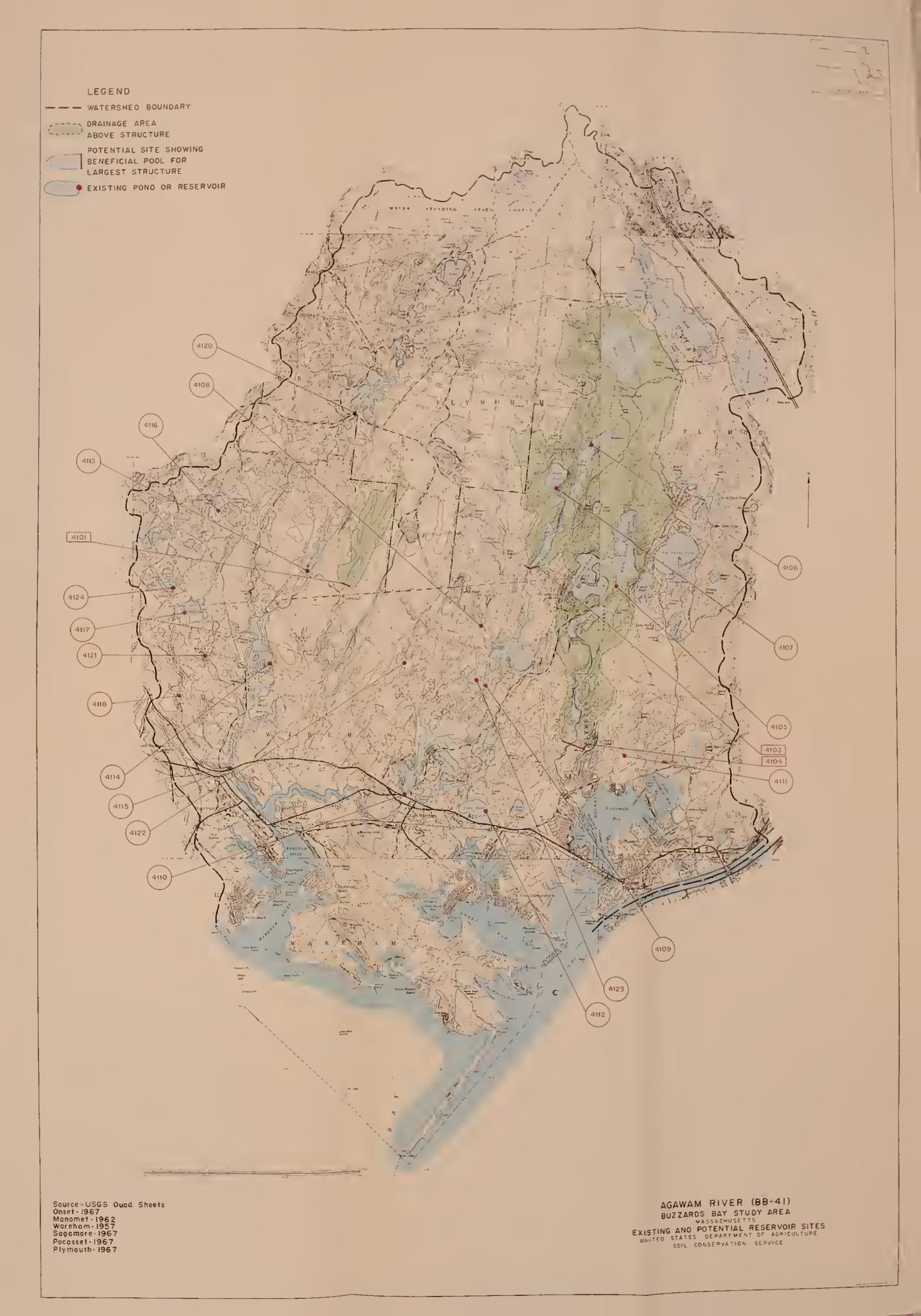


BB - 4122











BUZZARDS BAY STUDY AREA SITE DATA FOR

Subwatershed BB-42, Weweantic River

This subwatershed covers about 28,700 acres in Carver, Kingston, Middleborough, Plymouth, Rochester and Wareham, in Plymouth County. There is a U.S. Geological Survey stream gaging station on the Weweantic River at South Wareham. This subwatershed was considered as a potential multiple purpose flood control project under Public Law 566, however preliminary investigations were terminated because of insufficient local interest.

The Weweantic River is the major stream in the subwatershed. It originates as South Meadow Brook in Plymouth and flows generally southerly along the Carver-Middleborough border, to tidewater at Buzzards Bay in Wareham. Elevations range from a high of about 190 feet in Plymouth to sea level at Buzzards Bay.

Geology of the subwatershed is characterized by granitic bedrock overlain by 25 to 35 feet of outwash sand and gravel.

One potential reservoir site and 2 existing reservoirs were studied.

POTENTIAL SITE BB-4201

Location:

On the Weweantic River about 200 feet upstream from Rochester Road in Carver, Mass. Half of the dam and reservoir area is in Middleboro, Mass.

Snipatuit Pond, Mass. USGS quadrangle

Latitude: 41°49'48" Longitude: 70°46'21"

Facilities	Facility Pacility	Elevation
Affected:	37 houses	85
	34 garages	85
	3 houses	80
	2 barns	80
	2 sheds	80

Geologic Conditions:

Both abutments are outwash sand and gravel with cobbles and boulders. Depth to bedrock in the foundation is estimated to be from 25 to 35 feet. Waterholding capabilities appear to be poor to fair; leakage is expected through both abutments and the foundation. Pervious borrow material for dam construction was located near the site; impervious material was not located.

Engineering Notes:

Preliminary designs indicate that a concrete drop structure emergency spillway will probably be required at this site.

SUMMARY DATA FOR POTENTIAL UPSTREAM RESERVOIR SITES

* * * * * * * * * * * * * * * * * * * *	FE LD	CENT	3D)	46-21 CFS	, 35	1.87 3.28		* * * * *
1	* SAFE * YIELD	*PERCENT *CHANCE	# (H	70-4	* *	* *	* * *	* * *
		FILL *PERCENT VOL *CHANCE (1000 *	(CY) * (MGD)	VGITUDE		19		*****
	рам	HGT	FT	48 LOP PEAK FL	17	16 15		****
ËR		+ TOP + ELEV	(MSL)	41-49-	86.6	86.1 85.0		*****
IC RIVE	Z F E R R	ELEV AREA * ELEV	(AC) *	TITUDE = 5.0	* 1526 *	82.0 1689 * 82.0 1671 *	* * *	****
WEWEANT	DESIGN HIGH WATER	ELEV	(MSL)	RUNOFF	81.6	82.0		* * * * * *
SUBWATERSHED WEWEANTIC RIVER		COST * COST A CO	(MSL) AC FI IN (4) + (MSL) + (MSL) + (MSL) FI CY) + (MGD) 	USGS QUAD-SNIPATUIT POND, MASS LATITUDE 41-49-48 LONGITUDE 70-46-21 100-YR PRIN SPWY DESIGN STORM RUNDFF = 5.00 IN, PEAK FLOW = 2383 CFS	1680 *	3720 * 2250 *		***
SUBWAT	ILLWAY		NI ***	ATUIT P	0.2 1	77.1 D 1227 0.8 3720 * 78.6 D 2117 1.2 2250 *		中央市市市市市市市市市市市市市市市市市市市市市市市市市市市市市市市市市市市市
	EMERGENCY SPILLWAY	STORAGE AT CREST	AC FT IN	D-SNIP	337	1227		***
	EMERGE		(MSL) A	ISGS QUA	74.6 N	77.1 D 78.6 D		***
	* *		s #	10		* *	* * *	
S BAY		COST/ DEPTH SURF AT AC DAM	(FT)	8989 A		7.1		***
UZZARD		COST/ AREA SURF AC	(\$)	MI = 1	17170	506 9030 679 7010		本本本本本本
STUDY AREA-BUZZARDS BAY	00F	AREA	(4) (VC) (4) (LL)	DA= 29.67 SQ MI = 18989 AC STREAM WATER QUALITY (B)	230	506		****
STUDY	BENEFICIAL POOL	COST PER AC FT	(\$)	DA= 29. STRE	39400	990 0.6 4620 1880 1.2 2530		****
1	BENEFI	STORAGE	Z	(3)	0.1	0.6		*****
1		STO	(MSL) AC FT IN	TE-BB-4201 SITE RATING (3)	100	1880		*****
1 1 1	**************************************	COST COST COST DEPTH ELEV STORAGE PER AREA SURF AT AC FT AC DAM	(ASL) AC FI IN (4) (AC) (4) (FI)	SITE-BB-4201 SITE RATING	14.6	77.1		中中中市市市市市市市市市市市市市市市市市市市市市市市市市市市市市市市市市市

EMERGENCY SPILLWAY STORAGE AND COSTS ARE BASED ON TOTAL STORAGE, INCLUDING BENEFICIAL POOL.

EMERGENCY SPILLWAY TYPE CODE— C=CONCRETE CHUTE, D=CONCRETE DROP, E=EXCAVATED, T= TWO SPILLWAYS, N= NONE
TABULAR DATA ARE BASED ON PRELIMINARY INFORMATION. FIGURES SHOWN ARE PRIMARILY FOR COMPARISON PURPOSES.

ELEVATIONS ARE SHOWN TO THE NEAREST 0.1 FOOT TO SHOW VARIATION BETWEEN DEVELOPMENTS ONLY, AND ARE NOT TO BE
CONSIDERED ACCURATE TO THAT DEGREE.

** DO NOT USE FOR FINAL SITE SELECTION OR LAND ACQUISITION. ** 2645

The following potential sites were identified in the Public Law-566 preliminary watershed investigation of the Weweantic River. Many of the sites did not meet the criteria for detailed study in this inventory. For further information concerning these sites, please refer to the "Reconnaissance Investigation Report for the Weweantic River Watershed", March 1971 or contact the U. S. Department of Agriculture, Soil Conservation Service, Amherst, Massachusetts 01002.

POTENTIAL SITE BB-4212

Location:

On South Meadow Brook about 1,000 feet downstream from the Plymouth-Carver town line in Carver, Mass.

Plymouth, Mass.

USGS quadrangle

POTENTIAL SITE BB-4215

Location:

On South Meadow Brook about 4,000 feet east of Route 58 in Carver, Mass.

Plympton, and Plymouth, Mass. USGS quadrangles

POTENTIAL SITE BB-4217

Location:

On South Meadow Brook about 200 feet upstream from Popes Points Street in Carver, Mass.

Snipatuit Pond, Mass.

USGS quadrangle

POTENTIAL SITE BB-4221

Location:

On the Weweantic River about 200 feet upstream from Rochester Road in Carver, Mass.

Snipatuit Pond, Mass.

USGS quadrangle

POTENTIAL SITE BB-4222

Location:

On the Weweantic River about 1,500 feet downstream from Route 58 in Carver, Mass.

Snipatuit Pond, Mass.

USGS quadrangle

POTENTIAL SITE BB-4235

Location:

On Rocky Meades Brook about 700 feet upstream from the Carver-Middleborough fown line in Middleborough, Mass.

Plympton, Mass.

USGS quadrangle

POTENTIAL SITE BB-4237

Location:

On Double Brook about 500 feet upstream from France Street in Middleborough, Mass.

Snipatuit Pond, Mass.

USGS quadrangle

POTENTIAL SITE BB-4238

Location:

On an unnamed tributary to South Meadow Brook about 1,200 feet east of South Meadow Street in Carver, Mass.

Plymouth, Mass.

USGS quadrangle

Location:

On the Weweantic River about 500 feet downstream from Main Street in Wareham, Mass.

Snipatuit Pond, Mass. USGS quadrangle

Latitude: 41^o47'23" Longitude: 70^o45'54"

Surface Area Height of Drainage Area
(Acres) Dam (Ft.) (Acres) (Sq. Miles)
27 26,700 41.72

Potential for

Flat topography limits expansion potential. Sandy soils may limit expansion potential.

Expansion:

Remarks:

The dam is an earthfill structure faced with stone. The spillway is a two-bay concrete ogee weir with timber gates to control the water level. An old power plant building is located near the spillway. Concrete in the spillway has surface cracks and is badly spalled. There is seepage along the entire length of the embankment.

Ownership and

The pond is owned by the town of Wareham and is used for recreation.

Use:

EXISTING SITE BB-4203 (Horseshoe Pond)

Location:

On the Weweantic River at tidewater in Wareham, Mass.

Wareham, Mass. USGS quadrangle

Latitude: 41⁰45'53"

Longitude: 70⁰44'52"

Surface AreaHeight of
(Acres)Drainage Area
(Acres)(Sq. Miles)851128,55044.61

Potential Expansion:

Raising the existing pond level by 10 feet would about double the surface area. Three streets would be affected. Sandy soils may limit expansion potential.

Remarks:

The dam is an earthfill mill dam. The principal spillway is a multi-bay concrete drop structure with provision for flashboards. The center bay has a mechanical gate to allow draining of the pond. There is also a secondary concrete chute spillway. The concrete in the principal spillway is cracked and spalled.

Ownership and Use:

The pond is owned by H. & E. Robbins and is used for recreation.

EXISTING SITE BB-4204 (Blackmere Reservoir)

Location:

On Beaverdam Creek about 1,700 feet upstream from Route 6 in Wareham, Mass.

Wareham, Mass. USGS quadrangle

Latitude: 41°48'17"

Longitude: 70°44'10"

Surface Area (Acres)

Height of Dam (Ft.)

Drainage Area

(Acres)

 $\frac{(Sq. Miles)}{0.16}$

Potential for

The small drainage area limits expansion potential.

Expansion:

Remarks:

The dam is an earthfill structure. The principal spillway

is a sheet-steel drop inlet with a corrugated metal pipe conduit. There is also a timber flume located to the west

of the principal spillway.

Ownership and

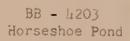
Use:

The reservoir is owned by David Mann and is used for cran-

berry bog irrigation.



BB - 1,202





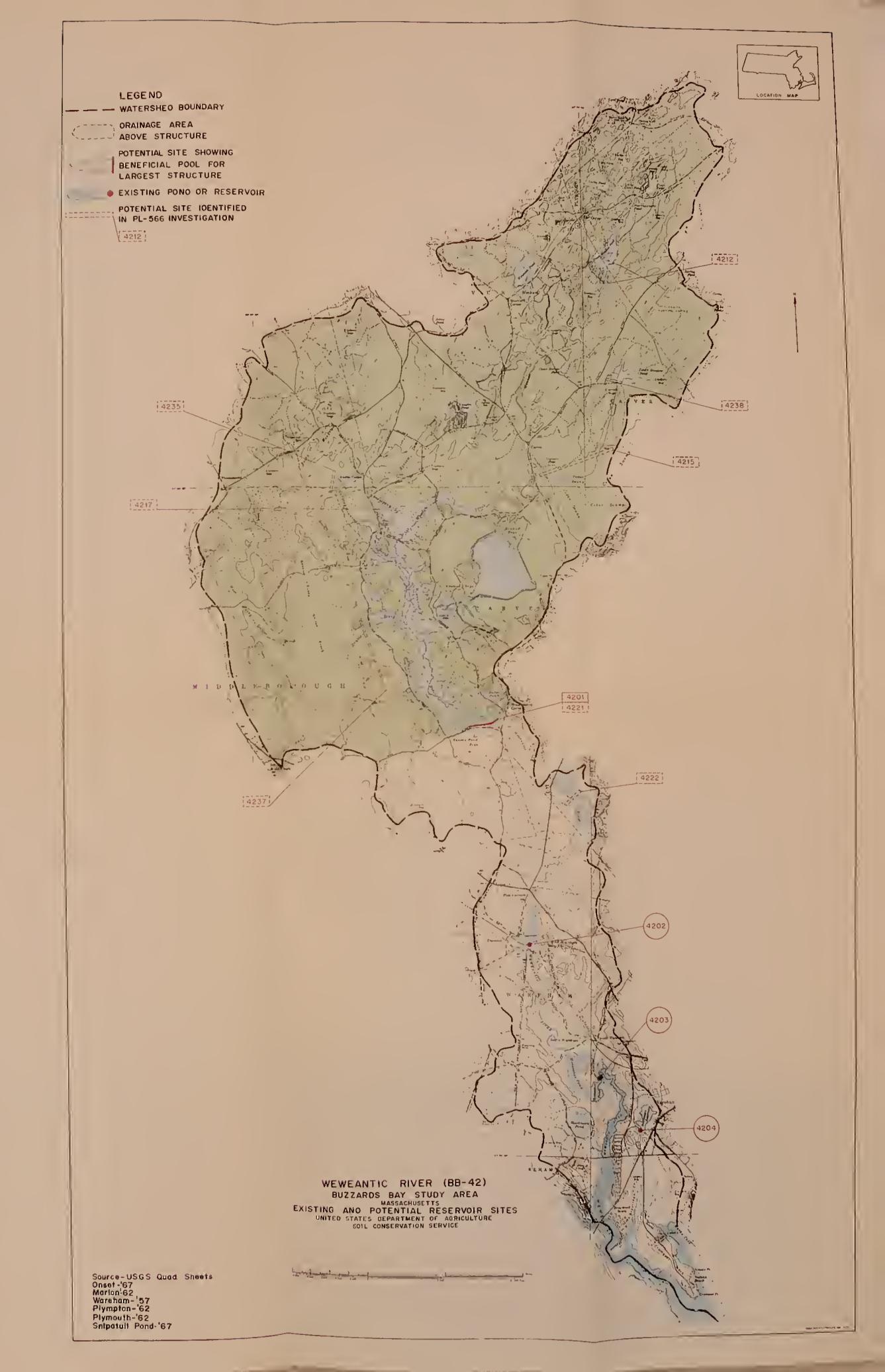


BB-4204 Blackmere Reservoir

EXISTING RESERVOIRS
SUBJECTED BB-42
WENEAUTIC RIVER









BUZZARDS BAY STUDY AREA SITE DATA FOR

Subwatershed BB-43, Crane Brook

This subwatershed covers about 9,500 acres in Carver and Plymouth in Plymouth County. This subwatershed was considered as part of a potential flood control project in the Weweantic River Watershed. Preliminary investigations were terminated because of insufficient local interest.

Crane Brook is the major stream in the subwatershed. It originates in Plymouth and flows generally southwesterly to the confluence with the Weweantic River in Carver. Crane Brook flows through cranberry bogs for most of its length. Elevations range from a high of about 210 feet in Plymouth to a low of about 70 feet in Carver. Geology of the subwatershed is characterized by granite bedrock overlain by 25 to 50 feet of outwash sand and gravel.

No potential or existing reservoir sites which met inventory criteria were located in this subwatershed.

The following potential sites were identified in the Public Law-566 preliminary watershed investigation of the Weweantic River. For further information concerning these sites, please refer to the "Reconnaissance Investigation Report for the Weweantic River Watershed", March 1971, or contact the U.S. Department of Agriculture, Soil Conservation Service, Amherst, Mass. 01002.

POTENTIAL SITE BB-4339

Location:

On an unnamed tributary to Crane Brook in Cedar Swamp about 4,500 feet west of Tremont Street in Carver, Mass.

Plymouth, Mass. USGS quadrangle

POTENTIAL SITE BB-4340

Location:

On Crane Brook at Federal Pond about 700 feet west of the Plymouth-Carver town line in Carver, Mass.

Wareham, Mass. USGS quadrangle

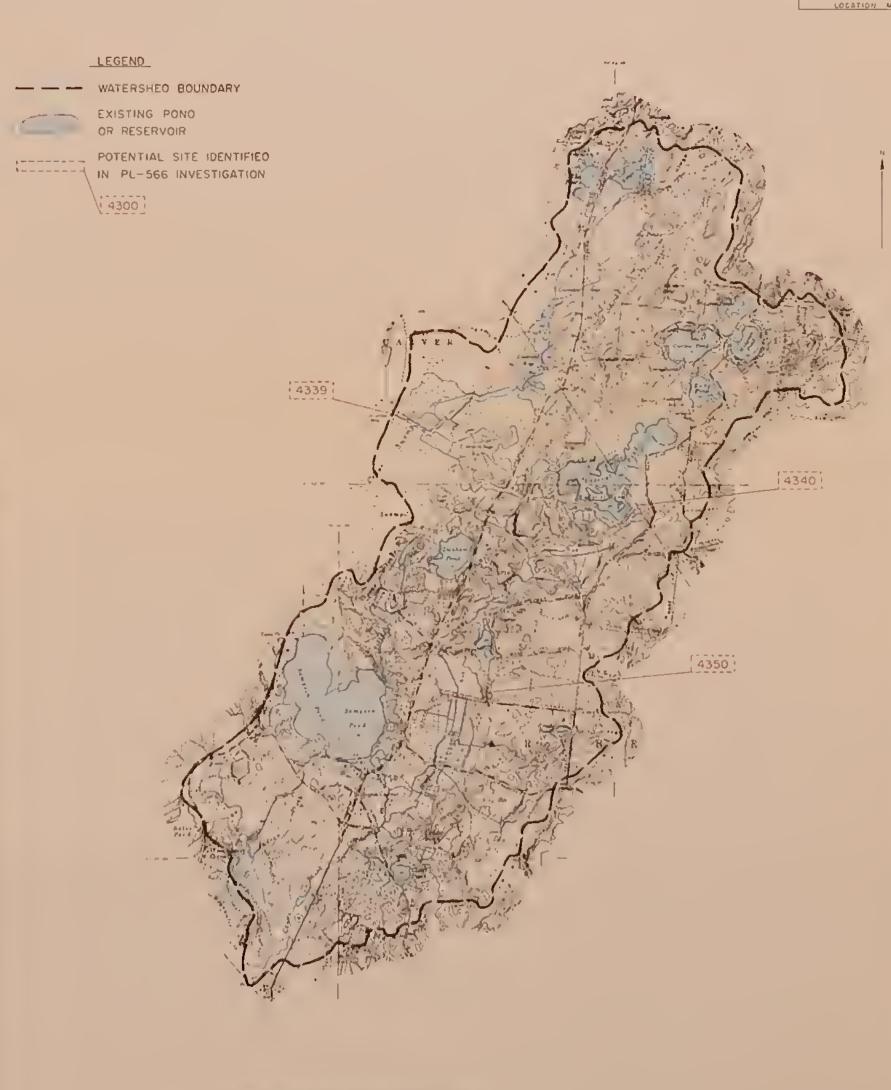
POTENTIAL SITE BB-4350

Location:

On Crane Brook about 3,000 feet upstream from Cranberry Road in Carver, Mass.

Wareham, Mass. USGS quadrangle





Source - USGS Quied Sheets

Plymouth, Mass - 1952 Wareham, Mass - 1957 Shipaniii, Mass - 1962 CRANE BROOK (BB-43)
BUZZARDS BAY STUDY AREA
MASSACHUSETTS

EXISTING AND POTENTIAL RESERVOIR SITES
UNITED STATES DEPARTMENT OF AGRICULTURE
SOIL CONSERVATION SERVICE



BUZZARDS BAY STUDY AREA SITE DATA FOR

Subwatershed BB-44, Mattapoisett River

This subwatershed covers about 53,700 acres in Acushnet and Fairhaven in Bristol County; and Marion, Mattapoisett, Middleborough, Rochester and Wareham in Plymouth County.

Major streams in the subwatershed include the Mattapoisett and Sippican Rivers. The Mattapoisett River originates in Rochester and flows south through Mattapoisett to the Atlantic Ocean. The Sippican River also originates in Rochester and flows southeasterly to the Weweantic River in Marion. Elevations range from a high of 155 feet in South Middleboro to sea level in Mattapoisett. Geology of the subwatershed is characterized by granite bedrock overlain by 30 to 50 feet of outwash sand and gravel.

Three potential reservoir sites and 9 existing reservoirs were studied.

POTENTIAL SITE BB-4401

Location:

On an unnamed tributary to Mattapoisett River about 100 feet upstream from Tinkham Lane in Mattapoisett, Mass. Most of the reservoir area is in Acushnet, Mass.

Marion, Mass. USGS quadrangle

Latitude: 41°40'54" Longitude: 70°51'27"

Facilities	Facility	Elevation
Affected:	House	35
	Garage	35
	2 sheds	35
	House	30
	Garage	30
	2 sheds	30

Geologic Conditions: Both abutments are outwash sand and gravel with some large boulders. Bedrock outcrops high on the left abutment. Depth to granite bedrock in the foundation is estimated to be from 40 to 50 feet. Waterholding capabilities appear to be poor. Leakage is expected through both abutments and the foundation. Pervious borrow material for dam construction was located near the site.

Engineering Notes:

The right abutment is recommended for the excavated emergency spillway location.

Location:

On the Mattapoisett River about 1000 feet downstream from Tinkham Lane in Mattapoisett, Mass.

Marion, Mass. USGS quadrangle

Latitude: 41°40'56" Longitude: 70°50'28"

Facilities	Facility	Elevation
Affected:	House	25
	2 garages	25
	Shed	25
	Tinkham Lane	25
	Pumphouse and well	20

Geologic Conditions: Both abutments are fine, poorly graded, outwash sand and gravel. Depth to granite bedrock in the foundation is estimated to be from 40 to 50 feet. Waterholding capabilities appear to be poor. Leakage is expected through both abutments and the foundation. Pervious borrow material for dam construction was located near the site; impervious material was not located.

Engineering Notes:

Preliminary designs indicate that a concrete drop structure emergency spillway will probably be required at this site. The large drainage area may permit maintainance of a pool where inflow will be sufficient to overcome expected seepage losses.

POTENTIAL SITE BB-4403

Location:

On the Mattapoisett River about 100 feet upstream from Rounseville Road in Rochester, Mass.

Marion, Mass. USGS quadrangle

Latitude: 41°44'10" Longitude: 70°51'47"

Facilities	Facility	Elevation
Affected:	16 houses	60
	Garage	60
	3 sheds	60
	17 houses	55
	2 buildings	55
	15 garages	55
	7 sheds	55
	Hartley Road	55
	Snipatuit Road	55 50 50
	6 houses	50
	Commercial building	50
	7 garages	50
	3 sheds	50
	Highway garage	50

POTENTIAL SITE BB-4403 (continued)

Geologic Conditions: Both abutments are outwash sand and gravel with some large boulders. Depth to bedrock in the foundation is estimated to be from 30 to 40 feet. Waterholding capabilities appear to be poor. Leakage is expected through both abutments and the foundation. Pervious borrow material for dam construction was located near the site; impervious material was not located.

Engineering Notes:

Preliminary designs indicate that a concrete drop structure emergency spillway will probably be required at this site. The large drainage area may permit maintenance of a pool where inflow will be sufficient to overcome expected seepage losses.

SUMMARY CATA FOR POTENTIAL UPSTREAM RESERVOIR SITES

	SAF	* PERCENT *CHANCE * (MGD)	70-51-27 710 CFS	* * * * * * * * * * * * * * * * * * *	70-50-28 211 CFS	0.21 0.93 1.55 1.81	70-51-47 958 CFS	* 0 0 2 1	NONE 0 SES.
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	DAM	HGT FT	****** 54 LONGI PEAK FLOW	118 118 118	**** 6 LO EAK F	15 16 16 16	* A	20 20 20 20 20	**************************************
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1	je :	********* STORAGE AT CREST AC FT I	**************************************	735 882 866 963	QUAD-MARION, MASS PRIN SPWY DESIGN	251 624 997 1140	-MAR SPW	95 195 602 1008 2227 2366	C C C C C C C C C C C C C C C C C C C
3 3 3 3 3 3	EMERGENCY	CREST CREST ELEV TYPE (MSL) A	* D. R.	34.5 E	******** USGS QUA 10-YR PRI	18.5 C 20.7 D 22.1 D 22.5 D	.*************************************		I TE BA CH CH FCO FCO
3	*	*	* "		***		* -	* * * * * * *	DESIGN CR COSTS ARE CONCRETE MINARY IN REST 0.1
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AREA-BUZZARUS		****** COST/ SURF AC AC (\$)	**************************************	1279C 976C 937C	**************************************	2323C 1299C 966C 856C	**************************************	2487C 2499C 2249C 673C 651C	BASED ON 1973 S.C.S. DESIGN CR SPILLWAY STORAGE AND COSTS ARE SPILLWAY TYPE CUDE— C=CUNCRETE TA ARE BASED ON PRELIMINARY IN ARE SHUWN TO THE NEAREST 0.1 ACCURATE TO THAT DEGREE.
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	* * * * * *	ELEV **	******** SITE-BB-4401 SITE RATIN	28.5 30.0 32.0	**************************************	18.5 20.7 22.1 22.5	**************************************	444.7 444.5 49.8 522.3	NOTES

EXISTING SITE BB-4401 (Tinkham Pond)

Location: On an unnamed tributary to Mattapoisett River at Tinkham Lane in Mattapoisett, Mass. Most of the pond is in Acushnet, Mass.

Marion, Mass. USGS quadrangle.

Latitude: 41°40'54" Longitude: 70°51'27"

Surface Area Height of Drainage Area
(Acres) Dam (Ft.) (Acres) (Sq. Miles)
7 9 2,150 3.36

Potential for

Please refer to Site Data and Design Summary Table for Potential Site BB-4401.

Expansion:

Remarks:

Tinkham Lane forms the dam. There are two spillways. The principal spillway is a double-bay concrete flume with flashboards. The other spillway is a rectangular three-bay concrete drop structure which outlets to a stone box culvert. Trees and brush are growing on the dam. Concrete in the spillways has spalled.

Ownership and

The pond is owned by Decas Bros. and is used for cranberry bog irrigation and recreation.

EXISTING SITE BB-4403 (Harley Mill Pond)

Location:

On the Mattapoisset River at Rounseville Road in Rochester, Mass.

Marion, Mass. USGS quadrangle

Latitude:41°44'10" Longitude:70°51'47"

Surface Area Height of Drainage Area

(Acres) Dam (Ft.) (Acres) (Sq. Miles)

7,600 11.88

originally 78

Potential for

Please refer to Site Data and Design Summary Table for

Potential Site BB-4403.

Expansion:

Remarks:

The dam has been breached. A mill building located near the

dam has burned down.

Ownership and

The pond area is owned by Rounseville Sawmill and is no longer in use.

Use:

EXISTING SITE BB-4404

Location:

On an unnamed tributary to the East Branch of the Sippican River about 2,300 feet south of the Middleborough-Rochester town line in Rochester, Mass.

Snipatuit Pond, Mass. USGS quadrangle

Latitude: 41^o47'27" Longitude: 70^o47'54"

Surface Area Height of Drainage Area
(Acres) Dam (Ft.) (Acres) (Sq. Miles)
7 6 1,400 2.19

Potential for Expansion: The pond could be expanded into Forbes Swamp. A body of shallow water would be created by expansion.

Remarks:

The dam is an earthfill structure. There are three outlet structures. One is a double bay concrete flume. The next structure is a single bay concrete flume. Near the center of the dam is a concrete drop structure. Concrete in all structures is spalled to some extent.

Ownership and Use:

The pond is owned by Slocum Gibbs and is used for cranberry bog irrigation.

EXISTING SITE BB-4405

Location:

On the East Branch of the Sippican River about 4,000 feet west of County Road in Rochester, Mass.

Snipatuit Pond, Mass. USGS quadrangle

Latitude: 41°47'43"

Longitude: 70°47'14"

Surface Area Height of Drainage Area
(Acres) Dam (Ft.) (Acres) (Sq. Miles)
26 12 2,750 4.30

Potential for Expansion: Expansion would affect several upstream cranberry bogs. A body of shallow water would be created by expansion.

Remarks:

The dam is an earthfill structure. The spillway is a twobay concrete flume with flashboards. There is also a timber riser which controls flow to a canal.

Ownership and

The pond is owned by A. D. Makepeace Co. and is used for cranberry bog irrigation.

Use:

EXISTING SITE BB-4406

On the East Branch of the Sippican River about 900 feet up-Location: stream from Pierceville Road in Rochester, Mass.

Snipatuit Pond, Mass. USGS quadrangle

Latitude: 41^o46'12"

Longitude: 70°47'00"

Surface Area (Acres)

Height of Dam (Ft.)
12

Drainage Area (Acres) 4,050

(Sq. Miles)

Potential for Expansion:

Sandy soils may limit expansion potential. Expansion would affect upstream cranberry bogs.

Remarks:

The dam is an earthfill structure. There are three outlets. One is a three-bay concrete flume. The other is a timber flume. . The third structure is a concrete drop inlet and steel pipe conduit. All outlets have provison for flashboards. There is another reservoir located immediately upstream. Water level is about 0.5 feet higher.

Owner ship and

The pond is owned by A. D. Makepeace Company and is used for cranberry bog irrigation.

Use:

EXISTING SITE BB-4407 (Leonards Pond)

Location:

On the Sippican River at Marys Pond Road in Rochester. Mass.

Marion and Snipatuit Pond, Mass. USGS quadrangles

Latitude: 41°44'53" Longitude: 70°48'18"

Surface Area (Acres) Height of Dam(Ft.)

Drainage Area (Acres) 9,800

Potential for Expansion: Expansion would create a large body of shallow water. Sandy soils may limit expansion potential.

Remarks:

The dam is an earthfill structure faced with stone and stone masonry. There are three spillway structures in the dam. The left structure is a stone masonry two-bay drop structure with flashboards. The center structure, outletting under a mill building, is a concrete open flume with flashboards. The right structure is a stone drop structure with five bays of sloping timber flashboards. There are also two concrete flumes located on the east side of the reservoir which deliver water to cranberry bogs. Several areas behind the stone masonry on the dam have washed away.

Existing Site BB-4407 (continued)

Ownership and

Use:

Ownership of the pond was not determined. The pond is controlled by Howard Hiller and is used for cranberry bog irrigation and recreation.

Existing Site BB-4408(Hathaway Pond)

Location:

On the Sippican River about 700 feet upstream from the Marion-Rochester town line in Rochester, Mass.

Marion, Mass. USGS quadrangle

Latitude: 41°43'59" Longitude: 70°47'43"

Surface Area Height of Drainage Area (Acres) Dam(Ft.) (Acres) Sq. Mi. 15.86

Potential for Expansion:

Raising the pond level by 8 feet would create a pool area of about 90 acres.

Remarks:

The dam is an earthfill structure. There are two spill-ways. One is a stone masonry sidewalled structure with two bays of flashboards. The other is a five-bay covered concrete flume with flashboards. There is also a concrete fish ladder. Trees and bruch are growing on the dam. The upstream slope is nearly vertical in places, but appears stable. Wave erosion has occurred near the concrete spillway. Concrete in the spillway has spalled.

Ownership and Use:

Ownership of the pond was not determined. The pond is controlled by Hiller Brothers and is used for cranberry bog irrigation.

Existing Site BB-4409

Location:

On an unnamed tributary to the West Branch of the Sippican River about 3000 feet south of the Middleborough-Rochester town line in Rochester, Mass.

Snipatuit Pond, Mass. USGS quadrangle

Latitude: 41°47'48" Longitude: 70°49'02"

Surface Area Height of Drainage Area (Acres) Dam(Ft.) (Acres) Sq. Mi.

Potential for Expansion: The pond could be expanded into Forbes Swamp. A much longer dam would be required. A body of shallow water would be created by expansion.

Remarks:

The dam is an earthfill structure. There is also a 4-foot high earthfill dike. The spillway is a timber flume with two bays of flashboards. Trees and shrubs are growing on the dam.

Ownership and Use:

Ownership of the pond was not determined. The pond is used to store water for use in cranberry bogs.

Existing Site BB-4410(Mill Pond)

Location:

On the West Branch of the Sippican River about 300 feet upstream from Walnut Plain Road in Rochester, Mass.

Snipatuit Pond, Mass. USGS quadrangle

Latitude: 41°45'36" Longitude: 70°49'30"

Surface Area Height of Drainage Area (Acres) Dam(Ft.) (Acres) Sq. Mi.

Potential for Expansion: Flat topography limits expansion potential.

Remarks:

The dam is an earthfill structure. A low stone wall runs along the toe of the downstream slope. The spillway system consists of a two-bay and a single-bay concrete flume with flashboards. There appears to be seepage near the center of the dam which surfaces about 15 feet downstream of the downstream toe.

Ownership and Use:

The pond is owned by Decas Bros. and is used for cranberry bog irrigation.

Existing Site BB-4411(Snipatuit Pond)

Location:

On the Mattapoisett River about 4600 feet upstream from Snipatuit Road in Rochester, Mass.

Snipatuit Pond, Mass. USGS quadrangle

Latitude: 41°45'45" Longitude: 70°52'09"

Surface Area (Acres)

Height of Dam(Ft.)

Drainage Area

Potential for Expansion: The relatively small drainage area limits expansion potential. A large body of shallow water would be created if the pond were expanded into Cedar Swamp.

Remarks:

The dam is a short earthfill structure. The spillway is an open concrete flume adjacent to a fish ladder. The flume has provision for flashboards. Some of the earthfill near the spillway has been washed away.

Ownership and Use:

The pond is owned by the Massachusetts Department of Fish and Game and is used for recreation and cranberry bog irrigation.



BB - 6401 Tinkham Pond



8B - 4403 Harley Mill Pond



BB - 4407 Leonards Pond



BB - 4405





BB - hh08 Hathaway Pond



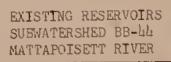
BB - 4409



BB - hhlo Mill Fond

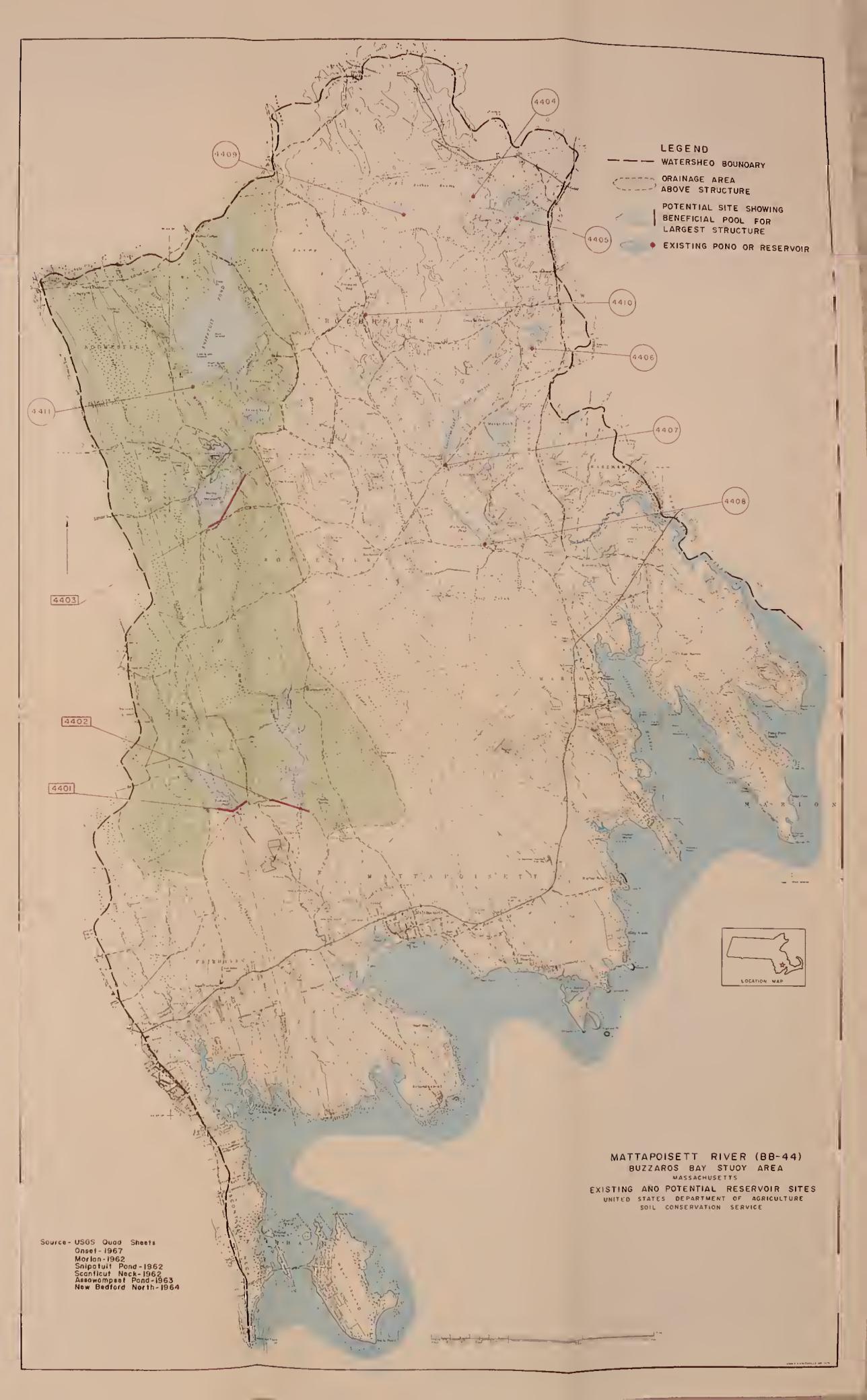


BB - 4411 Snipatuit Pond









BUZZARDS BAY STUDY AREA STIE DATA FOR

Subwatershed BB-45, Acushnet River

This subwatershed covers about 55,800 acres in Acushnet, Dartmouth, Fairhaven, Freetown, New Bedford, and Westport, all in Bristol County; and Rochester in Plymouth County.

Major streams in the subwatershed include the Acushnet and Paskamanset Rivers. The Acushnet originates in the headwaters of New Bedford Reservoir in Acushnet and flows south to New Bedford Harbor and the Atlantic Ocean. The Paskamanset River originates in Dartmouth and flows south to Russells Mills where it becomes the Slocums River which flows to the Atlantic Ocean. Elevations range from a high of 220 feet in Dartmouth to sea level in Buzzards Bay.

Geology of the subwatershed is characterized by granite bedrock overlain by 10 to 15 feet of glacial till.

Three potential reservoir sites and one existing reservoir were studied.

POTENTIAL SITE BB-4501

Location:

On an unnamed tributary to Allens Pond (Dartmouth) about 2200

feet downstream from Cross St. in Westport, Mass.

Westport, Mass.-R.I. USGS quadrangle

Latitude: 41°31'46" Longitude: 71°01'50"

Facilities Affected:

Facility Cross Street

Elevation 50

Geologic Conditions:

Both abutments are glacial till underlain by bedrock. Depth to bedrock in the foundation is estimated to be from 10 to 15 feet. Waterholding capabilities appear to be good. Borrow material for dam construction was located near the site.

Engineering Notes:

Preliminary designs indicate that a monolithic conduit emergency spillway will probably be required at this site.

Location:

On an unnamed tributary to Allens Pond about 2600 feet downstream from Horseneck Road in Dartmouth, Mass.

Westport, Mass.-R.I. USGS quadrangle

Latitude: 41°31'22" Longitude: 71°01'31"

Facilities Affected:

Facility	Elevation
4 sheds	40
Horseneck Road	40
3 sheds	35

Geologic Conditions:

Both abutments are glacial till underlain by granite bedrock. Depth to bedrock in the foundation is estimated to be from 10 to 15 feet. Waterholding capabilities appear to be good. Borrow mâterial for dam construction was located near the site.

Engineering Notes:

The right abutment is recommended for the excavated emergency spillway location.

POTENTIAL SITE BB-4504

Location:

On Destruction Brook about 1900 feet upstream from Slades Corner Road in Dartmouth, Mass.

Westport, Mass.-R.I. USGS quadrangle

Latitude: 41°34'34" Longitude: 71°01'05"

Facilities Affected:

None below elevation 80

Geologic Conditions:

Both abutments are glacial till, underlain by granite bedrock. Depth to bedrock in the foundation is estimated to be very shallow. Waterholding capabilities appear to be good. Borrow material for dam construction was located near the site.

Engineering Notes:

The right abutment is recommended for the excavated emergency spillway location.

Location:

On an unnamed tributary to the Acushnet River about 1000 feet upstream from Middle Road in Acushnet, Mass.

New Bedford North, Mass. USGS quadrangle

Latitude: 41°42'54" Longitude: 70°54'45"

Facilities	Facility	Elevation
Affected:	4 houses	65
	2 houses	60
	Morses Lane	55

Geologic Conditions: Both abutments are englacial drift; shallow to gneiss bedrock. There are many large surface boulders. Depth to bedrock in the foundation is estimated to be from 15 to 20 feet. Waterholding capabilities appear to be fair to poor. Leakage is expected through both abutments and the foundation. Borrow material for dam construction was located near the site.

Engineering Notes:

Preliminary designs indicate that a monolithic conduit emergency spillway will probably be required at this site.

SUMMARY CATA FOR POTENTIAL UPSTREAM RESERVOIR SITES

SAFE YIELD	• PERCENT • CHANCE • (MGC)	71-01-50 157 CFS	0.14 0.17 0.20 0.23	71-01-31 258 CFS 0.15 0.26 0.33	LATITUDE 41-34-34 LONGITUDE 71-01-05 JES 5.00 IN, PEAK FLOW = 623 CFS 9 153 79.9 18 20 ***** 1 157 80.1 18 22 ***** 1 157 80.1 18 19 0.42 4 161 80.4 18 26 0.50	
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A . Y	COST PER AC FT (\$)	USGS QUAD-WESTPURT, WASS 100-YR PRIN SPWY DESIGN STORM 39.8 E 122 4.1 2790 •	3760 3000 2540 2230	USGS GUAD-WESTORT, MASS 00-YK PRIN SPWY DESIGN STORM 29-5 E 199 4-1 1880 6 29-5 E 195 4-1 2220 6 34-2 E 445 9-3 1300 6 35-8 E 561 11.7 1130 6	JSGS QUAD-WESTPORT, MASS D-YR PRIN SPHY DESIGN STORM 74.5 E 558 4.1 1120 ° 74.5 E 604 4.5 1140 ° 74.8 E 591 4.4 1290 ° 71.6 T 287 2.0 2860 ° 71.5 T 356 2.7 2310 °	DO N. 1973 S.C.S. DESIGN CRIFFEL AND COST DATA. NAY STORAGE AND COSTS ARE BASED ON TOTAL STORAGE, INCLUDIN. LHAY STORAGE AND COSTS ARE BASED ON TOTAL STORAGE, INCLUDIN. LHAY STORAGE AND COSTS ARE BASED ON TOTAL STORAGE, INCLUDIN. RE BASED ON PRELIMINARY INFORMATION. FIGURES SHOWN ARE PRIM. SHOWN TO THE NEAREST 0.1 FCOT TO SHOW VARIATION BETWEEN DE. RAD ON THE SECREE.
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R G •	₹	PRI F	www	PR1 -	PRI PRI T	RIA SED UTE HAT
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AEA-BU	1	5 SG M WATER 4	25 24 32 36	** A TER ***	2 SQ M WATER 15 37 51 71	AY STO AY TYP BASED HOWN T
SIULY AREA-BUZZARLS BENEFICIAL POOL	CUST PER AC FT (\$)	CA= 0.55 SG MI = 352 AC SIRCAM WATER QUALITY (8)	3930 3100 2610 2280	1) CA= 0.90 SG MI = 576 AC 2.0 4340 30 1442C 7.0 4.1 2650 42 116.0 6.0 2000 52 116.0 8.0 1650 61 1032C 13.2	DA = 2.52 SQ MI = 1613 AC STREAM WATER QUALITY (B) 0.0 15 2.5 0.7 6900 37 18520 6.4 1.4 4170 51 15560 8.3 2.0 3080 71 11550 9.6 2.5 2450 85 9720 10.5	(1) COSTS ARE BASED ON 1973 S.C.S. DESIGN CRITERIA AND COST DATA. (1) COSTS ARE BASED ON 1973 S.C.S. DESIGN CRITERIA AND COST DATA. (2) EMERGENCY SPILLMAY STORAGE AND COSTS ARE BASED ON TOTAL STORAGE, INCLUDING BENEFICIAL POOL. (3) EMERGENCY SPILLMAY TYPE COTE— C—CONCRETE CHUIE, D—CCNCRETE DROP, E=EXCAVATED, T= TWO SPILLMAYS, N= NONE (4) TABULAR DATA ARE BASED ON PRELIMINARY INFORMATION. FIGURES SHOWN ARE PRIMARILY FOR COMPARISON PURPOSES. (5) ELEVATIONS ARE SHOWN TO THE NEAREST O.1 FOOT TO SHOW VARIATION BETWEEN DEVELOPMENTS ONLY, AND ARE NOT TO CONSIDERED ACCURATE TO THAT EGREE OF STATE OF THE PROPERTY OF T
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BENER	STORAGE AC FT I	• — 5 0	100 135 170 205	* 10 00004	•	(1) CO: (2) EM: (3) EM: (4) TA: (5) EL:
	ELEV (MSL)	SITE-88-45C1 SITE RATING 32.2 0	39.0 40.4 41.5 42.5	SITE-BB-4503 SITE RATING 21.5 0 0 0 29.5 195 31.7 289	SITE-88-4504 SITE RATING 64.5 0 70.3 71.6 72.5 338	0 1 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0

SUMMARY CATA FOR POTENTIAL UPSTREAM RESERVOIR SITES

	* SAFE * YIELD	FILL *PERCENT VOL *CHANCE (1000 * CY) * (MGD)	######################################		* L9		* 99	* 88	•	**************************************
	DAM		**************************************		68.0 22					CIAL POOL. TWO SPILLW
IVER		# TOP AREA # ELEV (AC) * (MSL	***** UDE 4 5.00	2 *	2 *	* 1	* 0	* 2	*	# # # # # # # # # # # # # # # # # # #
SUBWATERSHED ACUSHNET RIVER	DESIGN HIGH WATER	* TOP ELEV AREA * ELEV (MSL) (AC) * (MSL)	LATIT RUNOFF =		64.9 132					CLUDING B EXCAVATED
ATERSHED A		COST * PER * AC FT *	RE NORTH GN STORM	2010 *	2140 *	2440 *	2350 *	1800 *	*	######################################
SUBMA	EMERGENCY SPILLWAY	STORAGE AT CREST AC FT IN	**************************************		E 550 4.1					**************************************
	EMER(CREST + ELEV + TYPE + (MSL)	USGS Q1	4 62.5	4 62.5	* 61.8 E	* 62.4	. 65.0		SA CRITERI SA CRITERI SARE BASE CRETE CHUT
S BAY		* * I	1594 AC ITY (B)	3.0						**************************************
BUZZARC		COST/ SURF AC (\$)	***** MI = ER QUAL		3994C	28640	24840	16190		****** 973 S.C TORAGE YPE COD ED ON P
STUCY AREA-BUZZARCS BAY	BENEFICIAL POOL	ELEV STORAGE PER AREA SURF AT AC FT AC FT AC FT) (\$1) (\$1) (\$1)	**************************************	12	0 29	0 42	51	0 92		**************************************
STUD	BENEFICIAL POOL	COST PER AC FT (\$)	##### EA= STRE		11770		3780	2820		RE BASI CY SPII CY SPII DATA
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1		ELEV STOF	**************************************	J	100	218	336			* (1) (2) (4) (4) (4) (4) (7) (6) (7) (7) (7) (7) (7) (7) (7) (7) (7) (7
1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1		ELEV (MSL)	SITE-BB-4505 SITE RATING	49.0	54.0	57.3	59.9	62.5		NOTES.

EMERGENCY SPILLWAY TYPE CODE— C=CONCRETE CHUTE, D=CONCRETE DROP, E=EXCAVATED, T= TWO SPILLWAYS, N= NONE TABULAR DATA ARE BASED ON PRELIMINARY INFORMATION. FIGURES SHOWN ARE PRIMARILY FOR COMPARISON PURPOSES. ELEVATIONS ARE SHOWN TO THE NEAREST 0.1 FCOT TO SHOW VARIATION BETWEEN DEVELOPMENTS ONLY, AND ARE NOT TO CONSIDERED ACCURATE TO THAT DEGREE. ** DO NOT USE FOR FINAL SITE SELECTION OR LAND ACQUISITION. **

Existing Site BB-45C6 (Buttonwood Park Dam)

Location:

On Buttonwood Brook in Buttonwood Park, New Bedford, Massachusetts.

New Bedford North, Mass. USGS quadrangle

Latitude: 41°37'55" Longitude: 70°57'15"

Surface Area Height of Drainage Area

(Acres) Dam(Ft.) (Acres) Sq. Mi.

8 475 0.75

Potential for Expansion:

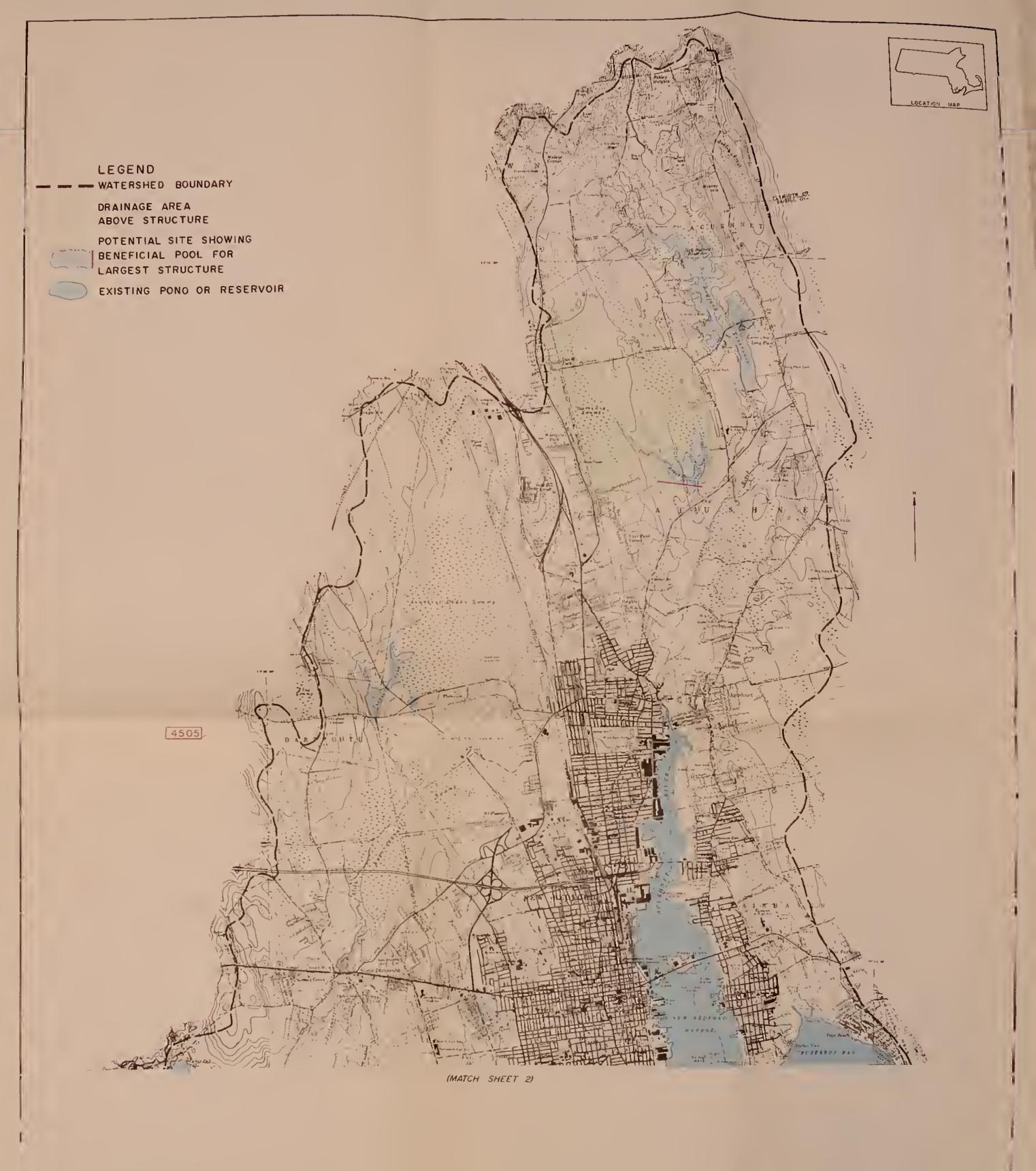
Limited. The pond is surrounded by Buttonwood Park and residential areas.

Remarks:

The dam is an earthfill structure with a road across the top. The spillway is a concrete and granite block box inlet attached to a semi-circular brick lined culvert. Water level in the pond can also be controlled with a gate valve.

Ownership and Use:

The pond is owned by the city of New Bedford and is used for recreation.



SOURCE-USGS OUAD. SHEETS
ASSAWOMPSET POND-1963
NEW BEDFORD NORTH-1964



ACUSHNET RIVER (BB-45)
BUZZARDS BAY STUDY AREA
MASSACHUSETTS

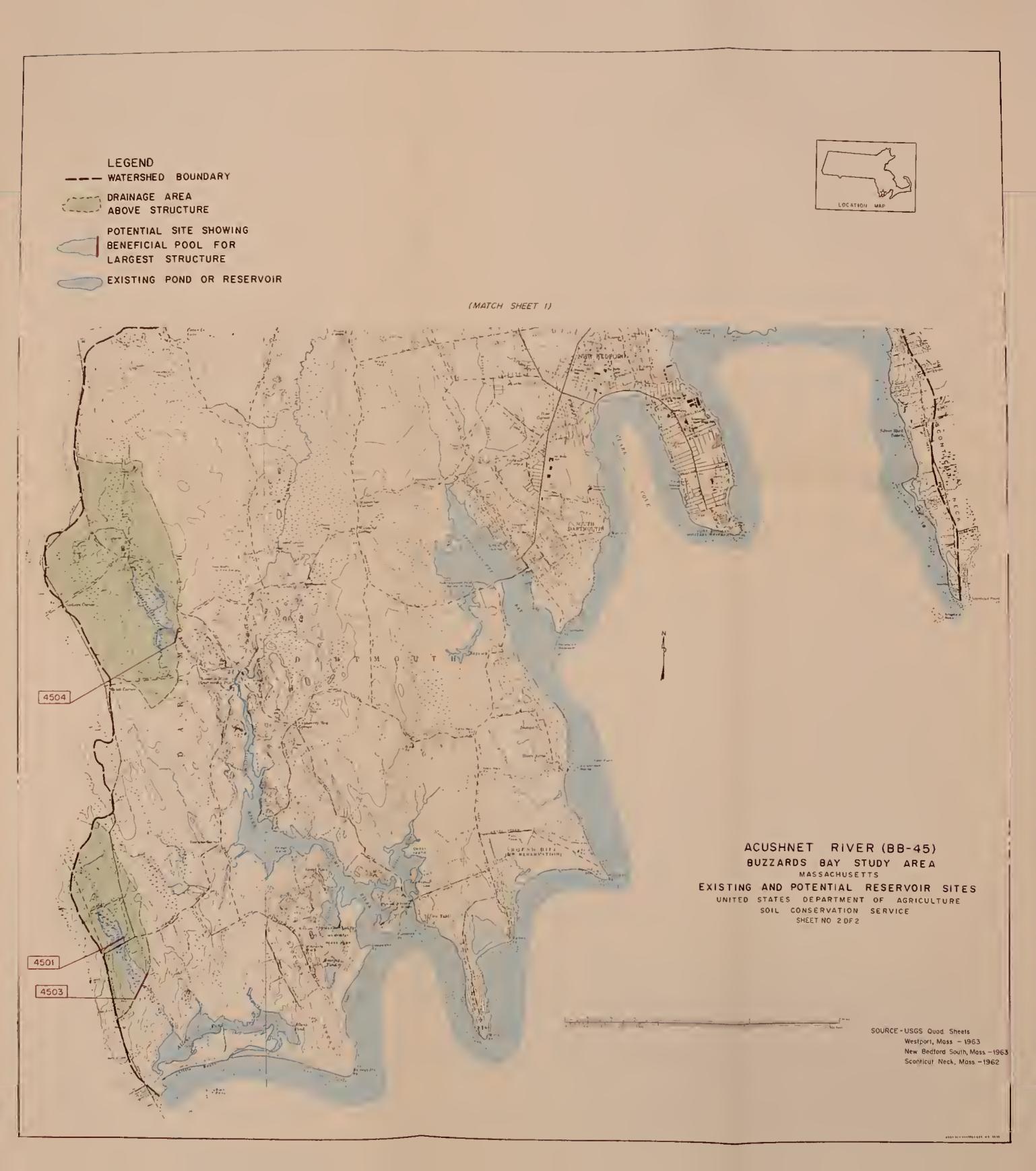
EXISTING AND POTENTIAL RESERVOIR SITES

UNITED STATES DEPARTMENT OF AGRICULTURE

SOIL CONSERVATION SERVICE

SHEET NO. 1 OF 2







BUZZARDS BAY STUDY AREA SITE DATA FOR

Subwatershed BB-46, Westport River

This subwatershed covers about 47,000 acres in Dartmouth, Fall River, Freetown, and Westport, all in Bristol County.

Major streams in the subwatershed include the East and West Branches of the Westport River and their tributaries. The West Branch originates in Rhode Island as Adamsville Brook and flows southeasterly through Westport to the Atlantic Ocean. The East Branch originates in Dartmouth as Shingle Island River which flows south into Noquochoke Lake. It emerges as the East Branch which continues flowing south through Westport to the ocean. Elevations range from a high of 384 feet on Copicut Hill in Dartmouth to sea level in Westport.

Geology of the subwatershed is characterized by granite bedrock overlain by 10 to 40 feet of glacial till.

Six potential reservoir sites and one existing reservoir were studied.

POTENTIAL SITE BB-4601

Location:

On an unnamed tributary to Bread and Cheese Brook about 600 feet upstream from Gifford Road in Westport, Mass.

Fall River East, Mass. USGS quadrangle

Latitude: 41°38'30" Longitude: 71°04'46"

Facilities	Facility	Elevation
Affected:	5 houses	140
	5 sheds	140
	Gifford Street	140
	5 houses	135
	3 Commercial buildings	135
	2 garages	135
	6 sheds	135
	3 houses	130
	Garage	130
	Shed	130
	Route 88	125
	House	120
	3 garages	120
	3 sheds	120

Geologic Conditions: Both abutments are poorly graded sand and gravel and silty sand. Depth to bedrock in the foundation is estimated to be from 20 to 25 feet. Waterholding capabilities appear to be good. Borrow material for dam construction was located near the site.

Engineering Notes:

Preliminary designs indicate that a concrete chute emergency spillway will probably be required at this site.

Location:

On Bread and Cheese Brook about 1400 feet upstream from American Legion Highway, Route 177, in Westport, Mass.

Fall River East, Mass. USGS quadrangle

Latitude: 41°38'12" Longitude: 71°04'00"

Facilities Affected:

Facility 2 houses

Elevation

Geologic Conditions: Both abutments are outwash sand and gravel with large boulders, underlain by gneiss bedrock. Depth to bedrock in the foundation is estimated to be from 15 to 20 feet. Waterholding capabilities appear to be poor. Leakage is expected through both abutments and the foundation. Pervious borrow material for dam construction was located near the site; impervious material was not located.

Engineering Notes:

Preliminary designs indicate that a concrete chute emergency spillway will probably be required at this site.

POTENTIAL SITE BB-4603

Location:

On Bread and Cheese Brook about 350 feet upstream from Interstate Route 195 in Westport, Mass.

Fall River East, Mass. USGS quadrangle

Longitude: 71°04'51" Latitude: 41°40'12"

Facilities Affected:

Facili ⁻	ty	Elevation
House		150
3 garages		150
3 houses		145
Garage		145
2 sheds		145
4 houses		140
3 garages		140
5 sheds		140
Old Bedford	Road	140
House		135
4 garages		135
Shed		135

Geologic Conditions: Both abutments are silty sand with gravel, cobbles and boulders. Depth to bedrock in the foundation is estimated to be from 30 to 40 feet. Waterholding capabilities appear to be fair. Slight leakage is expected through both abutments and the foundation. Borrow material for dam construction was located near the site.

Engineering Notes:

Preliminary designs indicate that a monolithic conduit emergency spillway will probably be required at this site.

Location:

On Angeline Brook about 400 feet upstream from Cornell Road in Westport, Mass.

Westport, Mass.-R.I. USGS quadrangle

Latitude: 41°33'05"

Longitude: 70⁰06'22"

Facilities Affected:

None below elevation 80

Geologic Conditions:

Both abutments are glacial till. Depth to bedrock in the founda-

tion is estimated to be from 10 to 15 feet. Waterholding

capabilities appear to be good. Borrow material for dam construc-

tion was located near the site.

Engineering Notes:

Preliminary designs indicate that a concrete chute emergency spillway will probably be required at this site.

POTENTIAL SITE BB-4605

Location:

On Angeline Brook about 200 feet upstream from Adamsville Road in Westport, Mass.

Westport, Mass.-R.I. USGS quadrangle

Latitude: 41°34'25"

Longitude: 70°05'149"

Facilities Affected:

Facility Garage

Elevation 160

Geologic Conditions: Both abutments are glacial till. Depth to bedrock in the foundation is estimated to be from 10 to 15 feet. Waterholding

capabilities appear to be good. Borrow material for dam construc-

tion was located near the site.

Engineering Notes:

Preliminary designs indicate that a concrete drop structure emergency spillway will probably be required at this site.

POTENTIAL SITE BB-4606

Location:

On an unnamed tributary to Snell Creek about 300 feet upstream from Main Road in Westport, Mass.

Westport, Mass.-R.I. USGS quadrangle

Latitude: 41°35'08"

Longitude: 70°05'29"

Facility	Elevation
2 houses	135
2 garages	130
4 houses	130
Industrial building	130
	2 houses 2 garages 4 houses

Geologic Conditions:

Both abutments are glacial till. Depth to bedrock in the foundation is estimated to be from 15 to 20 feet. Waterholding capabilities appear to be good. Borrow material for dam construction was located near the site.

Engineering Notes:

Preliminary designs indicate that a monolithic conduit emergency spillway will probably be required at this site.

SUMMARY DATA FOR POTENTIAL UPSTREAM RESERVOIR SITES

	*		STUDY AREA-B-B-B-B-B-B-B-B-B-B-B-B-B-B-B-B-B-B-B	3EA-81	JZZARDS	BAY	*	*	*	SUBMA	SUBWATERSHED	WESTPORT RIVER	RT RIVER	ER.		*	•	•
		BENEFI	BENEFICIAL POOL				* *	EMERGENCY		SPILLWAY	* *		GNATER	• •	DAM		* SAFE * YIELD	. E
ELEV (MSL) A	**************************************	# # # # # # # # # # # # # # # # # # #	**************************************	* * * * * * * * * * * * * * * * * * *	COST/ SURF AC (\$)	DEPTH DEPTH AT DAM (FT)	* CREST * CREST * ELEV *+ TYPE * (MSL)	* V * *	*0	2	######################################	# # EV	* * * * * * * * * * * * * * * * * * *	* TOP * ELEV * (MSL	* I L	FILL VOL (1000 (7)	** AT 95 *PERCENT *CHANCE *	ERCENT HANCE (MGD)
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106.1 111.8 119.6 128.1 132.5	100 407 1022 1558	0.0 0.6 2.5 10.1	10460 3330 1860 1500	10 25 56 88 148	41350 24450 21480 15820	4-1 9-8 17-6 26-0 30-5	* 122 * 111 * 119 * 128 * 132	88946	640 123 430 1045 1581	4.1 2.8 6.8 10.3	1920 * 8500 * 3150 * 1480 * 1480 *	800.40		136.3 136.2 138.7 138.6 139.8	3 34 2 34 1 37 6 37 8 38	108 105 136 135 150		**** 0.18 0.59 1.14
SITE-BB-4602 SITE RATING (3)	-4602 RATING	(3)	DA= 9.29 SQ MI = 5946 AC STREAM WATER QUALITY (B)	9.29 SQ MI EAM WATER (II = 5	= 5946 AC	USGS 100-YR	*	QUAD-FALL PRIN SPWY	RIVER DESIGN	***** EAST N STORM	RUNDF	# et 11	E 41-3	****** 8-12 L • PEAK	**************************************	* - 5	****** 11-04-00 115 CFS
500.7 500.7 500.7 500.5 500.5 500.5	0 100 414 1041 1835	0000	15450 4930 2330 1520	15 25 56 108 150	62700 36660 22570 18550	9.6 22.5 30.4 36.5	* * * * * * * * * * * * * * * * * * * *	73.4 T 50.7 T 58.5 T 66.4 T 72.5 T	2056 174 488 1115 1909	4.00 2.30 9.93	1160 * * * 4180 * 1146	77.6 63.5 71.4 75.3	183 88 143 168 132	666 666 80 80 80	37 37 37 47 47	316 92 1194 246 307	* * * * * *	*** .21 .75 .60
SITE-88-4603 SITE RATIN	TE-88-4603 SITE RATING	(2)	**************************************	**************************************	"I = 2	2221 AC	USG:	# W W	QUAD-FALL PRIN SPWY	RIVER E	EAST N STORM	RUNOF	**************************************	400	* * * 2 E A K	LONGITUDE FLOW =	* ~ 10	1-04-51 72 CFS
113.6 118.1 126.6 134.8 142.1	1 2 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7	0.0 0.5 3.0 7.8 15.0	18800 3930 1950 1200	14 30 78 148 226	6213C 2751C 1894C 1475C	3.6 8.2 16.7 24.9 32.0	* 126 * 126 * 136 * 145	129.0 C 118.1 N 126.6 E 134.8 E 142.1 E	768 128 574 1466 2804	4.1 0.7 3.0 7.8 15.2	2130 * 14720 * 3740 * 1910 * 1190 *	135.0 136.3 136.0 141.8	150 163 160 225 297	* * 141.8 146.3 145.1 149.3 * 153.8	8 32 3 36 1 35 3 39 44	293 398 366 482 620	* * * * * *	. 18 . 77 . 54
# + C X U F F F F F F F F F F F F F F F F F F	* 12653 * 12653	**************************************	**************************************	197 197 197 197 197 197 197 197 197 197	****** 973 S.C. 170RAGE A YPE CODE ED ON PRE 170 THE 10 THAT	S. DESI ND COST - C=CON ELIMINAL NEAREST CEGREE.	★ ⋖ Ш •	CRITERIA ARE BASED ETE CHUTE, INFCRMATI 1 FCOT TC	AN ON CON	**************************************	TA. DRAGE, DROP, SHOWN FION BE	GE, INCLUDING BENEFICIAL OP, E=EXCAVATED, T= TWO OWN ARE PRIMARILY FOR CO N BETWEEN DEVELOPMENTS O ACQUISITION. **	****** ATED, MARILY EVELOP	**************************************	POOL SPILL MPARI	* * * * * * * * * * * * * * * * * * *	NONE NONE NOT T	* # B # * 0

SUMMARY DATA FOR POTENTIAL UPSTREAM RESERVOIR SITES

	E	95 CENT (CE	6-22 6-22 CFS 118 40 71	CFS # 18 # 53	5-29 CFS CFS 113 116 120	# 89 # U
	SAFE YIELD	* AT 95 *PERCENT *CHANCE * (MGD)	70-06-2 623 CFS 623 CFS 0-18 0-71 1-40	70-05-4 392 CFS ** ** 0.18 0.29 0.39	70-05-29 140 CFS 140 CFS 0.13 0.16 0.16	NONE SES.
1		FILL ***********************************	######################################		LATITUDE 41-35-08 LONGITUDE RUNDFF = 5.00 IN, PEAK FLOW = 134.0 32 * 140.7 21 71 * 135.2 37 * 141.2 21 75 * 136.2 40 * 142.0 22 82 * 137.0 43 * 142.6 23 88 * 137.8 45 * 143.1 23 94 *	S, N= PURPO ARE N
1	DAM	HGT FT	**************************************	25 LON PEAK FL 20 19 19 20	21 22 23 23	L POOL. SPILLWAYS OMPARISON ONLY, AND
~		TOP ELEV (MSL)	# # # # # # # # # # # # # # # # # # #	E 41-34-25 LONGI ** 159.3 20 ** 156.6 18 ** 158.2 19 ** 158.2 19 ** 159.3 20	**************************************	BENEFICIAL P D, T = TWO SP ILY FOR COMP LOPMENTS ONL
T RIVER	DESIGN +	AREA *	######################################	LATITUDE 1 99 * 3 76 * 2 92 * 8 97 * 3 110 * 4 4 4 4 4 4 4 4 4	ATITUDE F = 5.0 32 * 40 40 * 45 * 45	NG BENEFICI ATED, T= TW MARILY FOR EVELOPMENTS
3	* I	ELEV (MSL)	**************************************	LA RUNOFF 155.1 152.3 154.2 154.8 156.3	RUNDF 134.0 135.2 136.2 137.0	CLUDI XCAV PRI EEN D
SUBWATERSHED		* * * * *	ASS ***********************************	0BESIGN STORM 4-1 1710 # 1-2 4600 # 2-3 3010 # 3-4 2190 # 5-1 1680 #	PORT, MASS DESIGN STORM 4.1 6770 # 5.0 6750 # 6.1 5820 # 7.1 5140 #	A. DRAGE, DROP, SHOWN IION BE
SUBMA	SPILLWAY	* * * * * * * * * * * * * * * * * * *	######################################	QUAD-WESTPORT, MASS PRIN SPWY DESIGN S C 367 4.1 17 D 113 1.2 46 D 206 2.3 30 D 299 3.4 21 C 449 5.1 16	QUAD-WESTPORT, MASS QUAD-WESTPORT, MASS PRIN SPWY DESIGN S E 108 4.1 67 E 104 4.0 81 E 132 5.0 67 E 159 6.1 58 E 187 7.1 51	TERIA AND COST DATA. BASED ON TOTAL STORAGE, CHUTE, D=CONCRETE DROP, CRMATION. FIGURES SHOWN COT TO SHOW VARIATION B E SELECTION OR LAND ACQ
1	EMERGENCY SI	STORAGE AT CREST AC FT II	* Q Z +	QZ (UAD-WEST RIN SPWY E 108 E 132 E 159 E 187	SED ON TOTAL SED ON TOTAL IUTE, D=CONCRE MATION, FIGUR I TO SHOW VAR
	EMER(###### USGS QUA 00-YR PRI 58.5 E 44.7 E 55.0 E 65.5 C		USGS QUA 100-YR PRI 130-8 E 131-8 E 131-8 E 132-8 E	ESIGN CRITERIA AND COST DAI OSTS ARE BASED ON TOTAL STO CONCRETE CHUTE, D=CONCRETE INARY INFORMATION, FIGURES EST 0.1 FCOT TO SHOW VARIAT EE.
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ARDS BA		**************************************	######################################	1 = 1062 QUALITY 15130 13210 11420 1 9820 1	I = 314 QUALITY 41560 1 36640 1 32890 1	NY STORAGE AND CONTROL OF THE CODE CONTROL OF THE CODE CONTROL OF THE CODE CONTROL OF THE CODE CONTROL OF THE TO THAT DEGRED ON THE TO THAT DEGRED OF THE TO THAT DEGRED OF THE TO THAT DEGRED OF THE TO THE
A-BUZZ		**************************************	******* SG MI WATER Q 9 23 61 55 31 99 18 138 15	SQ MI 11 134 157 11 77	SQ MI ATER Q 3 41 25 36 28 32 31 30	1973 S.C. STORAGE TYPE CODI ASED ON PI WN TO THE E TO THAT
STUDY AREA-BUZZARDS BAY	L POOL	***** COST PER AR AC FT (A	######################################	DA= 1.66 SQ MI = 1C62 STREAM WATER QUALITY (11 2 5220 34 15130 7 3220 47 13210 9 2300 57 11420 11 1730 77 9820 13	= 0.49 SQ MI = 314 STREAM WATER QUALITY (3 41560 10 6960 25 36640 11 5970 28 32890 12 5250 31 3060C 13	BASED ON 1973 S.C.S. D SPILLWAY STORAGE AND CO SPILLWAY TYPE CODE— C= NTA ARE BASED ON PRELIM ARE SHOWN TO THE NEAR ACCURATE TO THAT DEGR
ST	eneeeeeeeeeeeeeeeeeeeeeeeeeeeee	**************************************	* 0 0 0 * * * * * * * * * * * * * * * *	SITE—BB-4605 SITE RATING (1) STREAM WATER QUALITY (1) STREAM WATER QUALITY (1) STREAM WATER QUALITY (1) SITE AND WATER QUALITY (SITE—BB-4606 SITE RATING (1) STREAM WATER QUALITY (121.6 0 0.0 3.8 8440 20 41560 10 131.8 128 4.9 6960 25 36C4C 11 132.8 155 5.9 5970 28 3289C 12 133.7 183 7.0 5250 31 3060C 13	COSTS ARE BASED ON 1973 S.C.S. DESIGN CRIENCENCY SPILLWAY STORAGE AND COSTS ARE EMERGENCY SPILLWAY TYPE CODE— C=CONCRETE TABULAR DATA ARE BASED ON PRELIMINARY INFELEVATIONS ARE SHOWN TO THE NEAREST 0.1 F CONSIDERED ACCURATE TO THAT DEGREE.
1	# E	******* STORAGE AC FT II	0 0 0 0 495 2 1285 7 2103 11 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8	0 0 100 1 193 2 285 3 436 4	006 100 128 4 155 5	EMERCOST EMERCOST TABUL
1		******* ELEV (MSL) AC	\$\frac{8}{11} \frac{1}{6} \fra	SITE-BB-4605 SITE RATING 141.8 0 146.2 100 148.5 193 150.3 285 152.5 436	SITE-BB-4606 SITE RATING 121.6 0 130.6 131.8 132.8 132.8 133.7 183	(3) (3) (4) (4) (4) (4) (4)
1	•	ELEV (MSL	\$1118 \$1118 \$144 \$444 \$144 \$144 \$144 \$14	SITE-B SITE 141.8 146.2 148.5 150.3 152.5	SITE—B SITE—B 121.6 130.6 131.8 132.8 133.7	NOTES

Existing Site BB-4609(Forge Pond)

Location:

On the East Branch of the Westport River just upstream from Forge Road in Westport, Mass.

Fall River East, Mass. USGS quadrangle

Latitude: 41°37'48" Longitude: 71°03'13"

Surface Area (Acres)

Height of Dam(Ft.)

Drainage Area (Acres)

Potential for Expansion: The pond is surrounded by streets which would be affected by expansion. Topography limits any significant increase in surface area.

Remarks:

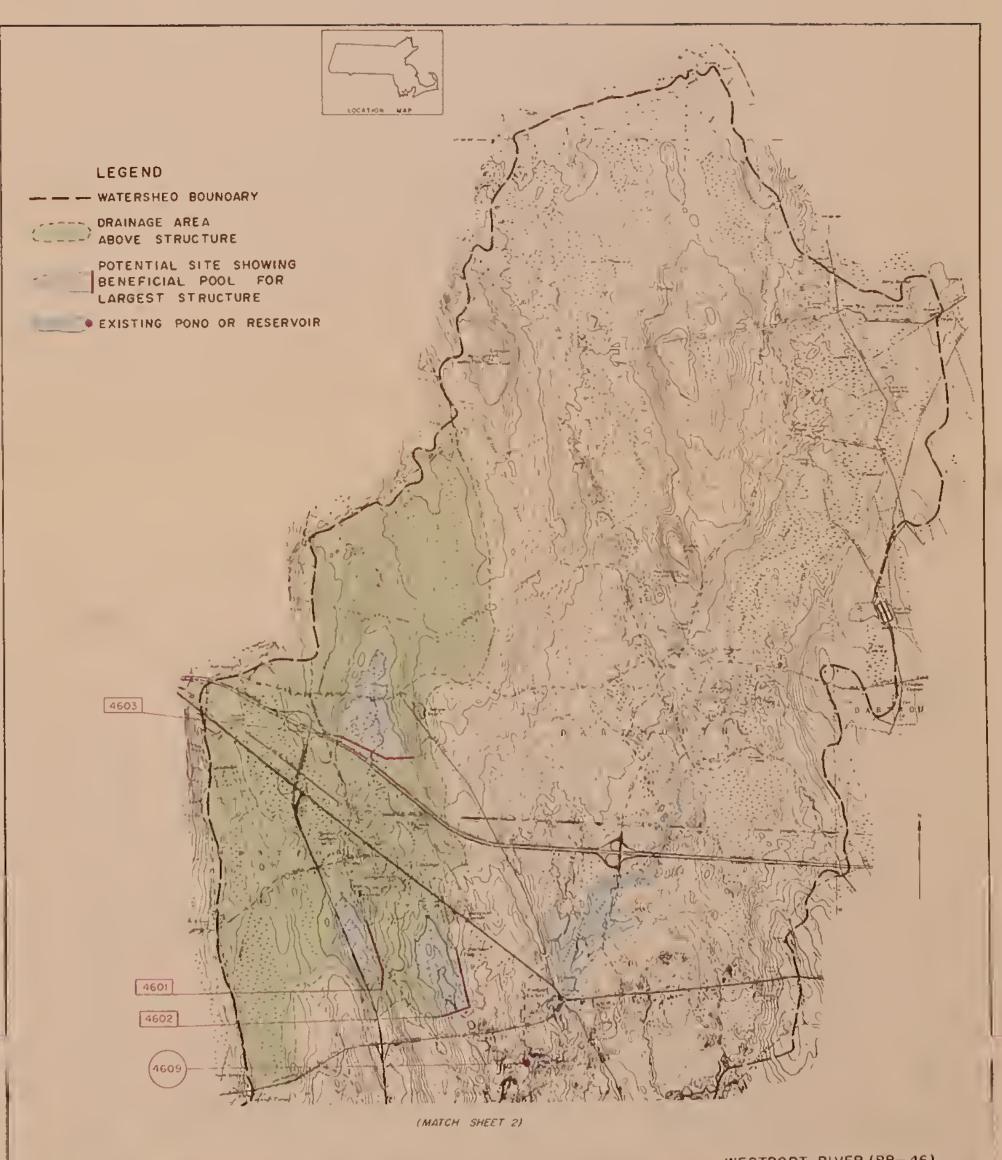
The dam is an earthfill structure with a concrete gravity section near the right abutment. The gravity section serves as the principal spillway. Three sets of I-beams serve as flashboard channels. The flashboards can be used to drain the pond. There is also a mill inlet located near the center of the dam.

Ownership and Use:

The pond is owned by Hoit Manufacturing Company and is used for cooling and fire protection.



Existing Site BB-4609(Forge Pond)



100 The 160 The

SOURCE - U.S.G.S. OUAD. SHEETS

WESTPORT - 1963

FALL RIVER EAST-1963 NEW BEDFORD NORTH-1964 WESTPORT RIVER (BB-46)

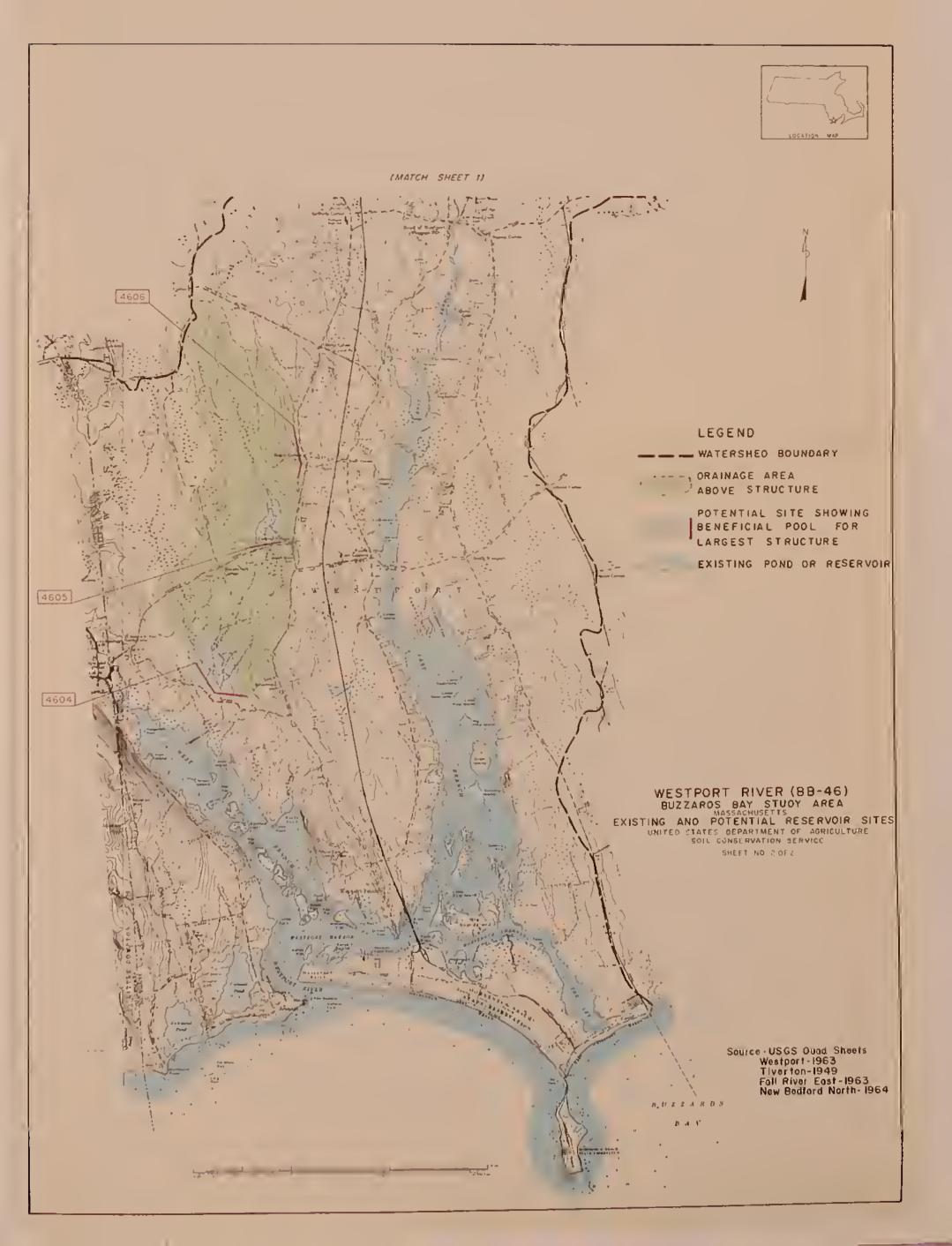
BUZZARDS BAY STUDY AREA

MASSACHUSETTS

EXISTING AND POTENTIAL RESERVOIR SITES

UNITED STATES DEPARTMENT OF AURICULTURE SOIL CONSERVATION SERVICE SHEET NO 1 OF 2







ISLANDS STUDY AREA SITE DATA FOR

Subwatershed IS-69, Dukes

The Dukes Study Area covers about 52,600 acres in Chilmark, Edgartown, Gay Head, Gosnold, Oak Bluff, Tisbury, and West Tisbury; all in the islands comprising Dukes County. The islands are located in the Atlantic Ocean off the southern Coast of Massachusetts.

Geology in the subwatershed is characterized by Cretaceous clay overlain by sand and gravel.

Six potential reservoir sites and 6 existing reservoirs were studied.

POTENTIAL SITE IS-6901

Location:

On Mill Brook about 2100 feet downstream from the Gay Head Road in Chilmark, Mass.

Squibnocket, Mass. USGS quadrangle

Latitude: 41°20'24" Longitude: 70°44'27"

Facilities
Affected:

Facility	Elevation
Barn	70
2 sheds	70
House	6 5
Barn	65
Gravel road	65
House	60
Gay Head Road	60
House	40

Geologic Conditions:

Both abutments are outwash sand and gravel with possible clay beds low on the left abutment and high on the right. Depth to Cretaceous clay in the foundation is estimated to be from 25 to 50 feet. Waterholding capabilities appear to be fair; slight leakage is expected through both abutments. Borrow material for dam construction was located near the site.

Engineering Notes:

Waterholding capabilities may be improved if a cut-off is made to the Cretaceous clay. The left abutment is recommended for the excavated emergency spillway location.

POTENTIAL SITE IS-6902

Location:

On the Tiasquam River about 300 feet upstream from Middle Road in Chilmark, Mass.

Tisbury Great Pond, Mass. USGS quadrangle

Latitude: 41°22'11" Longitude: 70°42'25"

Facilities	Facility	Elevation
Affected:	House	130
	Recreation Hall	130
	House	120
	Barn	1 20
	Meetinghouse Road	1 20
	Shed	115
	Middle Road	110
	Teaberry Lane	110

Geologic Conditions: Both abutments are morainal sand and gravel with large boulders. Depth to Cretaceous clay in the foundation is estimated to be from 80 to 100 feet. Waterholding capabilities appear to be fair to poor. Leakage is expected through both abutments. Borrow material for dam construction was located near the site.

Engineering Notes:

The left abutment is recommended for the excavated emergency spillway location.

POTENTIAL SITE IS-6903

Location:

On Paint Mill Brook about 100 feet upstream from North Road in Chilmark, Mass.

Naushon Island, Mass. USGS quadrangle

Latitude: 41°23'20" Longitude: 70°43'05"

Facilities Facility Elevation
Affected: House 100
Shed 100

Geologic Conditions:

Both abutments are thin morainal sand and gravel, shallow to red or gray clay. Depth to Cretaceous clay in the foundation is estimated to be from 5 to 15 feet. Waterholding capabilities appear to be good. Borrow material for dam construction was located near the site.

Engineering Notes:

The right abutment is recommended for the excavated emergency spillway location.

POTENTIAL SITE IS-6904

Location:

On the Tiasquam River about 600 feet downstream from the Chilmark-West Tisbury town line in West Tisbury, Mass.

Vineyard Haven, Mass. USGS quadrangle

Latitude: 41°22'43" Longitude: 70°41'16"

Facilities	Facility	Elevation
Affected:	Middle Road	80
	Barn	80
	2 houses	75
	House	70
	4 Barns	65
	House	60
	Barn	60
	House	35

Geologic Conditions: Both abutments are morainal sand and gravel underlain by clay. Depth to Cretaceous clay in the foundation is estimated to be from 10 to 15 feet. Waterholding capabilities appear to be good. Borrow material for dam construction was located near the site.

Engineering Notes:

The right abutment is recommended for the excavated emergency spillway location.

POTENTIAL SITE IS-6905

Location:

On Mill Brook about 500 feet downstream from Fisher Pond in West Tisbury, Mass.

Vineyard Haven, Mass. USGS quadrangle

Latitude: 41°23'55" Longitude: 70°41'30"

Facilities	Facility	Elevation
Affected:	North Road	90
	House	70

Geologic Conditions: Both abutments are morainal sand and gravel, possibly bedded sand and gravel. The right abutment has poorly graded gravel at the surface. Depth to Cretaceous clay in the foundation is estimated to be from 5 to 10 feet. Waterholding capabilities appear to be fair; leakage is expected through both abutments. Borrow material for dam construction was located near the site.

Engineering Notes:

The right abutment is recommended for the excavated emergency spillway location. Waterholding capabilities may be improved if a cut-off to the clay is made.

POTENTIAL SITE IS-6906

Location:

On Mill Brook about 4800 feet upstream from Edgartown-West Tisbury Road in West Tisbury, Mass.

Vineyard Haven, Mass. USGS quadrangle

Latitude: 41°23'44" Longitude: 70°40'25"

Facilities	Facility	Elevation
Affected:	2 houses	50
	Vineyard Haven Road	50
	South Road	50
	North Road	50
	3 houses	45
	2 barns	45
	House	40

Geologic Conditions: Both abutments are outwash sand and gravel. Depth to Cretaceous clay in the foundation is estimated to be from 30 to 40 feet. Waterholding capabilities appear to be fair to good; slight leakage is expected through both abutments. Borrow material for dam construction was located near the site.

Engineering Notes:

Preliminary designs indicate that a concrete drop structure emergency spillway will probably be required at the site. Waterholding capabilities may be improved by an effective cut-off.

SUMMARY DATA FOR POTENTIAL UPSTREAM RESERVOIR SITES

	* SAI	FILL *PERCENT FILL *PERCENT HGT VOL *CHANCE (1000 *	**************************************	30 · 57 * * * * * * * * * * * * * * * * * *	**************************************	26 53 * ***** 22 39 * 0.17 25 48 * 0.23 26 55 * 0.29 28 64 * 0.37	**************************************	14 6 * * * * * * * * * * * * * * * * * *	**************************************
DUKES	* *	* TOP ELEV AREA * ELEV (MSL) (AC) * (MSL)	**************************************		**************************************	124.8 54 * 127.8 121.5 39 * 124.5 123.6 49 * 126.6 125.1 56 * 128.2 127.3 66 * 130.0	**************************************	95.1 23 * 98.1 96.9 30 * 99.9 97.4 32 * 100.0	**************************************
SUBWATERSHED DUKES	SPILLWAY * HIG	ST PER EL IN (\$) * (MS	SNOCKET, MASS DESIGN STORM	4.1 3210 ** 3.5 4380 ** 6.4 3190 ** 8.2 2730 **	**************************************	4.1 2680 * 12 2.5 5160 * 12 3.4 3890 * 12 4.4 3200 * 12 6.0 2560 * 12	*	0.6 10510 * 9 1.1 6430 * 9 1.4 5550 * 9	**************************************
	F EMERGENCY SP	*	USGS QUAD-SQUIE	99 60 62 61 91	USGS QUAD-TISB	122.3 E 294 119.1 E 177 121.1 E 244 122.8 E 314 125.0 E 422	**************************************	92.8 E 29 94.5 E 55 95.0 E 67	*
LANDS			######################################		AC	6.1 * 33430 14.6 * 2983C 18.2 * 2479C 20.5 *	######################################	9841C 6.3 * 3646C 8.0 * 8.5 * 4.5 * 8.5 *	**************************************
STUDY AREA-ISLANDS	BENEFICIAL POOL	COST COST COST DEPELEV STORAGE PER AREA SURF AT AC FT AC FT AC (MSL) AC FT IN (\$) (AC) (\$) (FT	SITE-IS-6901 DA= 1.35 SQ MI = 864 SITE-RATING (2) STREAM WATER QUALITY (9600 17 6370 26 4310 37 3640 49	**************************************	3 6250 23 4930 34 3750 44	**************************************	3 35480 10 21580 12	**************************************
	BENEFI	STORAGE AC FT IN	S-6901 RATING (2)	1	SITE RATING (3)	100 0.0 152 2.0 204 2.9 289 4.1	**************************************	10 0.0	(2)
***************************************		ELEV (MSL)	SITE-IS-6901	51.2 55.0 60.0 62.5	SITE-I SITE	108.1 116.6 118.6 120.3 122.5	SITE-1 SITE	90.3 92.0 92.5	**************************************

EMERGENCY SPILLWAY TYPE CODE— C=CONCRETE CHUTE, D=CONCRETE URUP, E=EXCAVAFED, T= TWO SPILLWAYS, N= NONE TABULAR DATA ARE BASED ON PRELIMINARY INFORMATION. FIGURES SHOWN ARE PRIMARILY FOR COMPARISON PURPOSES. ELEVATIONS ARE SHOWN TO THE NEAREST 0.1 FCOT TO SHOW VARIATION BETWEEN DEVELOPMENTS ONLY, AND ARE NOT TO BE CONSIDERED ACCURATE TO THAT DEGREE.

** DO NOT USE FOR FINAL SITE SELECTION OR LAND ACQUISITION. **

SUMMARY DATA FOR POTENTIAL UPSTREAM RESERVOIR SITES

SAFE YIELD	* PERCENT *CHANCE * CMCD)	******* 70-41-16 478 CFS	***** 0.18 0.80 1.51 2.07	70-41-30 395 CFS	0.17 0.29 0.49 0.65	70-40-25 1571 CFS 0.20 0.26 0.35	# * * * * * * * * * * * * * * * * * * *
* * * *	* * PE * * CH	*** 7.7.		3 3		1070	NON DSES
	FILL VOL (1000	**************************************	72 31 99 216 402	**************************************	. 33 33 78 78 91	LONGITUDE K FLOW = 22 44 22 46 22 25 50	* * * * * * * * * * * * * * * * * * *
DAM	HGT	***** 3 LC EAK F	31 22 36 49 62	* * EA	22 21 22 26 26 29 31	4	OOL. ILLWAYS ARISUN Y, AND
Q	TOP ELEV	**************************************	57.1 47.7 61.6 74.9 88.1	41-23-5 00 IN, F	79 . 9 . 9 . 9 . 9 . 9 . 9 . 9 . 9 . 9 .	40 4	**************************************
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>	COST PER AC FT	# H	2000 4880 1910 1120 800	# * * * * * * * * * * * * * * * * * * *	3130 3950 3600 2390 11890 1740	'> ~100	A. A. IRAGE UROP SHOW
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ζÇ	STORAGE AT CREST	* > - C	203 384	* N N N N N N N N N N N N N N N N N N N	~ W W W W & &	14 SP 14 SP 123 233	AND ON T ON T O=C ION.
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EMERGENCY	<u>—</u> ш-	:	51.9 42.3 56.3 81.8	-4-	74.1 73.1 74.5 78.9 82.1 83.4	0,0,0	#### ITER BAS CHU FORM
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101	DOA	*** DA		#### DA= ST	*	1 1 1	ARE **
BENEFICIAL POOL	. w =	* ~	0.0 0.7 4.8 12.8 25.0	* -	0 H S S S S S S S S S S S S S S S S S S	0.3	**************************************
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	STO AC FT	******* -6904 RATING	100 648 1743 3387	**************************************	100 206 418 630 736	100 133 187	* C C C C C
		SITE-IS-6904 SITE RATIN		**************************************	*	SITE-IS-6906 SITE RATIN 39.9 10 41.0 13 42.5 18	25.00.00.00.00.00.00.00.00.00.00.00.00.00
	> -	**** TE-1S SITE	31.5 39.8 53.8 67.1	TE	62.1 68.6 72.0 76.4 79.6	SITE 39.9 41.0 42.5	* I * \$
	ELEV	S	2000	SI	* 8 7 7 8 8	11E S1 39 41 42	**************************************

Existing Site IS-6907(Bliss Pond)

Location:

On Mill Brook about 700 feet upstream from Menemsha Crossroad on Marthas Vineyard in Chilmark, Mass.

Squibnecket, Mass. USGS quadrangle

Latitude: 41°21'10" Longitude: 70°45'07"

Surface Area Height of Drainage Area

(Acres) Dam(Ft.) (Acres) Sq. Mi.

3 10 250 0.39

Potential for

Raising the present pond level by 20 feet would create a 20 acre pool. Sandy soils may limit expansion po-Expansion: tential.

Remarks:

The dam is an earthfill. The spillway is a stone masonry drop structure with flashboards at each end. The outlet channel is lined with stone. Some portions of the dam have erosion problems.

Ownership and Use:

The pond is owned by the Bliss Farm and is used for stock water and wildlife habitat.

Existing Site IS-6908(Fisher Pond)

Location:

On Mill Brook about 2800 feet downstream from the Chilmark-West Tisbury town line in West Tisbury, Mass.

Vineyard Haven, Mass. USGS quadrangle

Latitude: 41°23'50" Longitude: 70°41'35"

Surface Area Height of Drainage Area

(Acres) Dam(Ft.) (Acres) Sq. Mi.

9 850 1.33

Potential for Expansion: Raising the pond level by 15 feet would create a 60 acre pool. Sandy soils may limit expansion potential.

Remarks:

The dam is an earthfill structure. The downstream face is riprapped. The upstream slope is partially riprapped. The spillway is a stone masonry drop structure with a concrete slab set at water level. The spillway has provision for flashboards at the downstream end. Trees and brush are growing on the right portion of the dam.

Ownership and Use:

The pond is owned by Edwin Woods and is used for wildlife habitat.

Existing Site IS-6909 (Crocker Pond)

Location:

On Mill Brook about 1100 feet upstream from South Road in West Tisbury. Mass.

Vineyard Haven, Mass. USGS quadrangle

Latitude: 41°24'11" Longitude: 70°40'55"

Surface Area (Acres)

Height of Drainage Area

Dam(Ft.) (Acres) Sq. Mi.

11 1,450 2.27

Potential for Expansion: Raising the pond level by 10 feet would create a 40 acre pool. North Road and several houses would be affected. Sandy soils may limit expansion potential.

Remarks:

The dam is an earthfill structure with stone masonry on the upstream and downstream face. The spillway system consists of two concrete structures. One has small stone masonry steps and a broad-crested weir which functions as an emergency spillway. The other spillway has provision for flashboards.

Ownership and Use:

The pond is owned by Seven Gates Farm Corporation and is used for wildlife habitat.

Existing Site IS-6910(Priester Pond)

Location:

On Mill Brook about 200 feet upstream from South Road in West Tisbury, Mass.

Vineyard Haven, Mass. USGS quadrangle

Latitude: 41°24'08' Longitude: 70°40'44"

Surface Area

rface Area Height of Drainage Area (Acres) Dam(Ft.) (Acres) Sq. Mi. 7 2,025 3.16

Potential for Expansion: Limited by Crocker Pond which is located immediately upstream. Sandy soils may limit expansion potential.

Remarks:

The dam is an earthfill structure with stone masonry on the upstream face. The spillway is a concrete weir with a notch which serves as the principal spillway. There are two other gated outlets which are no longer in use. Trees are growing on the dam. Water is flowing through the concrete in one area of the spillway.

Existing Site IS-6910(Priester Pond)(Cont.)

Ownership and Use:

The pond is owned by Mrs. Brooke Anderson and is used for wildlife habitat.

Existing Site IS-6911(Davis Pond)

Location:

On Tiasquam River about 2400 feet upstream from South Road in West Tisbury, Mass.

Vineyard Haven, Mass. USGS quadrangle

Latitude: 41°22'52" Longitude: 70°41'03"

Surface Area Height of Drainage Area (Acres) 8 (Acres) Sq. Mi.

Potential for Expansion:

Raising the pond level by 20 feet would create a 35 acre pool. Sandy soils may limit expansion potential.

Remarks:

The dam is an earthfill structure with riprap on the upstream face. The spillway is a stone drop structure with a concrete section with provision for flashboards. Brush is growing on the dam and there are many roots in the dam fill. Seepage was apparent along the downstream toe of the dam.

Ownership and Use:

Ownership of the pond was not determined. The pond is used for wildlife habitat.

Existing Site IS-6912(Looks Pond)

Location:

On Tiasquam River about 500 feet upstream from South Road in West Tisbury, Mass.

Vineyard Haven, Mass. USGS quadrangle

Latitude: 41°22'40" Longitude: 70°40'46"

Surface Area (Acres)

Height of Dam(Ft.) Drainage Area (Acres) 1.850

Potential for Expansion: Raising the present pond level by 5 feet would create a 15 acre pool. Sandy soils may limit expansion potential.

Remarks:

The dam is an earthfill structure. The main spillway is a concrete drop structure with riprap at the inlet. There is also another concrete drop structure with flashboards.

Ownership and Use:

Ownership of the pond was not determined. The pond is used for wildlife habitat.

ISLANDS STUDY AREA SITE DATA FOR

Subwatershed IS-70, Nantucket

The Nantucket Study Area covers about 31,500 acres on the island of Nantucket in the Atlantic Ocean south of Cape Cod.

No potential or existing reservoir sites which met inventory criteria were located.



IS - 6907 Bliss Pond

IS - 6910 Priester Pond



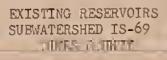
IS - 6908 Fisher Pond



IS - 6912 Davis Pond



IS - 6909 Crocker Pond

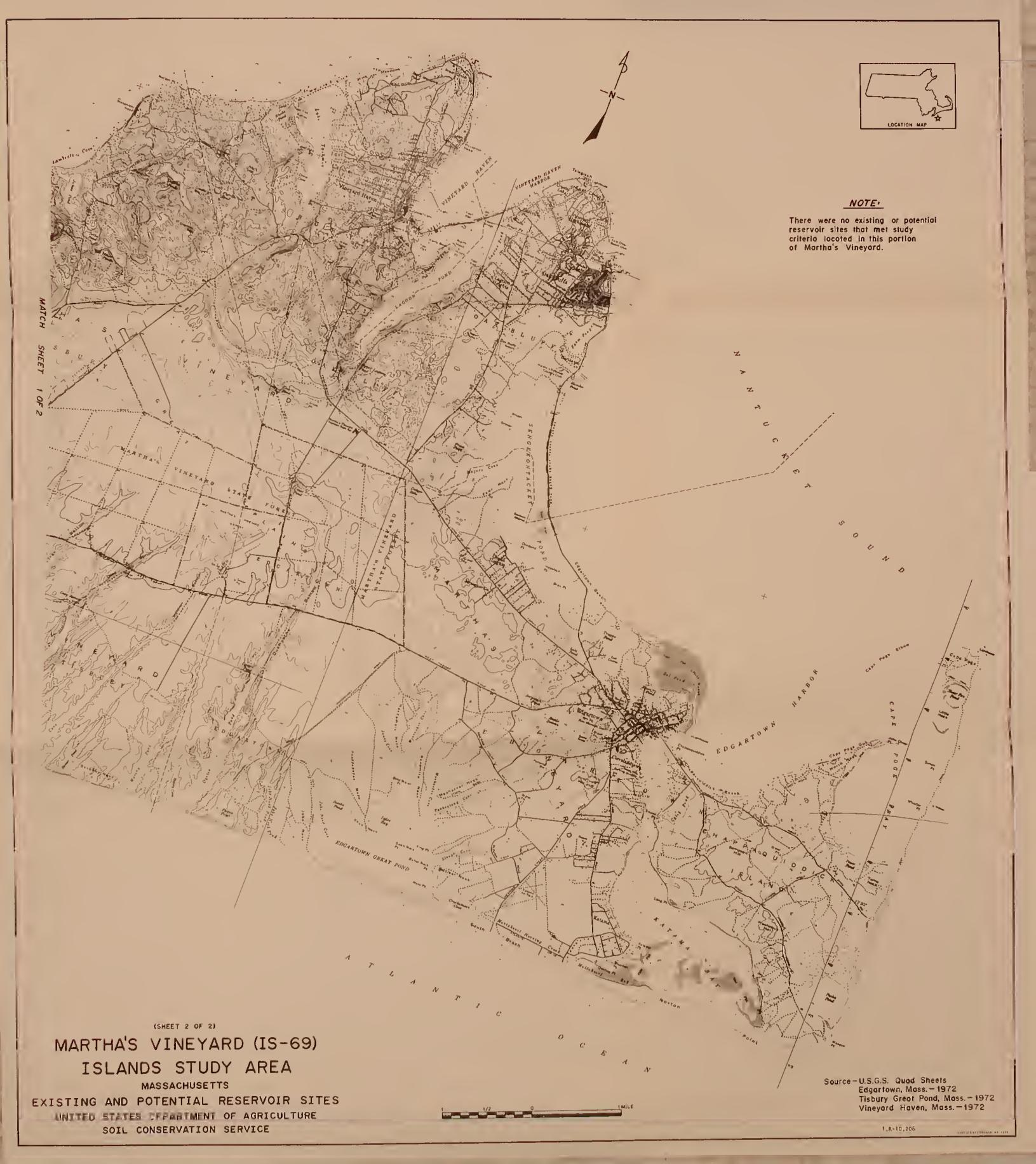


















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MUNICIPAL INDEX OF RESERVOIR SITE DATA

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Barnstable	CC-3901 CC-3903 CC-3905 CC-3906	103 104 105 107	106 106
Bourne	SS-3102 SS-3103 CC-4001 CC-4002	88 88 109 110	
Braintree	SS-2303 SS-2409 SS-2410 SS-2411 SS-2501	9 11 12 13 15	16
Brewster	CC-3301	93	
Carver	BB-4101 BB-4116 BB-4120 BB-4124	113 122 124 126	115
	BB-4201 BB-4212 BB-4215 BB-4217 BB-4221 BB-4222 BB-4238 BB-4339 BB-4340 BB-4350	127 129 129 129 129 129 130 134 134	128
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Cohasset	SS-2606 SS-2607 SS-2621	20 20 30	23 2l ₄

City or Town	Site No.	Narrative Information	Design Summary
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Hanover	SS-2713 SS-2721 SS-2722 SS-2724 SS-2725 SS-2740	34 43 44 45 45 52	38
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Holbrook	SS-2412	13	
Kingston	SS-2903 SS-2904 SS-2905 SS-2906 SS-2910 SS-2912 SS-2915	66 66 67 67 71 72 73	68 68 69 69

		Narrative	Design
City or Town	Site No.	Information Page	Summary Page
Marshfield	SS-2712 SS-2717 SS-2733 SS-2734 SS-2801 SS-2802 SS-2803 SS-2804 SS-2806 SS-2807 SS-2808 SS-2810 SS-2811	34 36,42 49 49 53 54 54 55 56,61 57 61 62 62	38 40 58 58 58 59 59 60
Mashpee	SS-2813 CC-3901 CC-3902 CC-3904	63 103 104 105	106 106
Mattapoisett	BB - կկ01 BB - կկ02	135 136 , 139	138 138
Middleborough	BB-4201 BB-4235 BB-4237	127 130 130	128
New Bedford	BB - 4505	148	
Norwell	SS-2620 SS-2701 SS-2703 SS-2705 SS-2706 SS-2726 SS-2727 SS-2732	30 31,41 32 32 33 46 46 48	37 37 37 38
Pembroke -	SS-2714 SS-2715 SS-2716 SS-2720 SS-2736 SS-2737 SS-2739 SS-2740 SS-2908 SS-2909	35 35 36,41 43 50 51 51 52 70	39 39 39

		Narrative	Design
City or Town	Site No.	Information Page	Summary Page
Plymouth	SS-2913 SS-2914 SS-3001 SS-3002 SS-3003 SS-3004 SS-3006 SS-3007 SS-3008 SS-3011 SS-3012 SS-3013 SS-3014 SS-3015 SS-3015 SS-3016 SS-3101 SS-3101 SS-3102 BB-4103 BB-4104 BB-4107 BB-4107 BB-4113 BB-4120	72 73 75,82 76,82 76 77 77 78 78 83 83 84 84 85 86 87 88 114,116 114 116 117 117 121 121 124	79 79 79 80 80 80 81
Plympton	SS-2911	71	
Randolph	SS-2409	11	
Rochester	BB-4403 BB-4404 BB-4405 BB-4406 BB-4407 BB-4408 BB-4409 BB-4410	136,139 140 140 141 141 142 143 143	138
Rockland	SS - 2723	7†7†	
Sandwich	CC-3201 CC-3202 CC-3203 CC-3204	89 90 90 91	

City or Town	Site No.	$\frac{\text{Narrative}}{\frac{\text{Page}}{}}$	Design <u>Summary</u> <u>Page</u>
Scituate	SS-2703	32	37
	SS - 2728	1,7	
	SS-2729	147	
	SS-2730	148	
Wareham	BB-4104	1114	115
	BB - 4105	116	
	BB - 4108	118	
	BB - 4109	118	
	BB - 4110	119	
	BB - 4111	120	
	BB - 4112	120	
	BB-4114	121	
	BB-4115	122	
	BB-4117	123	
	BB-4118	124	
	BB-4121	125	
	BB-4122	125	
	BB-4123	126	
	BB-4202	131	
	BB-4203	131	
	BB-4204	132	
Wellfleet	CC - 3501	95	96
Westport	BB - 4501	145	147
	BB - 4601	151	155
	BB - 4602	152	155
	BB - 4603	152	155
	BB - 4604	153	156
	BB-4605	153	156
	BB - 4606	154	156
	BB - 4609	157	
West Tisbury	IS-6904	161	164
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	IS-6906	162	164
	IS - 6908	165	
	IS - 6909	166	
	IS-6910	166	
	IS-6911	167	
	IS-6912	168	
Weymouth	SS-2615	27	
	SS-2616	28	
	SS-2617	28	
	SS-2618	29	
Yarmouth	CC-3801	99	100
	CC - 3802	101	

APPENDIX

This report is one of a series dealing with reservoir sites. Previous reports in the series are:

- 1. Study of Possible Water Storage Areas, Ipswich River Watershed, January 14, 1965.
- 2. Study of Possible Water Storage Sites, Upper Hoosic River and Upper Housatonic River, February 1966.
- 3. A Study of Potential Reservoir Sites in Massachusetts, Hudson River Basin, January 1968.
- 4. A Study of Potential Reservoir Sites, Housatonic Study Area, Massachusetts, June 1969.
- 5. Inventory of Potential and Existing Reservoir Sites, Merrimack Study Area, Massachusetts, March 1970.
- 6. Inventory of Potential Reservoir Sites, Neponset Study Area, Massachusetts, October 1970.
- 7. Inventory of Potential and Existing Upstream Reservoir Sites, Thames Study Area, Massachusetts, January 1971.
- 8. Inventory of Potential and Existing Upstream Reservoir Sites, Parker and North Shore Study Area, Massachusetts, June 1971.
- 9. Inventory of Potential and Existing Upstream Reservoir Sites, Nashua Study Area, Massachusetts, March 1972.
- 10. Inventory of Potential and Existing Upstream Reservoir Sites, Deerfield Study Area, Massachusetts, November 1972.
- 11. Inventory of Potential and Existing Upstream Reservoir Sites, Chicopee Study Area, Massachusetts, May 1973.
- 12. Inventory of Potential and Existing Upstream Reservoir Sites,

 Taunton and Narragansett Bay Study Areas, Massachusetts, January 1974.
- 13. <u>Inventory of Potential and Existing Upstream Reservoir Sites</u>, <u>Ipswich Study Area</u>, May 1974.
- 14. Inventory of Potential and Existing Upstream Reservoir Sites, Millers Study Area, July 1974.
- 15. Inventory of Potential and Existing Upstream Reservoir Sites, Connecticut Valley Study Areas, January 1975.

Reports will be prepared in future years for the remainder of the state. Basic data from which this report was prepared are on file in the Soil Conservation Service Office, 29 Cottage Street, Amherst, Massachusetts 01002.

